

Artificial fill (Holocene):

Mostly silty sand, clayey sand and sandy silt placed in previous grading operations.



Young Alluvium (Holocene):

Unconsolidated alluvium consisting of fine to coarse grained sand and lesser gravel and silt.



Old Alluvium (Pleistocene):

Slightly indurated, sandy alluvial fan deposits associated with the Santa Ana River system.



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PROJECT NAME
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HCA Design and Construction

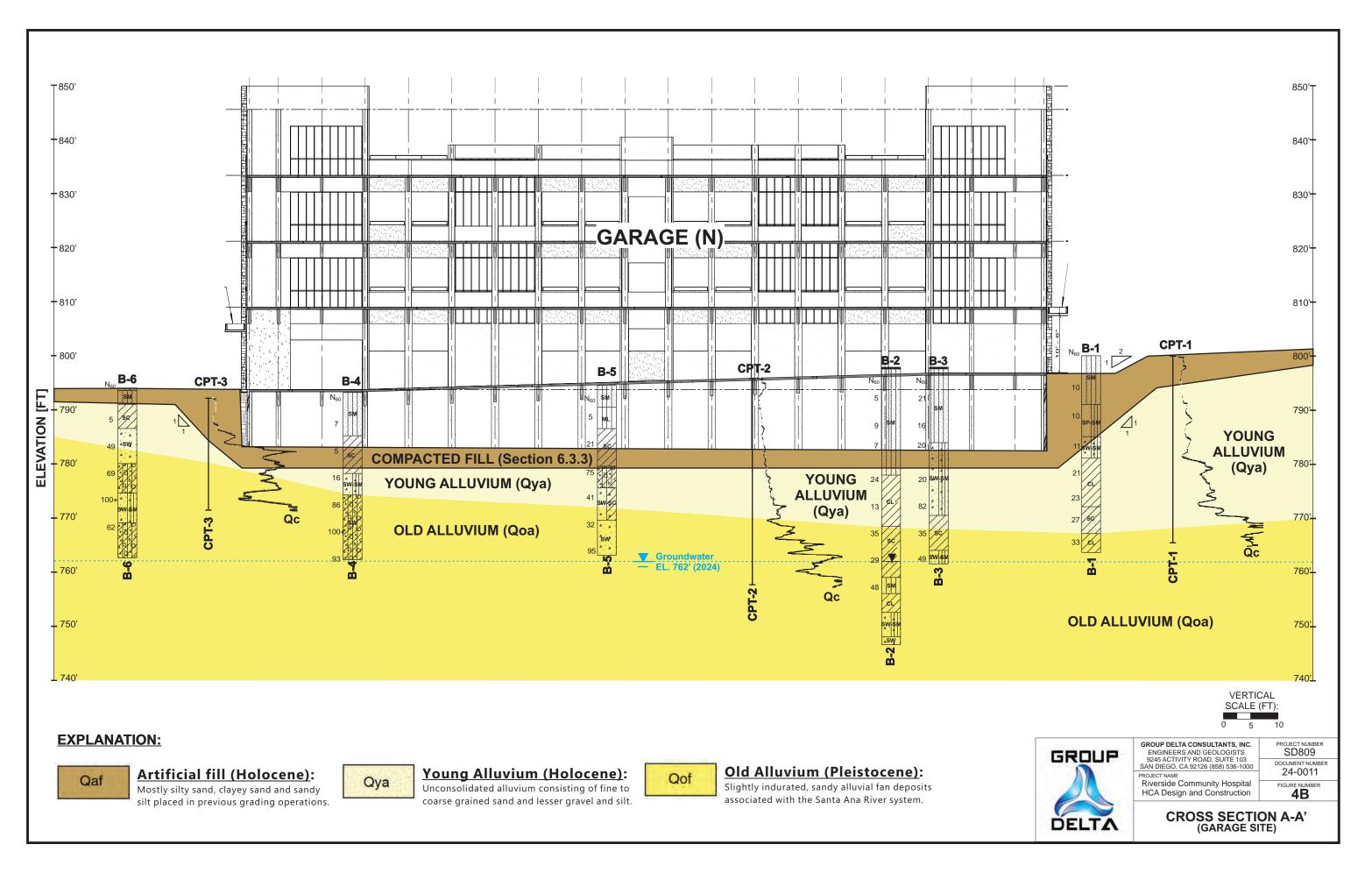
PROJECT NUMBER SD809

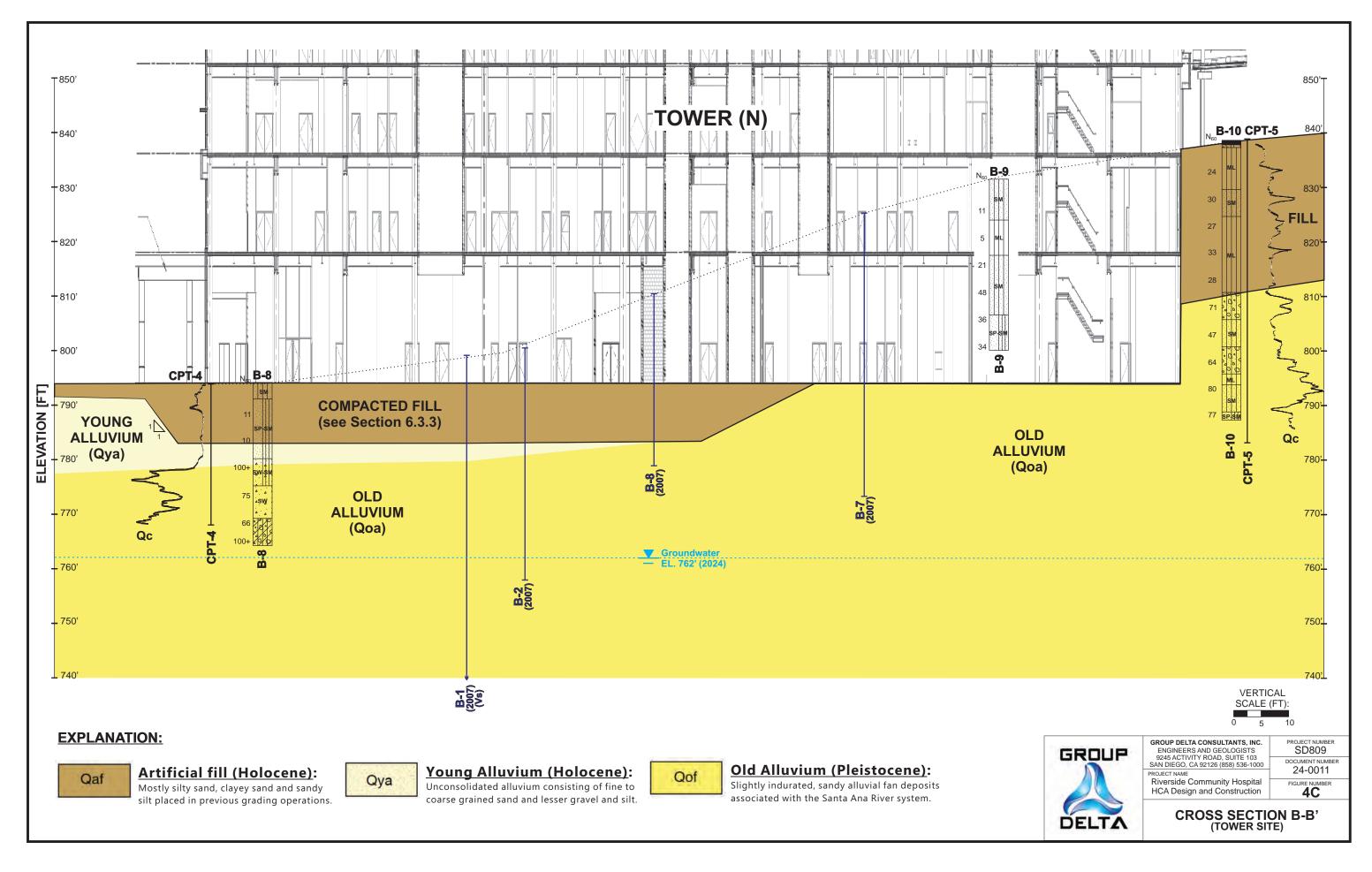
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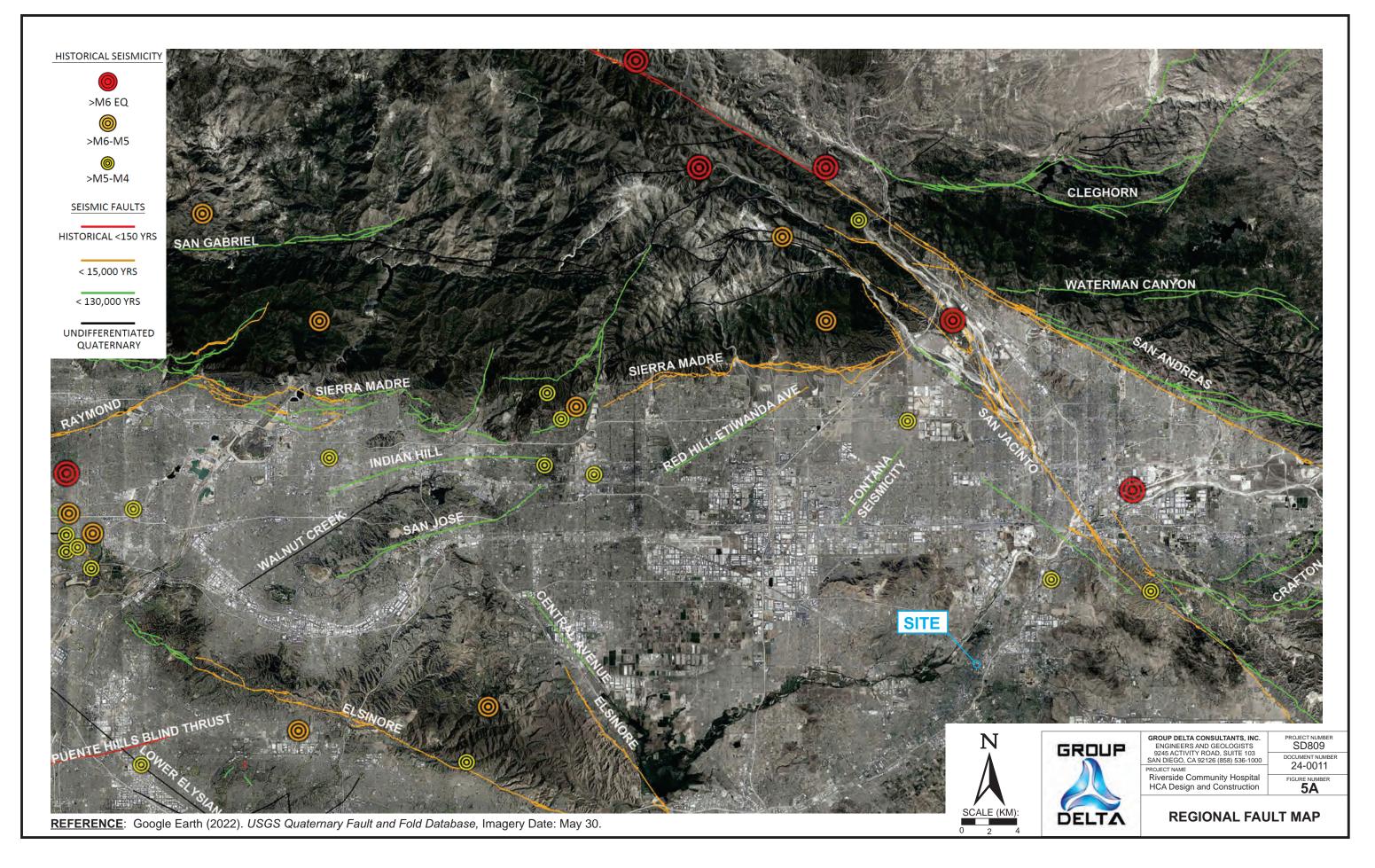
FIGURE NUMBER

LOCAL GEOLOGIC MAP

REFERENCE: Morton and Cox (2002). Geologic Map of the Riverside West 7.5' Quadrant, Riverside, California.









EXPLANATION:



FEMA 100-Year Flood Zone AE:

The approximate limits of the FEMA 100-year flood within the Tequesquito Arroyo are shown in blue.



FEMA 100-Year Flood Elevations;

The approximate elevations of the 100-year flood within the Tequesquito Arroyo is also shown [MSL].

NO SCALE



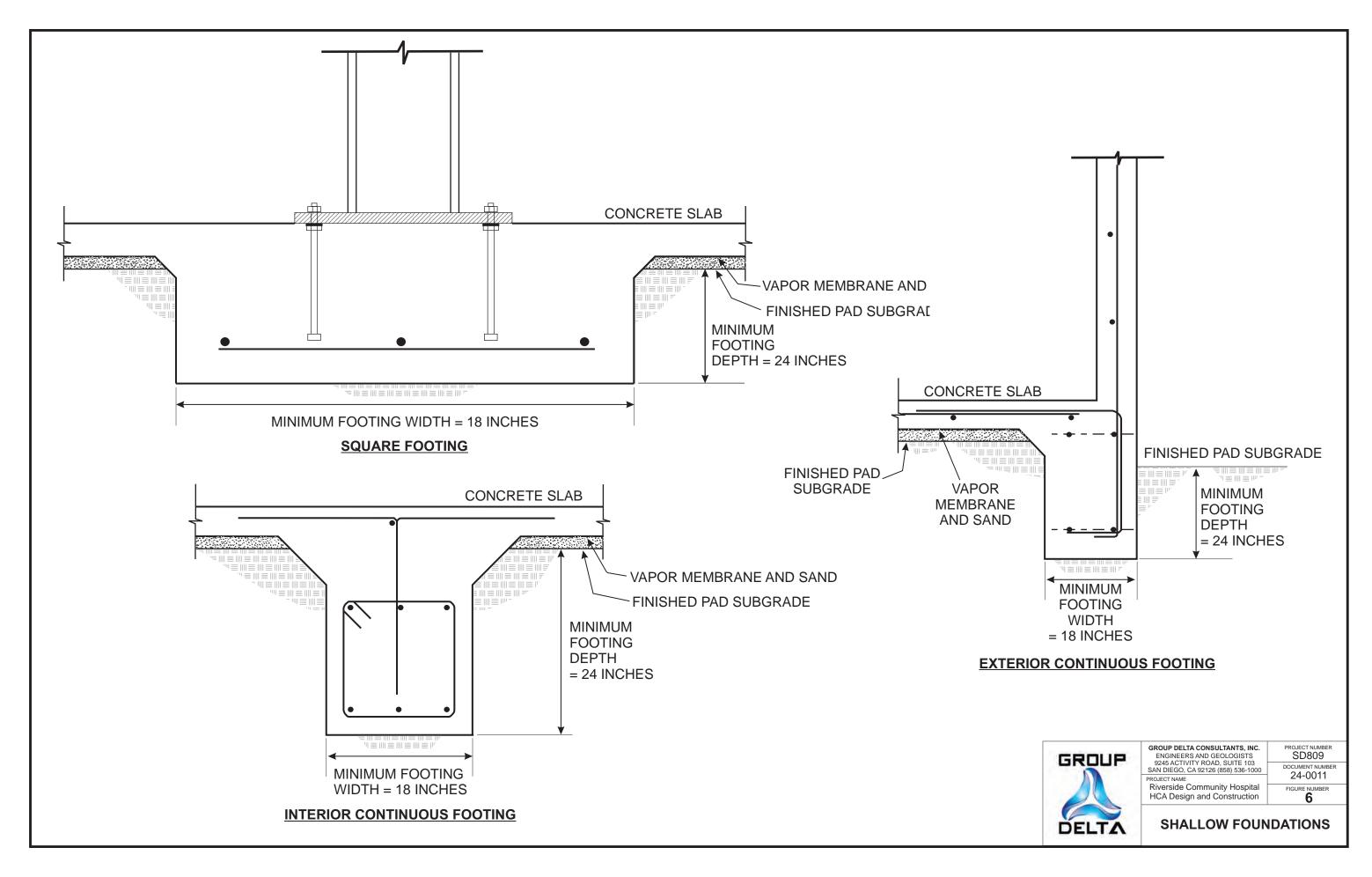
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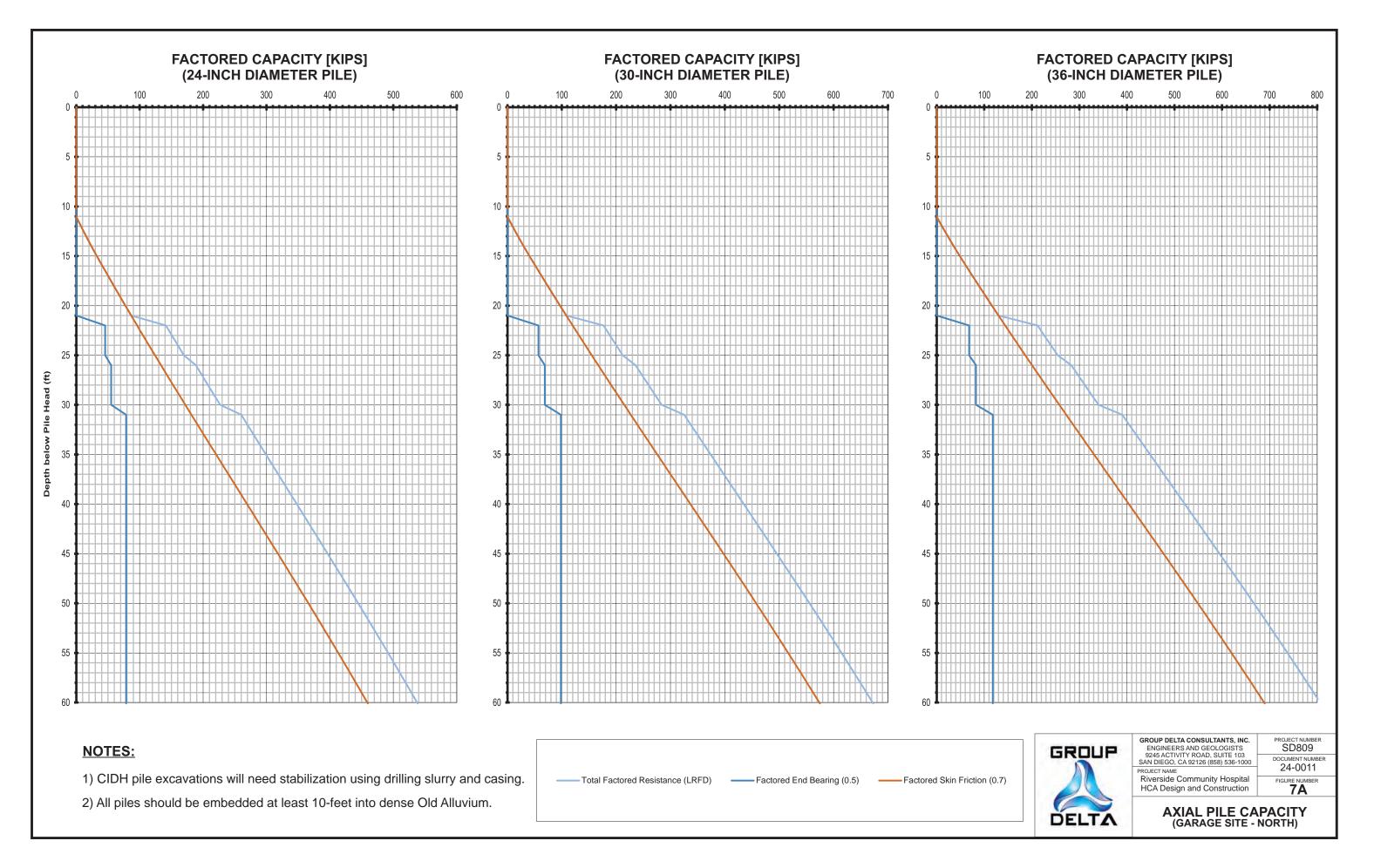
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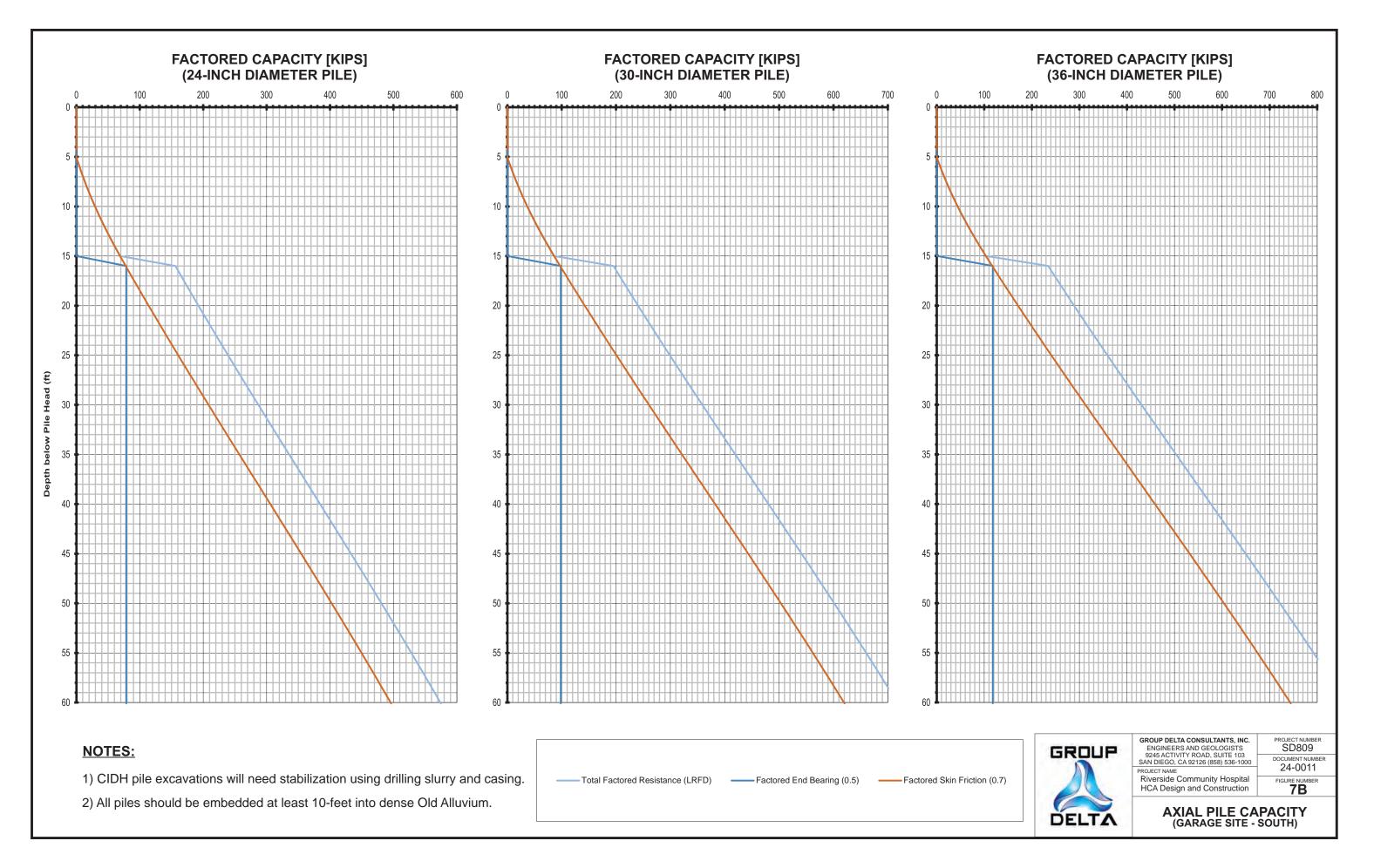
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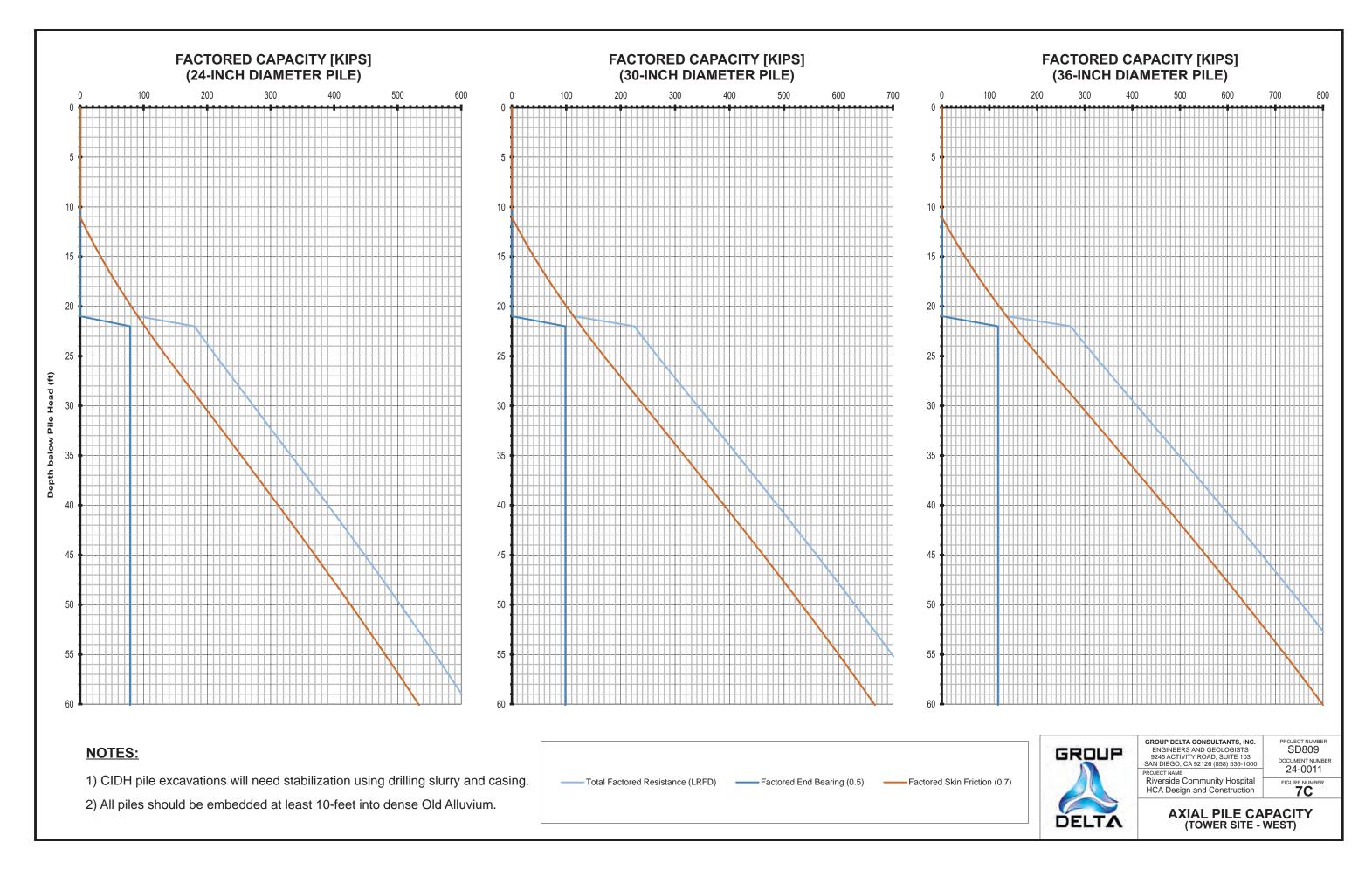
FEMA FLOOD MAP

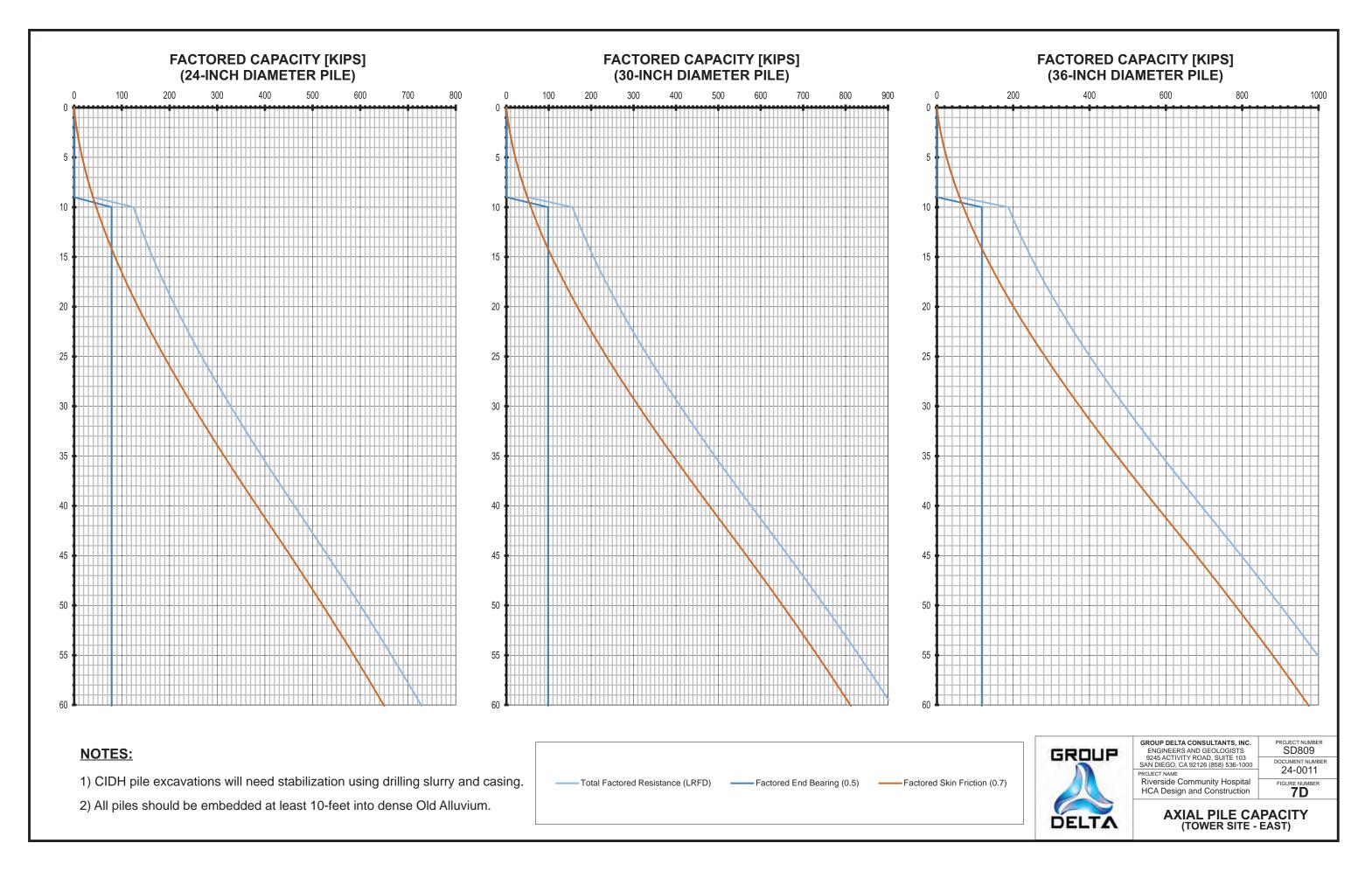
Reference: FEMA (2021). FEMA's National Flood Hazard Layer (NFHL) Viewer, Map 06065C0710G, https://hazards-fema.maps.arcgis.com/apps/webappviewer.

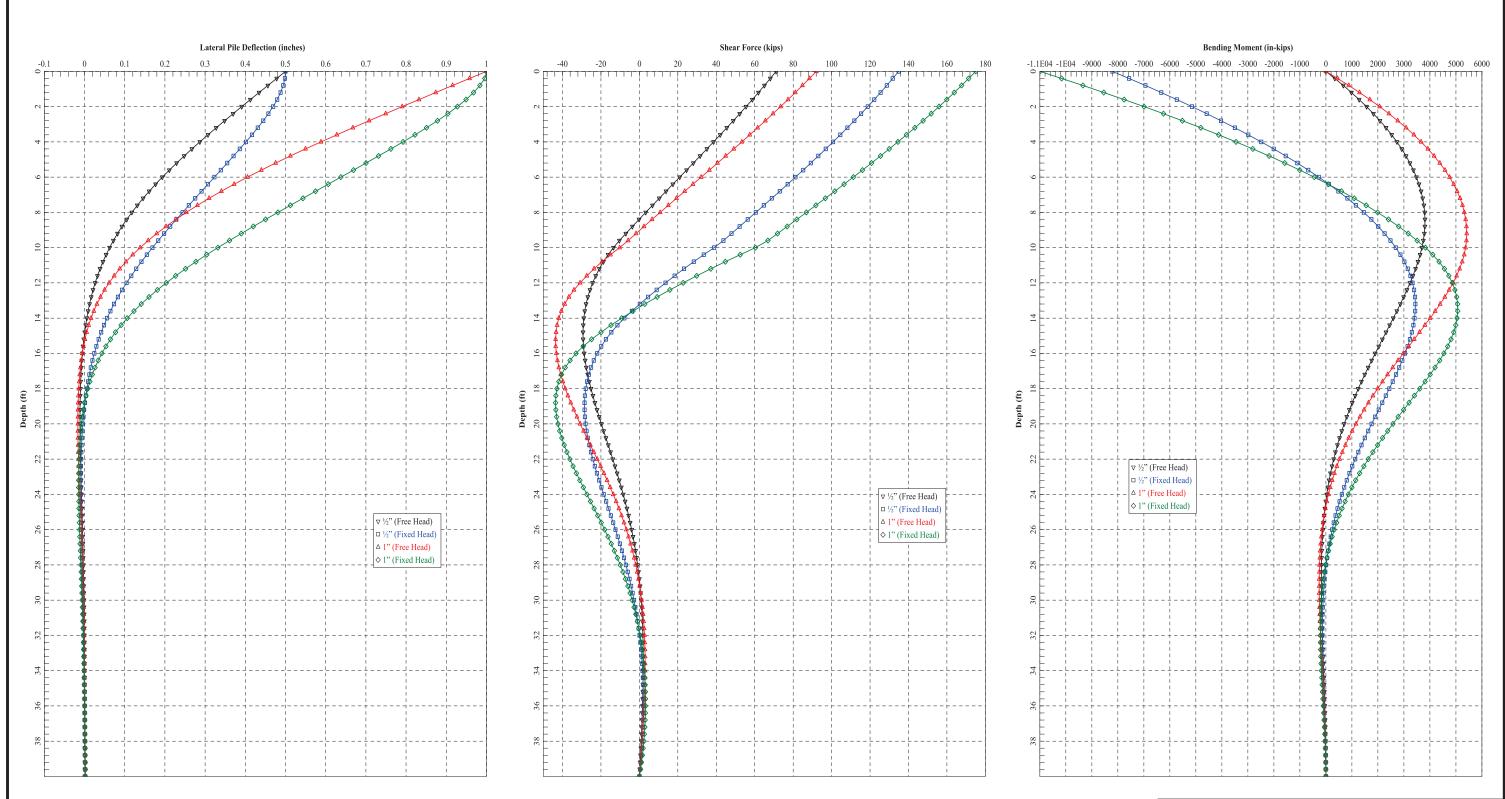












- 1) The approximate lateral response of a single 30-inch diameter, 40-foot long pile is shown in the deflection, shear and moment diagrams provided above.
- 2) The analyses assume at least 10-foot embedment into dense Old Alluvium, with 4,000 psi concrete strength and (6) No. 14 transverse bars (Grade 60).



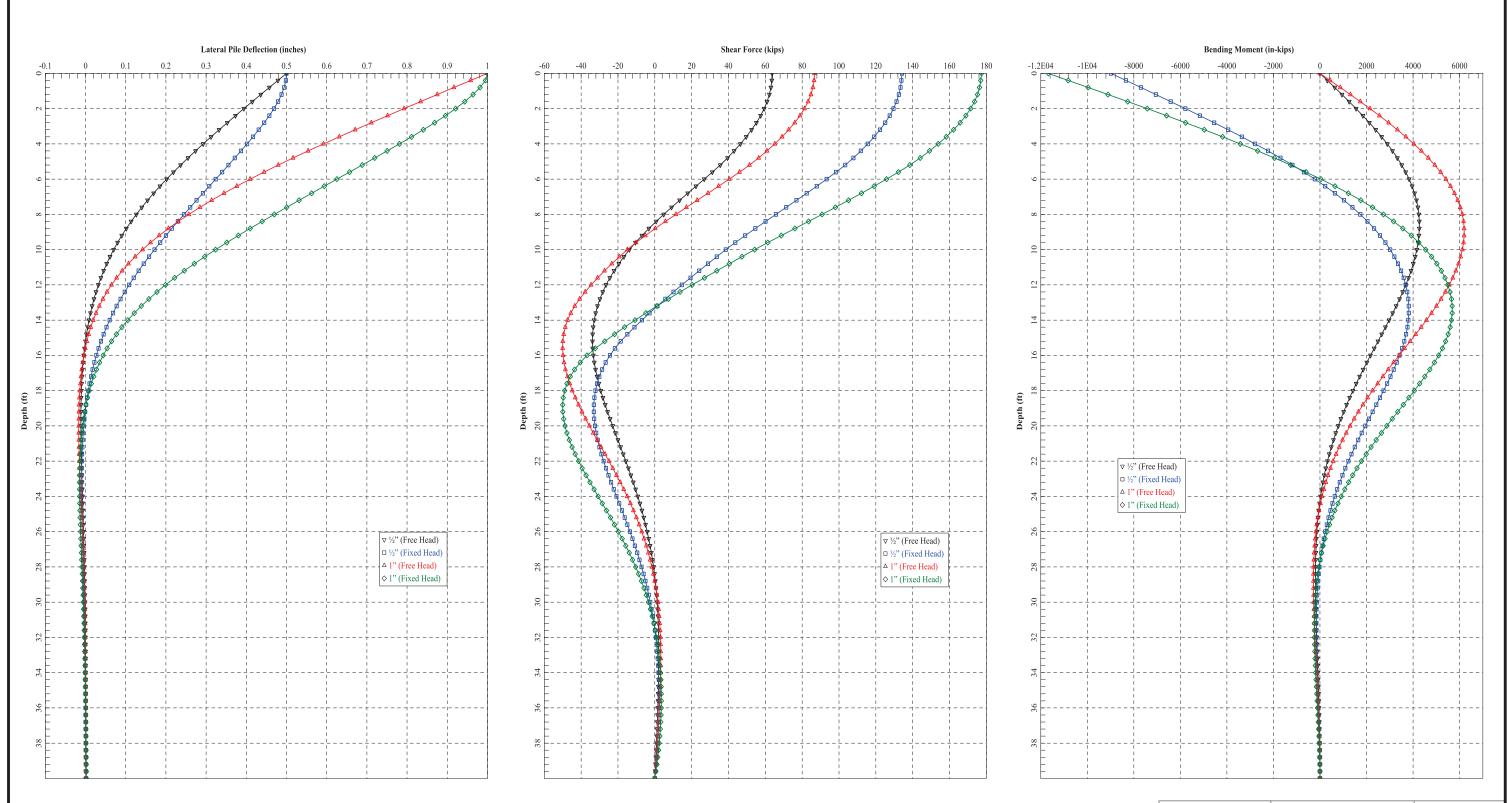
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DOCUMENT NUMBER 24-0011

LATERAL PILE CAPACITY (GARAGE SITE - NORTH)



- 1) The approximate lateral response of a single 30-inch diameter, 40-foot long pile is shown in the deflection, shear and moment diagrams provided above.
- 2) The analyses assume at least 10-foot embedment into dense Old Alluvium, with 4,000 psi concrete strength and (6) No. 14 transverse bars (Grade 60).



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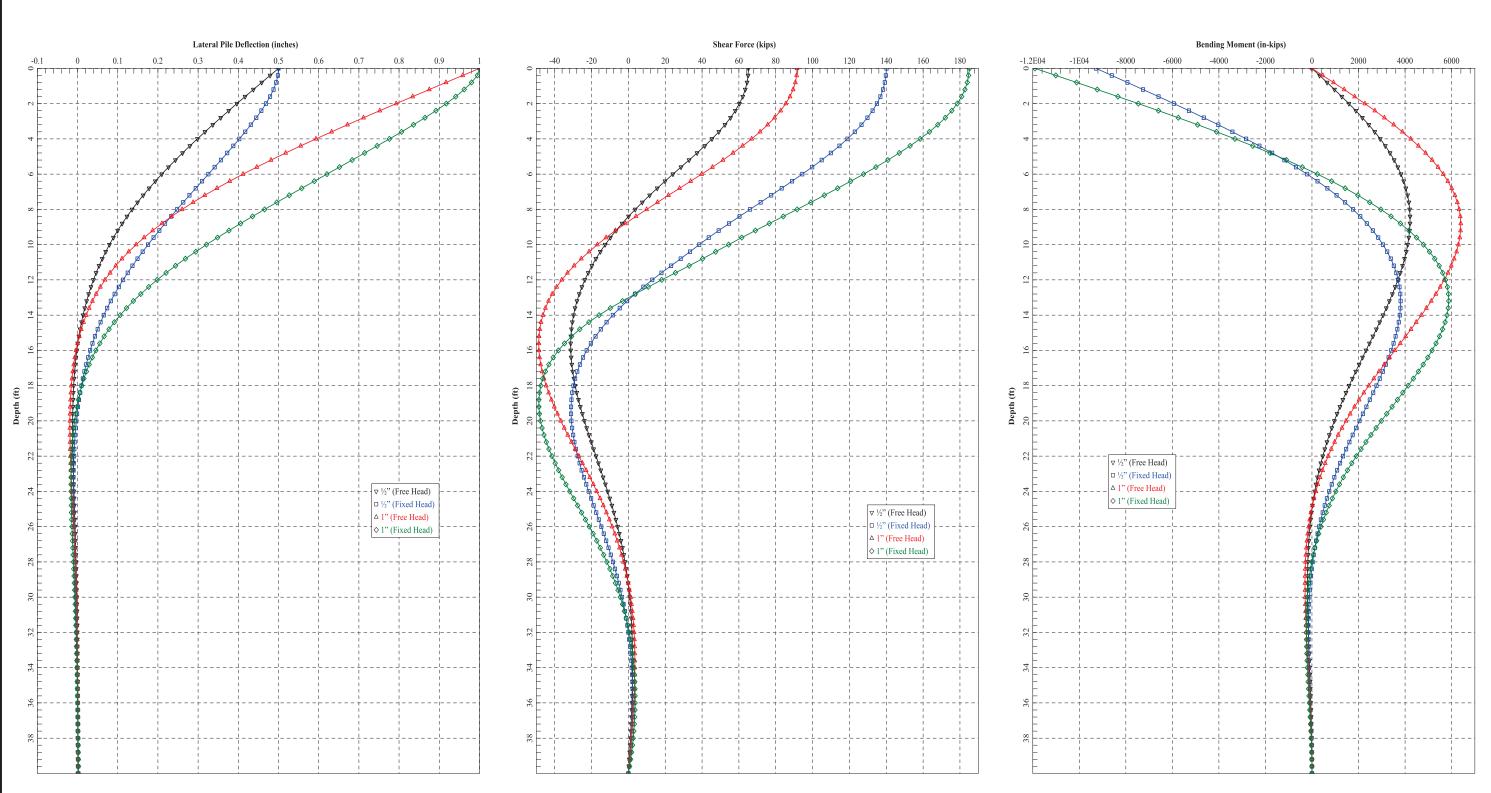
PROJECT NAME
Riverside Community Hospital
HCA Design and Construction

PROJECT NUMBER
SD809

DOCUMENT NUMBER
24-0011

FIGURE NUMBER

LATERAL PILE CAPACITY (GARAGE SITE - SOUTH)



- 1) The approximate lateral response of a single 30-inch diameter, 40-foot long pile is shown in the deflection, shear and moment diagrams provided above.
- 2) The analyses assume at least 10-foot embedment into dense Old Alluvium, with 4,000 psi concrete strength and (6) No. 14 transverse bars (Grade 60).

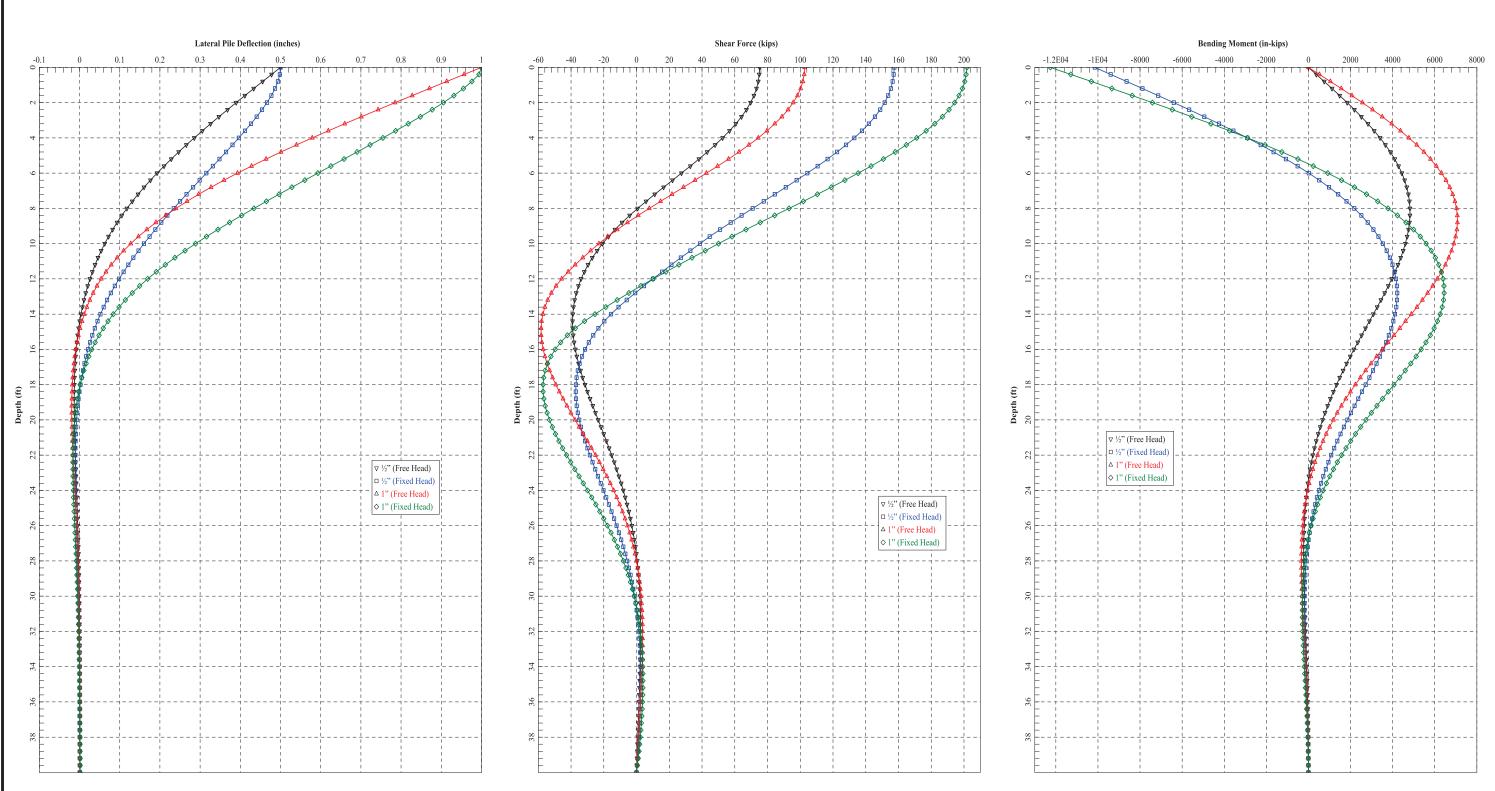


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HCA Design and Construction

PROJECT NUMBER
SD809
DOCUMENT NUMBER
24-0011
FIGURE NUMBER

LATERAL PILE CAPACITY (TOWER SITE - WEST)



- 1) The approximate lateral response of a single 30-inch diameter, 40-foot long pile is shown in the deflection, shear and moment diagrams provided above.
- 2) These analyses assume the eastern Tower piles are completely embedded within dense Old Alluvium, with a pile cut-off below Elevation 810 feet (MSL).



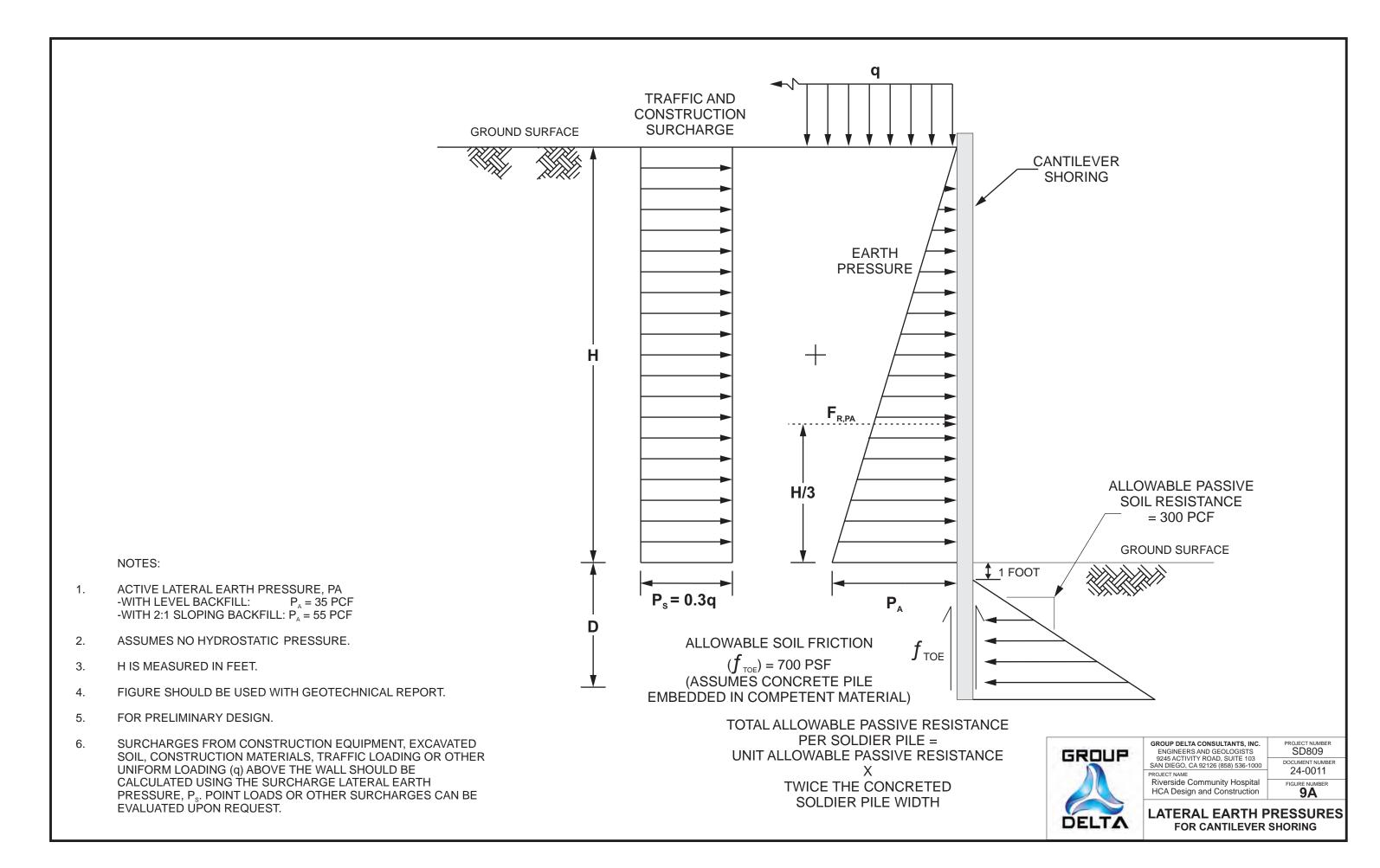
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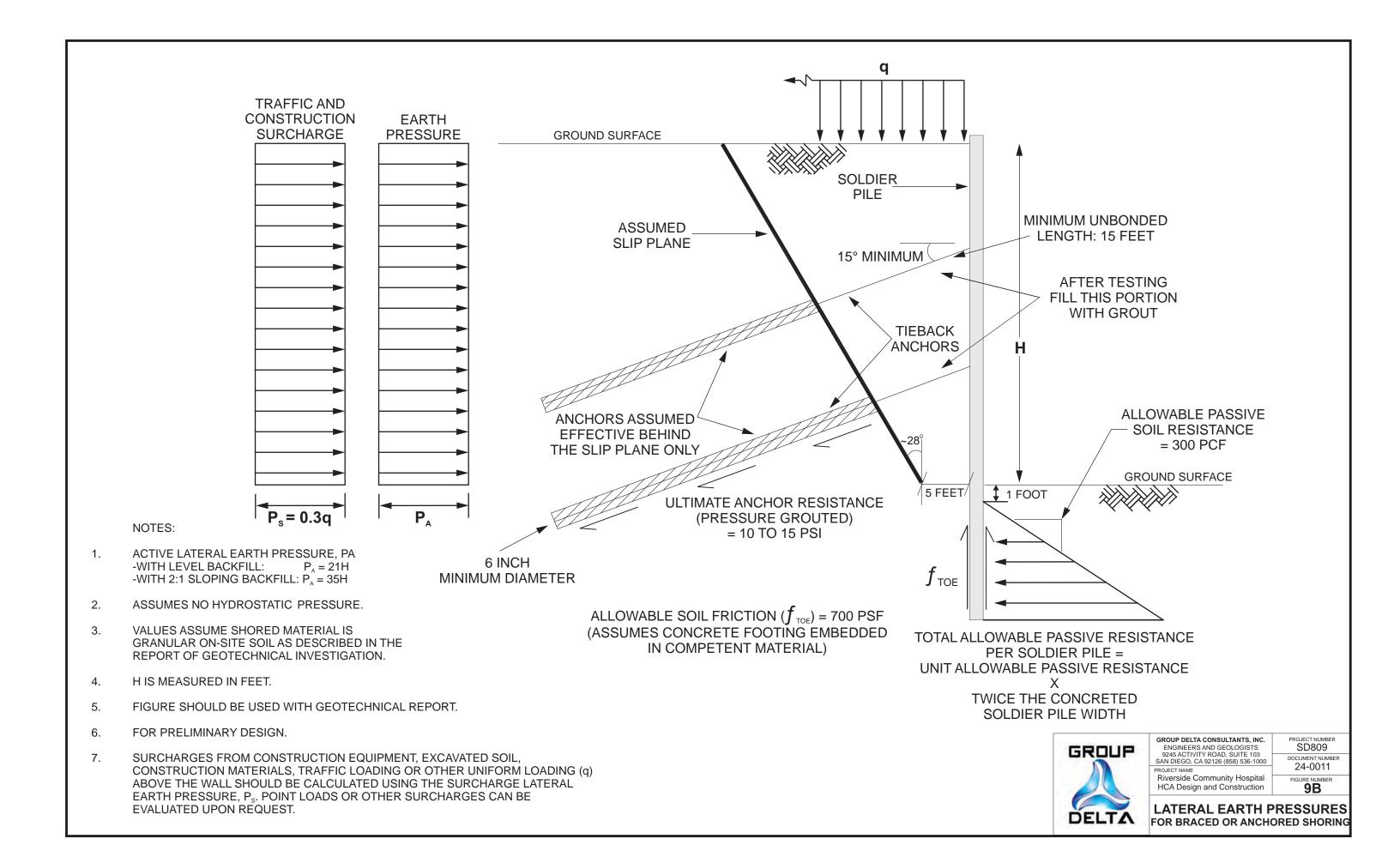
PROJECT NAME
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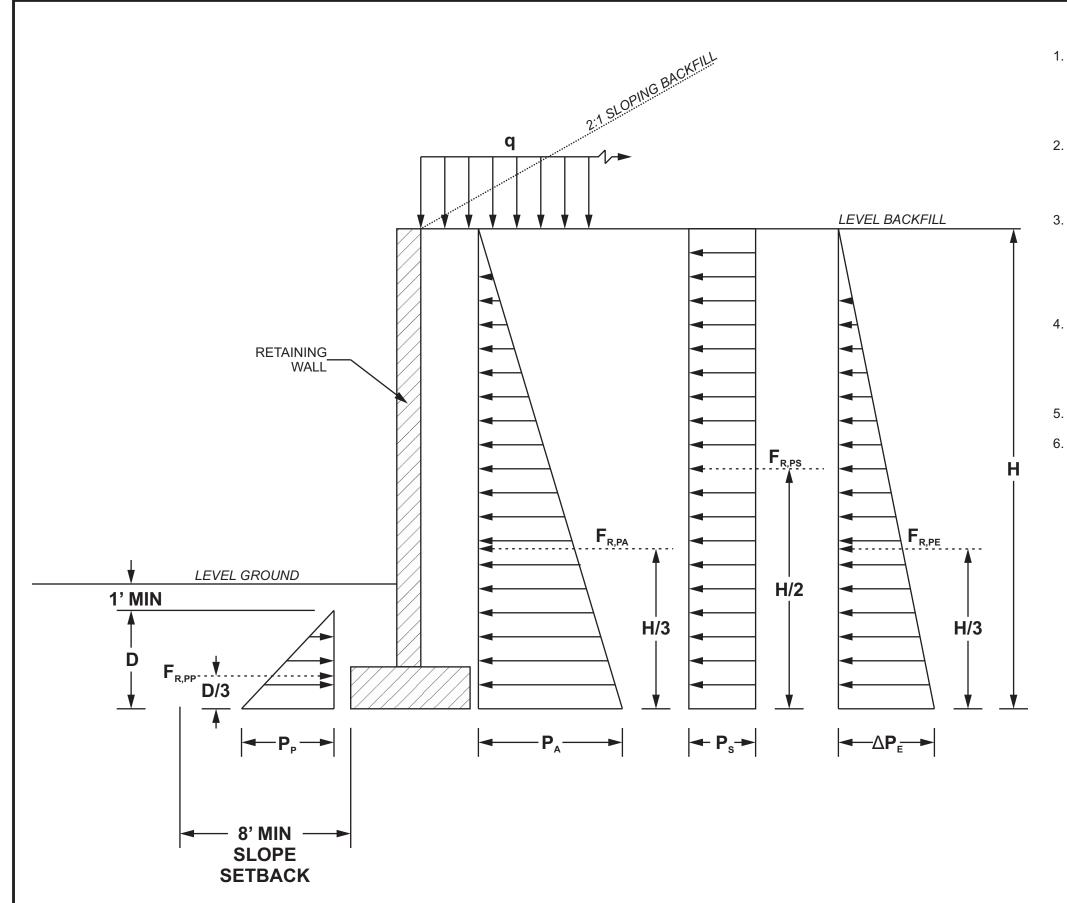
PROJECT NUMBER SD809

DOCUMENT NUMBER 24-0011

LATERAL PILE CAPACITY (TOWER SITE - EAST)







- 1. PASSIVE PRESSURES MAY BE INCREASED BY ⅓
 DURING SEISMIC LOADING. THE UPPER 12 INCHES
 OF MATERIAL NOT PROTECTED BY CONCRETE SLABS
 OR PAVEMENTS SHOULD NOT BE INCLUDED IN THE
 ESTIMATION OF PASSIVE RESISTANCE.
- 2. ASSUMES NO HYDROSTATIC PRESSURE. A WALL BACK DRAIN SHOULD BE INSTALLED AS RECOMMENDED IN THE WALL DRAINAGE DETAIL FIGURE.
- 3. SURCHARGES FROM CONSTRUCTION EQUIPMENT, EXCAVATED SOIL, TRAFFIC LOADING OR OTHER UNIFORM LOADING ABOVE THE WALL SHOULD BE CALCULATED USING THE SURCHARGE LATERAL EARTH PRESSURE, $P_{\rm S}$. POINT LOADS OR OTHER SURCHARGES CAN BE EVALUATED UPON REQUEST.
- 4. SEISMIC INCREMENT LATERAL EARTH PRESSURE $(\Delta P_{\rm E})$ IS BASED ON A DESIGN-LEVEL PEAK GROUND ACCELERATION OF 0.41g. SEISMIC INCREMENT SHOULD BE APPLIED TO WALLS SIX FEET OR GREATER IN HEIGHT.
- 5. 'H'AND 'D'ARE MEASURED IN FEET.
- 6. PRESSURES ASSUME GRANULAR AND
 NON-EXPANSIVE SOIL MATERIALS COMPACTED AS
 RECOMMENDED IN THE GEOTECHNICAL REPORT.

LATERAL EARTH PRESSURES

LATERAL EARTH PRESSURE TYPE	EQUIVALENT FLUID PRESSURE (PCF)	
ACTIVE, P _A	LEVEL BACKFILL	2:1 SLOPING BACKFILL
COMPACTED FILL	35	55
SEISMIC INCREMENT, ΔP _E *	23	
PASSIVE, P,**	300	
SURCHARGE, P _s		0.3q

*SEISMIC PRESSURE, $P_{AE} = P_A + \Delta P_E$

**PASSIVE RESISTANCE VERSUS DISPLACEMENT CURVES CAN BE PROVIDED UPON REQUEST.

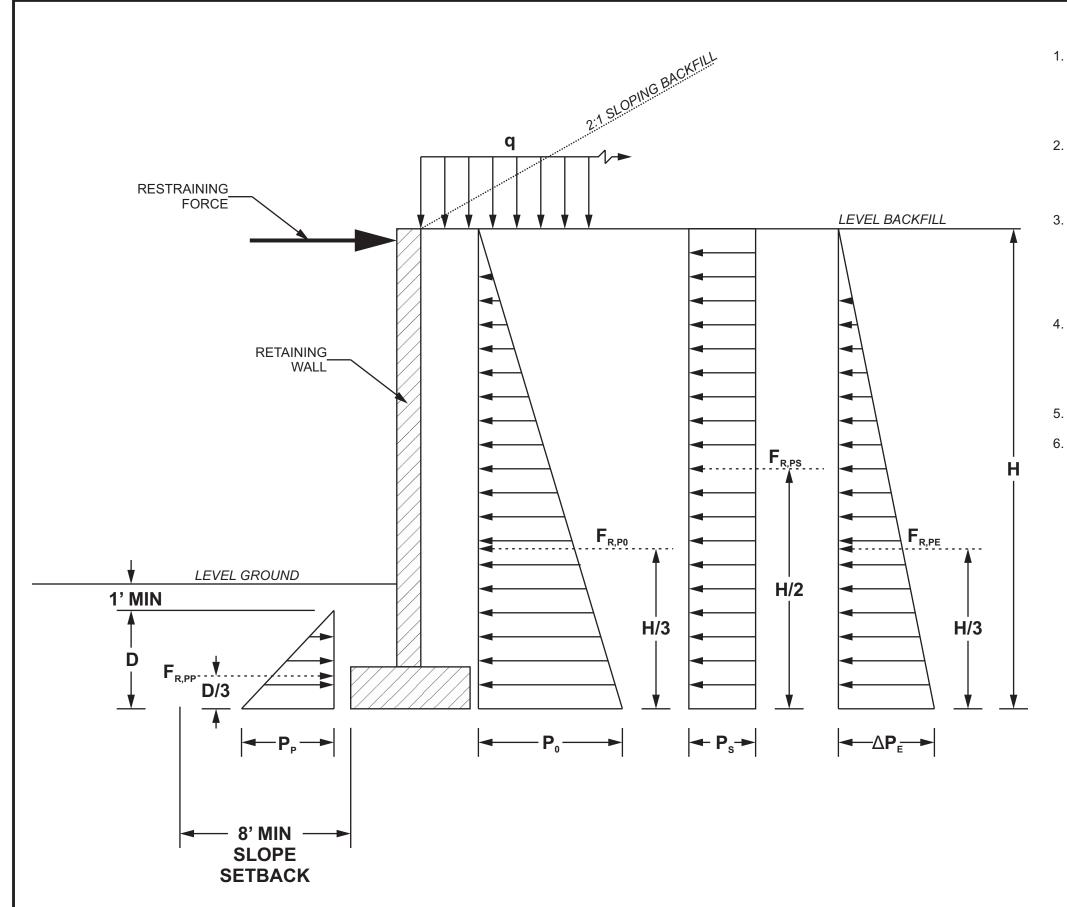


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DOCUMENT NUMBER
24-0011
FIGURE NUMBER

LATERAL EARTH PRESSURES FOR YIELDING RETAINING WALLS



- 1. PASSIVE PRESSURES MAY BE INCREASED BY ⅓
 DURING SEISMIC LOADING. THE UPPER 12 INCHES
 OF MATERIAL NOT PROTECTED BY CONCRETE SLABS
 OR PAVEMENTS SHOULD NOT BE INCLUDED IN THE
 ESTIMATION OF PASSIVE RESISTANCE.
- 2. ASSUMES NO HYDROSTATIC PRESSURE. A WALL BACK DRAIN SHOULD BE INSTALLED AS RECOMMENDED IN THE WALL DRAINAGE DETAIL FIGURE.
- 3. SURCHARGES FROM CONSTRUCTION EQUIPMENT, EXCAVATED SOIL, TRAFFIC LOADING OR OTHER UNIFORM LOADING ABOVE THE WALL SHOULD BE CALCULATED USING THE SURCHARGE LATERAL EARTH PRESSURE, $P_{\rm S}$. POINT LOADS OR OTHER SURCHARGES CAN BE EVALUATED UPON REQUEST.
- 4. SEISMIC INCREMENT LATERAL EARTH PRESSURE $(\Delta P_{\rm E})$ IS BASED ON A DESIGN-LEVEL PEAK GROUND ACCELERATION OF 0.41g. SEISMIC INCREMENT SHOULD BE APPLIED TO WALLS SIX FEET OR GREATER IN HEIGHT.
- 5. 'H'AND 'D'ARE MEASURED IN FEET.
- 6. PRESSURES ASSUME GRANULAR AND NON-EXPANSIVE SOIL MATERIALS COMPACTED AS RECOMMENDED IN THE GEOTECHNICAL REPORT.

LATERAL EARTH PRESSURES

LATERAL EARTH PRESSURE TYPE	EQUIVALENT FLUID PRESSURE (PCF)	
AT-REST, P _o	LEVEL BACKFILL	2:1 SLOPING BACKFILL
COMPACTED FILL	60	90
SEISMIC INCREMENT, ΔP _E *	(SEE FIGURE 9C)	
PASSIVE, Pp**	300	
SURCHARGE, P _s		0.5q

*SEISMIC PRESSURE, $P_{AE} = P_A + \Delta P_E$ (SEE FIGURE 9C)
**PASSIVE RESISTANCE VERSUS DISPLACEMENT CURVES CAN BE PROVIDED UPON REQUEST.



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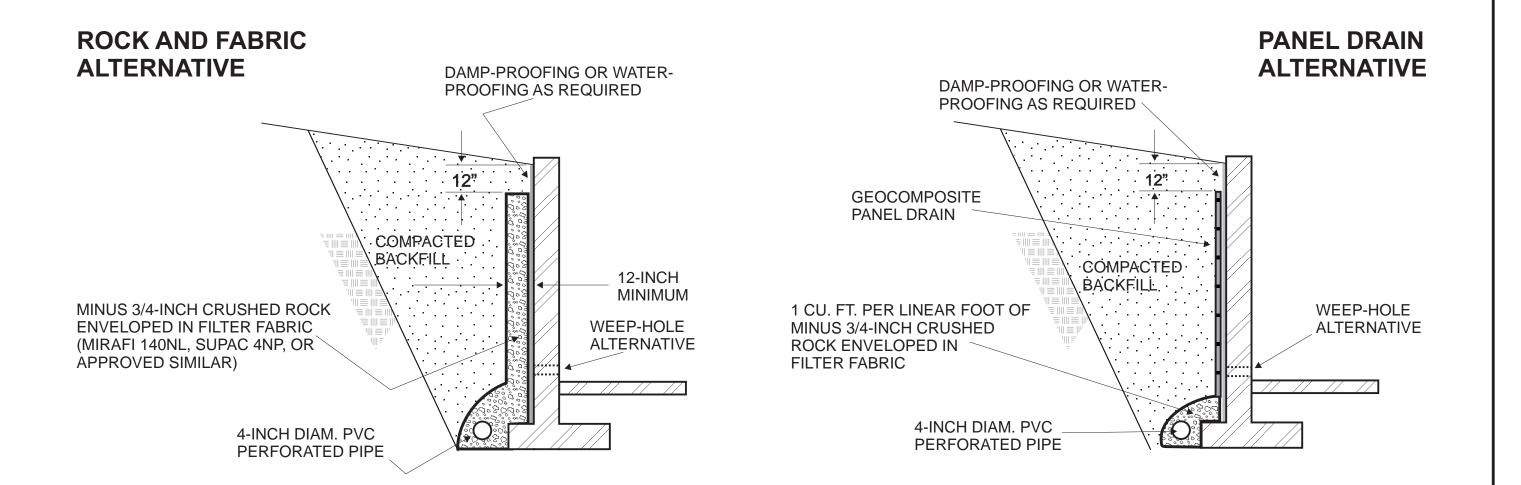
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DOCUMENT NUMBER
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FIGURE NUMBER

LATERAL EARTH PRESSURES FOR BRACED RETAINING WALLS



- 1) Perforated pipe should outlet through a solid pipe to a free gravity outfall. Perforated pipe and outlet pipe should have a fall of at least 1%.
- 2) As an alternative to the perforated pipe and outlet, weep-holes may be constructed. Weep-holes should be at least 2 inches in diameter, spaced no greater than 8 feet, and be located just above grade at the bottom of wall.
- 3) Filter fabric should consist of Mirafi 140N, Supac 5NP, Amoco 4599, or similar approved fabric. Filter fabric should be overlapped at least 6-inches.
- 4) Geocomposite panel drain should consist of Miradrain 6000, J-DRain 400, Supac DS-15, or approved similar product.



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FIGURE NUMBER

WALL DRAINAGE DETAILS

APPENDIX A FIELD EXPLORATION

APPENDIX A

FIELD EXPLORATION

The field exploration program included a geologic reconnaissance of the site, the advancement of 5 cone penetrometer test (CPT) soundings, the drilling of 10 exploratory borings, and borehole percolation testing at 12 locations. The subsurface explorations and field infiltration tests were completed between March 22nd and April 5th, 2024. The CPT soundings were advanced by Kehoe Testing and Engineering. The exploratory borings were advanced by Yellow Jacket Drilling. The infiltration tests were completed by Group Delta personnel using a hand auger. The exploration locations are shown on the Exploration Plans, Figures 3A to 3C. The CPT data and interpreted soil profiles are presented in Figures A-1 to A-5. Boring Records are provided in Figures A-6 to A-15. The field infiltration test results are discussed in the text of this report and presented in detail in the figures of Appendix D. The subsurface explorations are summarized in the table below.

Exploration ID	Exploration Date	Latitude	Longitude	Ground Surface Elevation [FT]	Exploration Depth [FT]	Figure No.
CPT-1	03/22/24	33.977986°	-117.382803°	800	34.0	A-1
CPT-2	03/22/24	33.977770°	-117.383393°	796	37.5	A-2
CPT-3	03/22/24	33.977144°	-117.383833°	793	20.7	A-3
CPT-4	03/22/24	33.976485°	-117.383034°	794	25.0	A-4
CPT-5	03/22/24	33.976042°	-117.381919°	839	54.7	A-5
			I		1	
B-01	04/04/24	33.978130°	-117.383124°	800	36½	A-6
B-02	04/01/24	33.977907°	-117.383271°	798	51½	A-7
B-03	04/02/24	33.977754°	-117.382979°	798	36½	A-8
B-04	04/04/24	33.977536°	-117.383915°	794	31	A-9
B-05	04/04/24	33.977440°	-117.383294°	796	31½	A-10
B-06	04/03/24	33.977308°	-117.384145°	794	31½	A-11
B-07	04/04/24	33.976992°	-117.383266°	794	28	A-12
B-08	04/01/24	33.976503°	-117.383073°	794	30	A-13
B-09	04/05/24	33.976063°	-117.382178°	832	31½	A-14
B-10	04/02/24	33.976190°	-117.381820°	838	51½	A-15

The 10 CPT soundings were advanced by Kehoe Testing and Engineering in general accordance with ASTM D5778 using a 30-ton truck mounted rig with a 15 cm² cone. Integrated electronic circuitry was used to measure the tip resistance (Qc) and skin friction (Fs) at one-inch intervals while the CPT was advanced into the soil using hydraulic down pressure. Note that each of the CPT soundings was pushed to practical refusal, as indicated by a CPT tip resistance in excess of 700 tons per square foot (TSF). A piezometer located behind the cone tip also measured transient pore pressure (u). The CPT data was used to characterize the soil profile are based on normalized cone resistance and friction ratio interpretations (Robertson, 2010). The first figure for each CPT sounding presents both the raw CPT data and the interpreted soil profile (Figures A-1a to A-5a). The raw CPT data is also shown in more detail in the following Figures A-1b to A-5b for each CPT sounding.



APPENDIX A

FIELD EXPLORATION (Continued)

At the location of soundings CPT-1, CPT-2 and CPT-5, shear wave velocity measurements were collected at 5-foot depth intervals using an air actuated hammer located inside the front jack of the rig. The interval shear wave data measured in each of these three CPT soundings is attached immediately after the interpreted soil parameters. The average shear wave velocity measured within the upper 34 to 35 feet (Vs_d) at the location of CPT-1 and CPT-2 was 876 to 899 ft/s. If we assume a uniform shear wave velocity of 1,940 ft/s for the Old Alluvium below that depth based on the lower bound of the shear wave velocity measurements conducted by others, the average shear wave velocity for the upper 100 feet of the soil profile (Vs₃₀) for the Garage site would be about 1,390 ft/s (CHJ, 2007). This correlates to a 2022 CBC Site Class C for the proposed Garage site.

The average shear wave velocity measured within the upper 55 feet (V_{Sd}) of the compacted fill and Old Alluvium along the east end of Tower Site in CPT-5 was 1,390 ft/s. If we again assume a shear wave velocity of 1,940 ft/s for the Old Alluvium below that depth, the average shear wave velocity for the upper 100 feet of the soil profile (V_{S30}) at the east end of the Tower site is estimated at 1,600 ft/s (CHJ, 2007). This also corresponds to a 2022 CBC Site Class C for the proposed Tower site.

The exploratory borings were advanced by Yellow Jacket Drilling using their CME 75 limited access track mounted rig, and their CME 85 truck mounted rig. Disturbed soil samples were collected from the exploratory borings using a 2-inch outside diameter Standard Penetration Test (SPT) sampler. Less disturbed samples were collected using a 3-inch outside diameter ring lined sampler (a modified California sampler). Automatic hammers with calibrated Energy Transfer Ratios (ETR) ranging from approximately 74 to 80 percent were used to collect all of the drive samples. For each sample, the number of blows needed to drive the sampler 12 inches was recorded on the logs. The field blow counts (N) were normalized to approximate a standard 60 percent ETR as shown on the logs (N_{60}). Bulk samples were also collected from the explorations at selected intervals.

The field exploration locations were determined by visually estimating, pacing and taping distances from landmarks shown on the Exploration Plans, Figures 3A to 3C. The locations shown should not be considered more accurate than is implied by the method of measurement used and the scale of the map. The lines designating the interface between differing soil materials on the logs may be abrupt or gradational. Further, soil conditions at locations between the excavations may be substantially different from those at the specific locations we explored. It should be noted that the passage of time may also result in changes in the soil conditions reported in the logs.

Logs for several previous field explorations completed by others are also included in Appendix A1 for reference. These logs include Borings B-1, B-2, B-6, B-7, B-8 and B-9 as well as CPT-1 to CPT-6 at the proposed Tower site (CHJ, 2008). Borings B-1 and B-2 are also included from a previous investigation for the MOB located along the southern edge of the Garage site (CHJ, 2012). The approximate locations of the relevant previous explorations are shown in Figures 3B and 3C.



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SOIL IDENTIFICATION AND DESCRIPTION SEQUENCE

Sequence			er to tion	ired	nal
Sedu	Identification Components	Field	Lab	Required	Optic
1	Group Name	2.5.2	3.2.2	•	0
2	Group Symbol	2.5.2	3.2.2	•	
	Description Components				
3	Consistency of Cohesive Soil	2.5.3	3.2.3	•	
4	Apparent Density of Cohesionless Soil	2.5.4		•	
5	Color	2.5.5		•	
6	Moisture	2.5.6		•	
	Percent of Proportion of Soil	2.5.7	3.2.4	•	•
7	Particle Size	2.5.8	2.5.8	•	•
	Particle Angularity	2.5.9			0
	Particle Shape	2.5.10			0
8	Plasticity (for fine-grained soil)	2.5.11	3.2.5		0
9	Dry Strength (for fine-grained soil)	2.5.12			0
10	Dilatency (for fine-grained soil)	2.5.13			0
11	Toughness (for fine-grained soil)	2.5.14			0
12	Structure	2.5.15			0
13	Cementation	2.5.16		•	
14	Percent of Cobbles and Boulders	2.5.17		•	
14	Description of Cobbles and Boulders	2.5.18		•	
15	Consistency Field Test Result	2.5.3		•	
16	Additional Comments	2.5.19			0

Minimum Required Sequence:

USCS Group Name (Group Symbol); Consistency or Density; Color; Moisture; Percent of Proportion of Soil; Particle Size; Plasticity (optional).

= optional for non-Caltrans projects

Where applicable:

Cementation; % cobbles & boulders; Description of cobbles & boulders; Consistency field test result

EXPLORATION IDENTIFICATION

Explorations are identified using the following convention

H-YY-NNN

Where:

H: Exploration type code

YY: 2-digit year (where utilized)

NNN: Exploration number

Hole Type Code and Description

Hole Type Code	Description
А	Auger Boring (Hollow or solid stem bucket)
BA	Bucket Auger
CPT	Cone Penetration Test
D	Driven (dynamic cone penetrometer)
НА	Hand Auger
HD	Hand driven (1-inch soil tube)
0	Other (note on LOTB)
Р	Rotary Percussion Boring (Alr)
R	Rotary drilled boring (Conventional)
1 107	Rotary core (self-cased wire-line, continuosly sampled)
1 B/W	Rotary cored (self cased wire-line, not continuosly sampled)
TP	Test Pit

Description Sequence Examples:

SANDY lean CLAY (CL); very stiff; yellowish brown; moist; mostly fines; some SAND, from fine to medium; few gravels; medium plasticity; PP=2,75.

Well-graded SAND with SILT and GRAVEL and COBBLES (SW-SM); dense; brown; moist; mostly SAND, from fine to coarse; some fine GRAVEL; few fines; weak cementation; 10% GRANITE COBBLES; 3 to 6 inches; hard, subrounded.

Clayey SAND (SC) medium dense, light brown; wet; mostly fine sand; little fines; low plasticity.



EXPLORATION RECORD LEGEND #1

REFERENCE: Caltrans Soil and Rock Logging, Classification, and Presentation Manual (2010).

С	Consolidation (ASTM D 2435)
CL	Collapse Potential (ASTM D 4546)
СР	Compaction Curve (CTM 216)
CR	Corrosion, Sulfates, Chlorides (CTM 643; CTM 417 CTM 422)
CU	Consolidated Undrained Triaxial (ASTM D 4767)
DS	Direct Shear (ASTM D 3080)
EI	Expansion Index (ASTM D 4829)
М	Moisture Content (ASTM D 2216)
OC	Organic Content (ASTM D 2974)
Р	Permeability (CTM 220)
PA	Particle Size Analysis (ASTM D 6913, ASTM D 7928)
Pl	Liquid Limit, Plastic Limit, Plasticity Index (AASHTO T 89, AASHTO T 90)
PL	Point Load Index (ASTM D 5731)
PM	Pressure Meter
R	R-Value (CTM 301)
SE	Sand Equivalent (CTM 217)
SG	Specific Gravity (AASHTO T 100)
SL	Shrinkage Limit (ASTM D 427)
SW	Swell Potential (ASTM D 4546)
UC	Unconfined Compression - Soil (ASTM D 2166) Unconfined Compression - Rock (ASTM D 2938)
UU	Unconsolidated Undrained Triaxial (ASTM D 2850)
UW	Unit Weight (ASTM D 2937)
WA	Percent passing the No. 200 Sieve (ASTM D 1140)

	SAMPLER GRAI	PHIC SYMBOLS
	Standard Penetration Te	est (SPT)
X	Modified California Sam	pler (2.4" ID, 3" OD)
	Shelby Tube	Piston Sampler
	NX Rock Core	HQ Rock Core
***	Bulk Sampler	Other (see remarks)

	DRILLING M	1ETHOD SYMBOLS	
Auger Drilling	Rotary Drilling	Dynamic Cone or Hand Driven	Diamond Core

Term	Definition	Symbol
Unit Change	Change in geoilogic unit	
Material Change Within Unit	Change of soil classification within geologic unit	

WATER LEVEL SYMBOLS

▼ Static Water Level Reading

REFERENCE: Caltrans Soil and Rock Logging, Classification, and Presentation Manual (2010).



APPARENT D	APPARENT DENSITY OF COHESIONLESS SOILS		
Description	SPT N ₆₀ (blows / 12 inches)		
Very Loose	0 - 4		
Loose	5 - 9		
Medium Dense	10 - 29		
Dense	30 -50		
Very Dense	Greater than 50		

PERCENT OR PROPORTION OF SOILS	
Description	Criteria
Trace	Particles are present but estimated to be less than 5%
Few	5 - 10%
Little	15 - 25%
Some	30 - 45%
Mostly	50 - 100%

CEMENTATION	
Description	Criteria
Weak	Crumbles or breaks with handling or little finger pressure
Moderate	Crumbles or breaks with considerable finger pressure
Strong	Will not crumble or break with finger pressure

MOISTURE		
Description	Criteria	
Dry	No discernable moisture	
Moist	Moisture present but no free water	
Wet	Visible free water	

CONSISTENCY OF COHESIVE SOILS*			
Description	Description SPT N ₆₀ (blows / 12 inches)		
Very Soft	0 - 1		
Soft	2 - 3		
Medium Stiff	4 - 7		
Stiff	8 - 14		
Very Stiff	15 - 30		
Hard	Greater than 30		

 Ref: Peck, Hansen, and Thornburn, 1974, "Foundation Engineering," Second Edition.

Note: Only to be used (with caution) when pocket penetrometer or other data on undrained shear strength are unavailable.

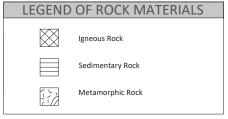
Not allowed by Caltrans Soil and Rock Logging and Classification Manual, 2010.

PARTICLE SIZE		
Description		Size (in)
Boulder		Greater than 12
Cobble		3 - 12
Gravel	Coarse	3/4 - 3
	Fine	1/5 - 3/4
	Coarse	1/16 -1/5
Sand	Medium	1/64 - 1/16
	Fine	1/300 - 1/64
Silt and Clay		Less than 1/300

PLASTICITY		
Description	Criteria	
Nonplastic	A 1/8-in. thread cannot be rolled at any water content	
Low	The thread can barely be rolled and the lump cannot be formed when drier than the plastic limit	
Medium	The thread is easy to roll and not much time is required to reach the plastic limit. The thread cannot be rerolled after reaching the plastic limit. The lump crumbles when drier than the plastic limit	
High	It takes considerable time rolling and kneading to reach the plastic limit. The thread can be rerolled several times after reaching the plastic limit. The lump can be formed without crumbling when drier than the plastic limit.	

REFERENCE: Caltrans Soil and Rock Logging, Classification, and Presentation Manual (2010), with the exception of consistency of cohesive soils vs. N_{60} .





BEDDING SPACING			
Description	Thickness / Spacing		
Massive	Greater than 10ft		
Very Thickly Bedded	3 ft - 10 ft		
Thickly Bedded	1 ft - 3 ft		
Moderately Bedded	4 in - 1 ft		
Thinly Bedded	1 in - 4 in		
Very Thinly Bedded	1/4 in - 1 in		
Laminated	Less than 1/4 in		

	WEATHERING DESCRIPTORS FOR INTACT ROCK					
	Diagnostic Features					
Description	Chemical Weathering - Discolo	oration - Oxidation	Mechanical Weathering and Grain Boundary Conditions	Texture	and Leaching	General Characteristics
Description	Body of Rock	Fracture Surfaces		Texture	Leaching	General Characteristics
Fresh	No discoloration, no oxidized	No discoloration or oxidation	No separation, intact (tight)	No change	No leaching	Hammer rings when crystalline rocks are struck
Slightly Weathered	Discoloration or oxidation is limited to surface of, or short distance from, fractures; some feldspar crystals are dull	Minor to complete discoloration or oxidation of most surfaces	No visible separation, intact (tight)	Preserved	Minor leaching of some soluble minerals	Hammer does not ring when rock is struck. Body of rock not weakened
Moderately Weathered	Discoloration or oxidation extends from fractures usually throughout; Fe-Mg minerals are "rusty"; feldspar crystals are "cloudy"	All fracture surfaces are discolored or oxidized	Partial separation of boundaries visible	Generally Preserved	Soluble minerals may be mostly leached	Hammer does not ring when rock is struck. Body of rock is slightly weakened
Intensely Weathered	Discoloration or oxidation throughout; all feldspars and Fe-Mg minerals are altered to clay to some extent; or chemical alteration produces in situ disaggregation, grain boundary conditions	All fracture surfaces are discolored or oxidized; surfaces friable	Partial separation, rock is friablr; in semi-arid conditions, granitics are disaggregated	Texture altered by chemical disintegration (hydration, argillation)	Leaching of soluble minerals may be complete	Dull sound when struck with hammer; usually can be broken with moderate to heavy manual pressure or by light hammer blow without reference to planes of weakness such as incipient or hairline fractures of veinlets. Rock is significantly weakened.
Decomposed	Discolored of oxidized throughout, but resistant minerals such as quartz may be unaltered; all feldspars and Fe-Mg minerals are completely altered to clay		Complete separation of grain boundaries (disaggregated)		ning of soluble	Can be granulated by hand. Resistant minerals such as quartz may be present as "stringers" or "dikes"

PERCENT CORE RECOVERY (RI	EC)
Σ LENGTH OF THE RECOVERED CORE PIECES (IN.) TOTAL LENGTH OF CORE RUN (IN.)	_ x 100

PERCENT CORE RECOVERY (REC) \$\sum_{\text{LENGTH OF THE INTACT CORE PIECES \$\geq 4\$ IN.}}\$ x 100 TOTAL LENGTH OF CORE RUN (IN.) RQD* INDICATES SOUNDNESS CRITERIA NOT MET

ROCK HARDNESS		
Description	Criteria	
Extremely Hard	Cannot be scratched with a pocketknife or sharp pick. Can only be chipped with repeated heavy hammer blows,	
Very Hard	Cannot be scratched with a pocketknife or sharp pick. Breaks with repeated heavy hammer blows.	
Hard	Can be scratched with a pocketknife or sharp pick with difficulty (heavy pressure). Breaks with heavy hammer blows.	
Moderately Hard	Can be scratched with a pocketknife or sharp pick with light or moderate pressure. Breaks with moderate hammer blows.	
Moderately Soft	Can be grooved 1/16 in. deep with a pocketknife or sharp pick with moderate or heavy pressure. Breaks with light hammer blow or heavy manual pressure.	
Soft	Can be grooved or gouged easily with a pocketknife or sharp pick with light pressure, can be scratched with fingernail. Breaks with light moderate manual pressure.	
Very Soft	Can be readily indented, grooved or gouged with fingernail, or carved with a pocketknife. Breaks with light manual pressure.	

FRACTURE DENSITY		
Description Observed Fracture Density		
Unfractured	No fractures	
Very Slightly Fractured	Core lengths greater than 3 ft	
Slightly Fractured	Core lengths mostly from 1 to 3 ft	
Moderately Fractured	Core lengths mostly 4 in. to 1 ft.	
Intensely Fractured	Core lengths mostly from 1 to 4 in.	
Very Intensely Fractured	Mostly chips and fragments.	

REFERENCE: Caltrans Soil and Rock Logging, Classification, and Presentation Manual (2010).



A GROUP DELTA

Group Delta Consultants

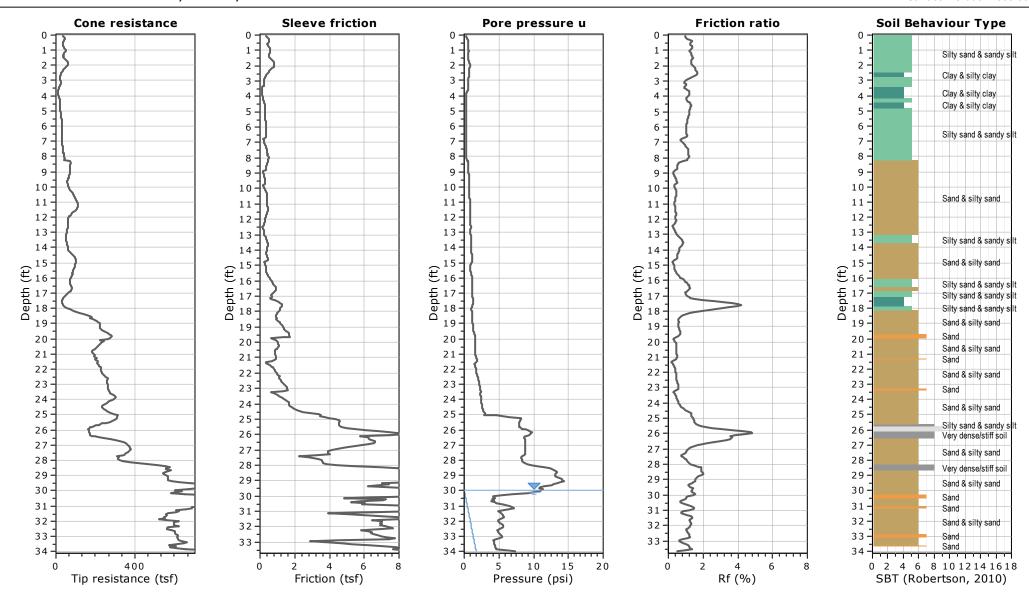
9245 Activity Road, Suite 103 San Diego, California 92126 www.GroupDelta.com

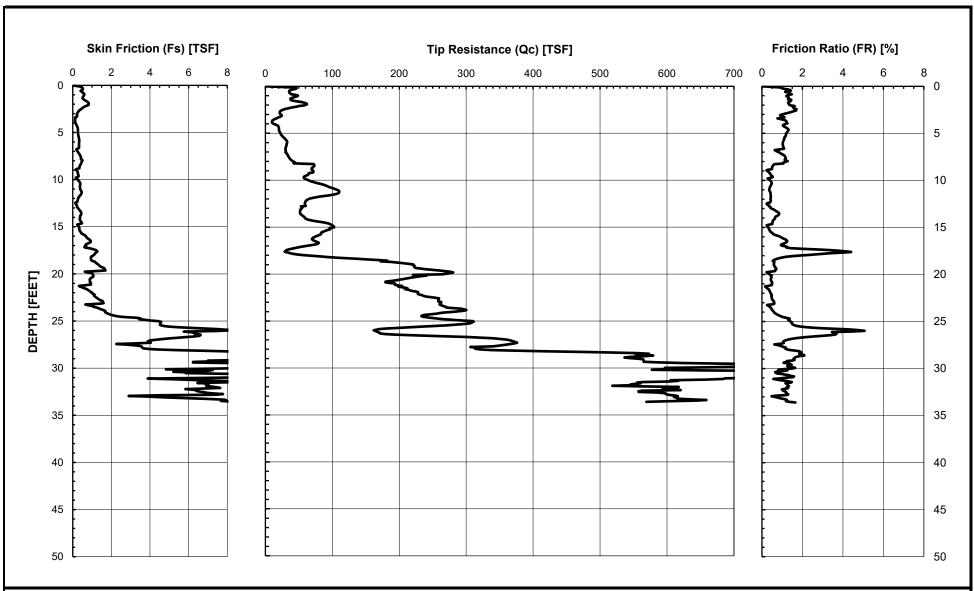
Project: Riverside Community Hospital

Location: 4468 Brockton Avenue, Riverside, California 92501

CPT-1Total depth: 33.99 ft, Date: 3/22/2024

Surface Elevation: 800.00 ft







CONE PENETOMETER DATA (CPT-1)

Document No. 24-0011 Project No. SD809 FIGURE A-1b

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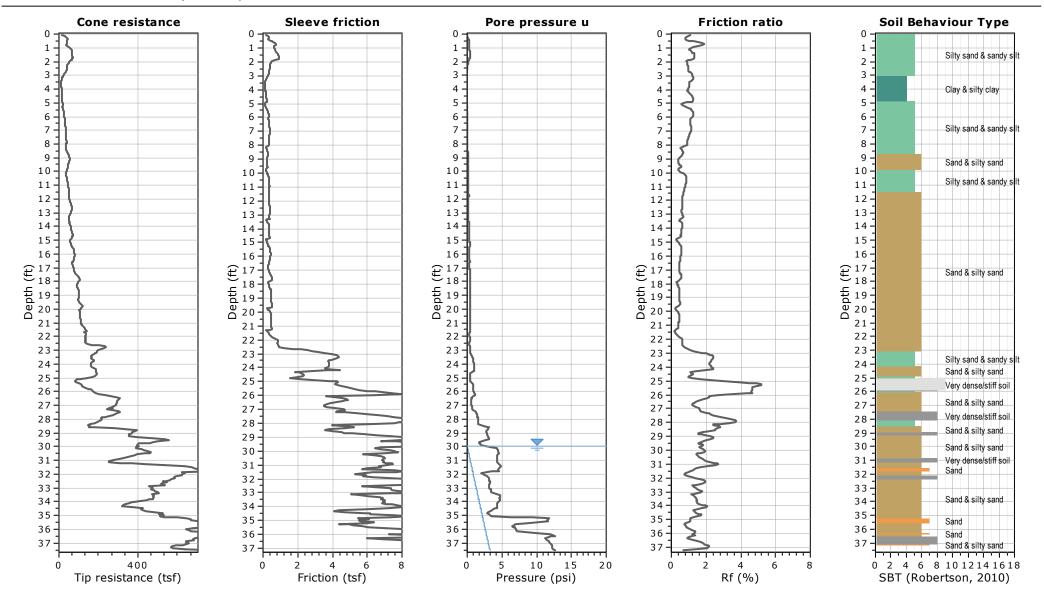
Project: Riverside Community Hospital

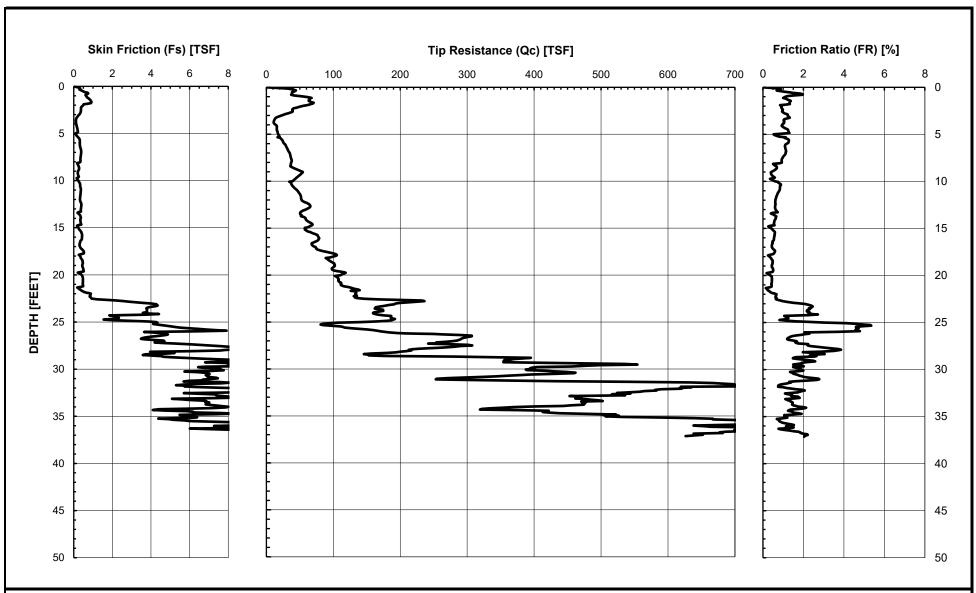
Location: 4468 Brockton Avenue, Riverside, California 92501

Total depth: 37.54 ft, Date: 3/22/2024

Surface Elevation: 796.00 ft

CPT-2







CONE PENETOMETER DATA (CPT-2)

Document No. 24-0011 Project No. SD809 FIGURE A-2b



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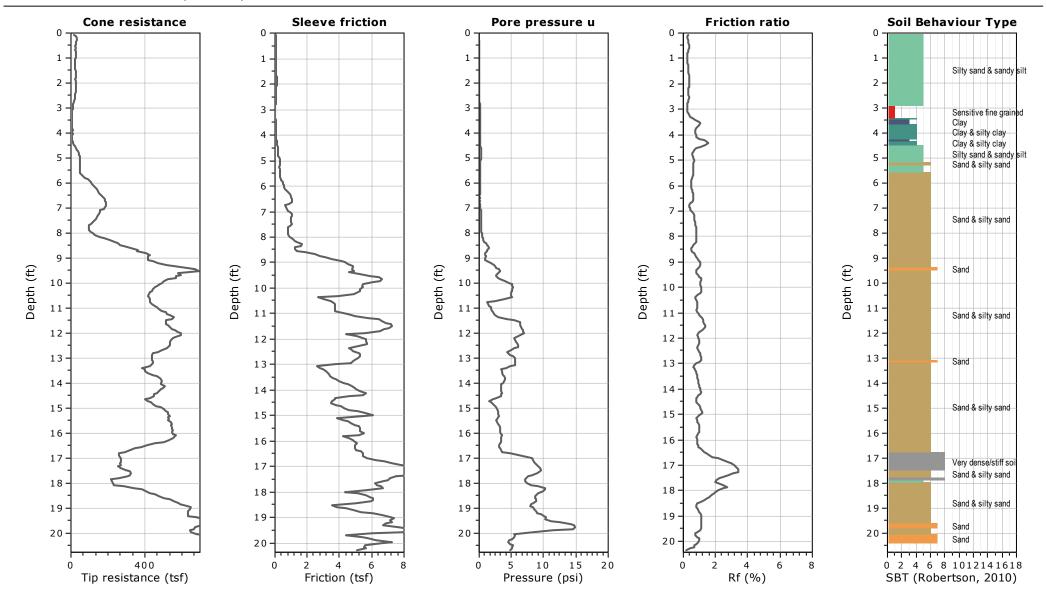
Project: Riverside Community Hospital

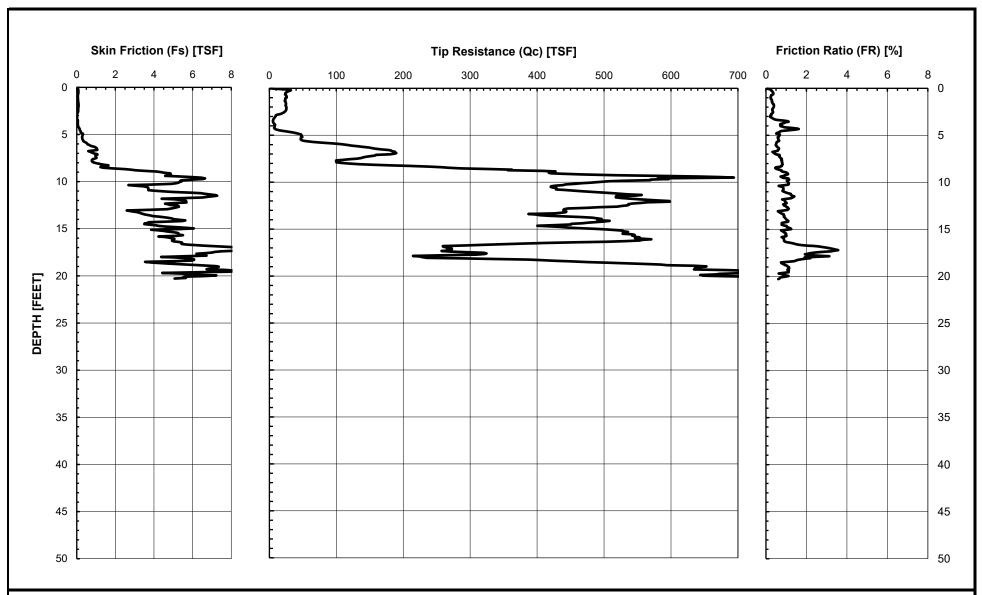
Location: 4468 Brockton Avenue, Riverside, California 92501

Total depth: 20.67 ft, Date: 3/22/2024

Surface Elevation: 793.00 ft

CPT-3







CONE PENETOMETER DATA (CPT-3)

Document No. 24-0011 Project No. SD809 FIGURE A-3b



Group Delta Consultants

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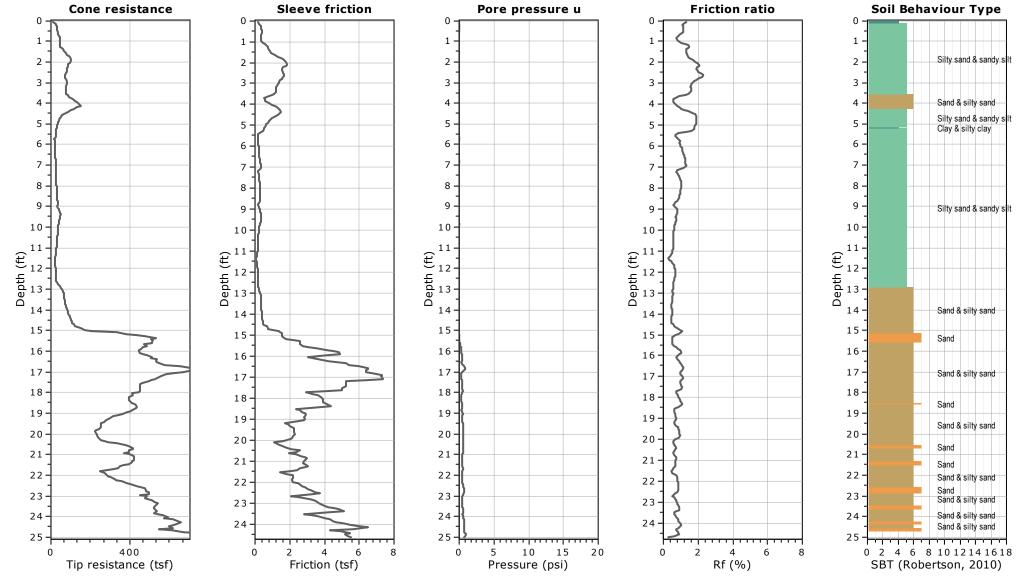
Project: Riverside Community Hospital

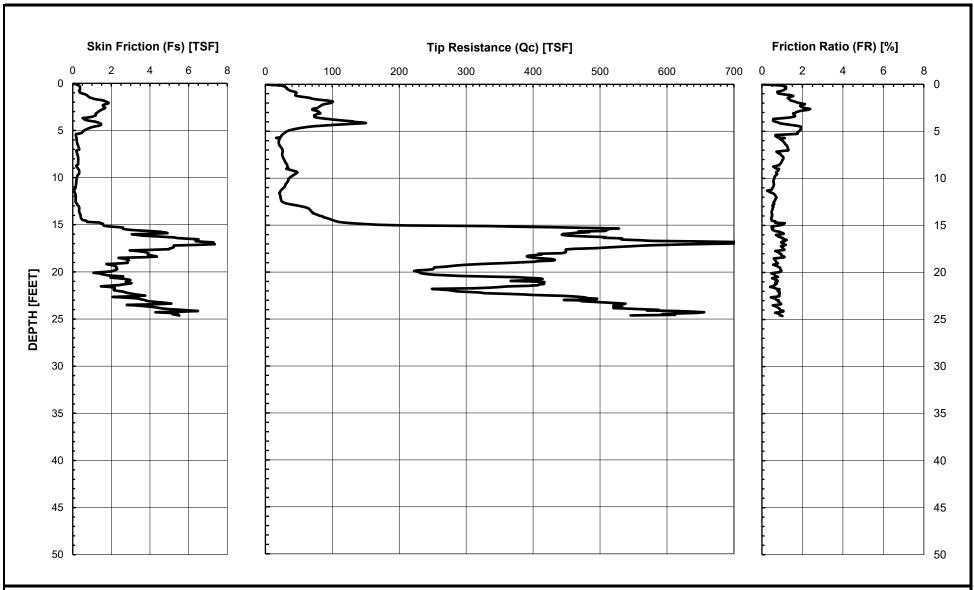
Location: 4468 Brockton Avenue, Riverside, California 92501

CPT-4 Total depth: 25.01 ft, Date: 3/22/2024

Surface Floration: 704 00 ft







CONE PENETOMETER DATA (CPT-4)

Document No. 24-0011 Project No. SD809 FIGURE A-4b



Group Delta Consultants

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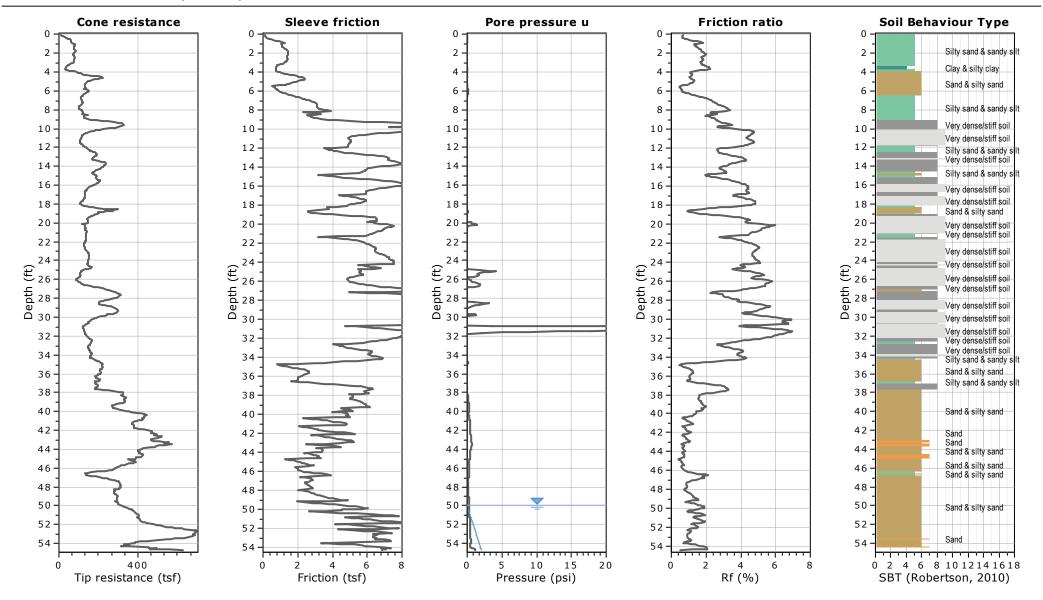
Project: Riverside Community Hospital

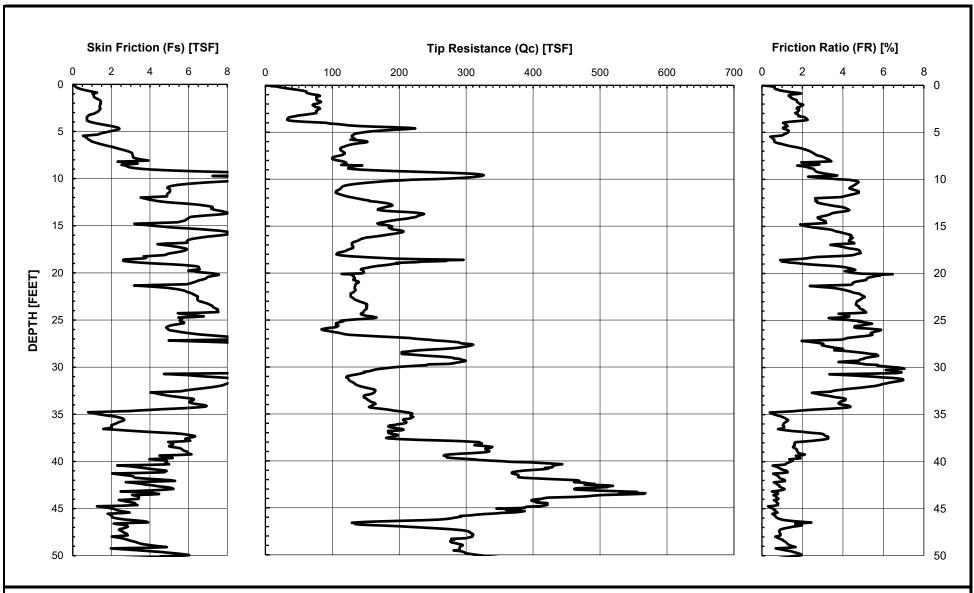
Location: 4468 Brockton Avenue, Riverside, California 92501

Total depth: 54.73 ft, Date: 3/22/2024

Surface Elevation: 839.00 ft

CPT-5





CONE PENETOMETER DATA (CPT-5)

Document No. 24-0011 Project No. SD809 FIGURE A-5b

Group Delta Consultants Riverside Community Hospital Riverside, CA

CPT Shear Wave Measurements

Location CPT-1	Tip Depth (ft) 5.02	Geophone Depth (ft) 4.02	Travel Distance (ft) 4.49	S-Wave Arrival (msec) 5.86	S-Wave Velocity from Surface (ft/sec) 766	Interval S-Wave Velocity (ft/sec)
	10.04	9.04	9.26	13.64	679	613
	15.03 20.01	14.03 19.01	14.17 19.11	20.74 27.28	683 701	692 756
	25.03	24.03	24.11	31.36	769	1225
	30.02	29.02	29.09	35.44	821	1220
	33.92	32.92	32.98	37.44	881	1946
ODT 0	5.00	4.00	4.40	4.04	4050	
CPT-2	5.02 10.01	4.02 9.01	4.49 9.23	4.24 13.50	1059 684	512
	15.03	14.03	9.23 14.17	19.80	716	785
	20.01	19.01	19.11	26.12	732	782
	25.03	24.03	24.11	32.00	754	850
	30.02	29.02	29.09	35.22	826	1545
	35.01	34.01	34.07	37.78	902	1945
CPT-5	5.02	4.02	4.49	2.46	1825	
	10.01	9.01	9.23	5.94	1554	1362
	15.06 20.05	14.06 19.05	14.20 19.15	8.96 13.04	1585 1469	1646 1214
	25.03	24.03	24.11	16.26	1483	1540
	30.02	29.02	29.09	20.44	1423	1190
	35.01	34.01	34.07	24.28	1403	1297
	39.99	38.99	39.04	26.98	1447	1842
	45.01	44.01	44.06	31.46	1400	1119
	50.03	49.03	49.07	35.20	1394	1341
	54.69	53.69	53.73	38.74	1387	1315

Shear Wave Source Offset -

2 ft

S-Wave Velocity from Surface = Travel Distance/S-Wave Arrival Interval S-Wave Velocity = (Travel Dist2-Travel Dist1)/(Time2-Time1)

E	3OR	RIN	G F	RECC	ORD)	PROJE Rivers			unity Ho	ospital			PROJECT SD809		BORING B-01
Prope	OCATION OSED P NG COM	i arkino PANY	g Gar	age and			Tavoro	DRILL	ING M	ETHOD	•	STAF 4/4	гт /2024	FINI 4/ LOGGED	ISH /4/2024	SHEET NO. 1 of 2 CHECKED BY
	w Jack <mark>vg EQU</mark> I								NG DIA	tem Au (in)	•	DEPTH (ft)	GROUN	JWJ D ELEV (ft)	DEPTH/ELE	MAF V. GROUNDWATER (
	75 Lim		Acces	s Rig #1	42		NOTES	8			36.5		800		▼ / na	
			., Dro	p: 30 in.	(Auton	natic)	_		%, N ₆	₅₀ ~ 74/0	60 * N ~ 1	1.23 * N				
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	ž	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DESC	CRIPTION /	AND CLASSIF	FICATION
	_		B-1						PA	- -		dark ye mediun	llowish b n SAND;	orown (10' ; some fine	R 4/4): moi:	edium dense; st; mostly fine to AVEL; nonplastic.
_5	795 		S-2	2 4 4	8	10				5 —		dense;	dark yel nedium	lowish bro	wn (10YR 4/	SM); medium /4); moist; mostly ice GRAVEL;
-10	790 	X	R-3	3 5 7	12	10	9.9	107		- 10 — - -		mediun mostly	n dense;	dark yello nedium SA		Γ (SP-SM); (10YR 4/6); moist; es; trace GRAVEL;
-15	785 		S-4	3 4 5	9	11				15 <u> </u>		dense; yellowis	light yell sh browr few fine	lowish bro n (10YR 4/	wn (10YR 6/	ostly fine to coarse
-20	780 		R-5	7 11 15	26	21	22.4	102	PA PI C	20 — - - -		moist; r (0% Gr	nostly fir	nes; little fi % Sand; 8	ne SAND; lo	brown (10YR 4/6): bw plasticity.
GR	924	5 A	ctivi	A CON ty Roa , Calif	ad, S	uite	103	INC	S. OF SU LO WI	THIS BOBSURFA CATION TH THE	MARY APPL DRING AND ACE CONDI S AND MAY PASSAGE ED IS A SIM	O AT THE ITIONS MA Y CHANGE OF TIME.	TIME OF AY DIFFE AT THIS THE DA	DRILLING: ER AT OTH S LOCATIC TA	ER DN	FIGURE A-6 a

E	3OR	INC	G F	RECC	RD	١ ١	PROJEC Rivers			unity H	ospital				ECT N 809	NUMBER	<u> </u>	BORING B-01
	CATION		_		-	0:1						STAF			FINIS			SHEET NO.
	OSEC P		Gara	age and	lower	Sites		DPII I	ING M	ETHOD		4/4	/2024	LOG		1/2024	CHE	2 of 2 CKED BY
	w Jack		llina							tem Au	ger			JW		J 1	MA	
	NG EQUI								NG DIA			EPTH (ft)	GROUN	-		DEPTH/		ROUNDWATER (ft)
CME	75 Lim	nited A	Acces	s Rig #1	42			8			36.5		800			y / n	a	
_	ING MET						NOTES											
Hami	mer: 14	0 lbs.	, Dro	p: 30 in.	(Auton	natic)	ETR	~ 74	%, N ₆	₀ ~ 74/	60 * N ~ 1	.23 * N						
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	2°	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DES	CRIPTI	ON A	ND CLAS	SSIFICA	TION
-			S-6	5 9 10	19	23				-		stiff; ye	llowish e SAND	brown	(10YI	R 5/6); r		Y (CL); very nostly fines;
- 30 -	770 		R-7	8 13 20	33	27	16.0	117		- 30 — -		(10YR	EY SANI 5/4); mo ow plast	oist; mc	med ostly f	lium der îne to c	nse; yel oarse S	lowish brown AND; some
- 35	765 		S-8	3 9 18	27	33				- 35 —		yellowi	LLUVIU sh brow medium	n (10Y	R 4/6	i); moist	; mostly	CL); hard; / fines; some ceous.
- - - -40										- - 40 —			epth: 36			ered		
45	_									- - -								
45 45	755 									45 — -								
	_									-								
										_								
<u>-</u>	_									-								
GR TOO DOT TOO	924	5 A	ctivi	A CON ty Roa o, Calif	ad, S	uite	103	INC	OF SU LO WI	THIS BOBSURFACATION TH THE	MARY APPL ORING AND ACE CONDI' S AND MAY PASSAGE (ED IS A SIMI	AT THE TIONS MA CHANGI OF TIME.	TIME OF AY DIFFI E AT THI THE DA	F DRILL ER AT (IS LOCA TA	ING. OTHE ATION	R N	F	FIGURE A-6 b

	30R	NI(GF	RECO)RD	١ ١	PROJE			unit. I I	onit-l					IUMBER	l	BORING B-02
	CATION		<u> </u>	\LCC	טווכ	,	Rivers	side C	ommı	unity Ho	ospital	STAR	т	SD	809 FINIS	Н		SHEET NO.
Prop		arking	g Gar	age and	Tower	Sites		DDILL	INC M	ETHOD			/2024	1.00	1	/2024	CUE	1 of 3
	w Jack		llina					1		tem Au	ger			JV		S Y	MA	CKED BY
RILLI	NG EQUI	PMEN	Т						NG DIA			PTH (ft)	GROUN			DEPTH/		ROUNDWATER
			ounte	d Rig #1	20		NOTE	8			51.5		798			▼ 36.	0 / 762	2.0
	ING MET mer: 14		., Dro	p: 30 in.	(Auton	natic)	NOTE		%, N ₆	o ~ 80/6	60 * N ~ 1.	33 * N						
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	Z	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DES	CRIPT	IA NOI	ND CLAS	SSIFICAT	ΓΙΟΝ
												PAVEN	IENT:	2-inch	es As _l	phalt Co	oncrete.	
			B-1							- -			llowish SAND	brown ; some	(10YI fines	R 4/4); r	moist; m	m dense; nostly fine to AVEL;
-5	_			1						- 5 —		yellowis medium nonplas	sh browi n SAND	n (10Y	'R 4/6): moist	: mostlv	loose; dark fine to VEL;
		X	S-2	2 2	4	5				-								
	790 									- -								
-10	_	X	R-3	2 5 5	10	9	3.2	109	PA DS	10 — –		(3% Gra	avel; 83	% Sar	nd; 14	% Fines	s)	
	785									- -								
-15				1						- 15 —								
	_	X	S-4	3	5	7				- -								
	780 									- -								
-20	_		R-5	4 12 15	27	24	22.6	106	PA PI C	20 — -			oist; m̀o					own (10YR medium
	-									_		(0% Gra	•	(Ca	1. OZ0.	/ Ein\		
	775 									-		(U% G13				o i iiies)	•	
GR				A CON				INC	OF SU	THIS BO BSURFA	MARY APPLII DRING AND A CE CONDIT	AT THE T IONS MA	TIME OF AY DIFFE	DRILL ER AT	JNG. OTHE		F	IGURE
				ty Roa , Calif					WI [*] PR	TH THE I	S AND MAY PASSAGE O ED IS A SIMP NS ENCOUN	F TIME. LIFICATI	THE DA	.TA				A-7 a

Е	BOR	IN	G F	RECC	ORD	۱ ۱	PROJEC Rivers			unity H	ospital			PROJECT SD809		BORING B-02
Propo Prillin	CATION osed Pa IG COME	arking PANY	g Gara	age and				DRILL	ING M	ETHOD	•	STAR 4/1	rt /2024	FINIS 4/	sн 1/2024	SHEET NO. 2 of 3 CHECKED BY
	w Jack		-						low S	tem Au		SEDTU (#1)	CROUN	JWJ	DEDTU <i>EL</i>	MAF EV. GROUNDWATER
				d Rig #1	20			8	NG DIA	. (111)	51.5	DEP IN (II)	798	D ELEV (II)	▼ 36.0	
AMPLI	NG MET	HOD					NOTES								-	
Hamn	ner: 14	0 lbs.	., Dro	p: 30 in.	(Auton	natic)	ETR	~ 80	%, N ₆	₅₀ ~ 80/	60 * N ~ 1	1.33 * N				
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	Z ⁰	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DESC	CRIPTION A	.ND CLASSI	FICATION
	 770		S-6	4 4 6	10	13				- - -		stiff; da	rk yellov ttle fine \$		(10YR 3/6	CLAY (CL); very); moist; mostly city.
-30		X	R-7	8 17 22	39	35	11.0	123		30 —		yellowis	sh browr	<u>า (</u> 10YR 4/4	1); moist; 'n	SC); dense; dark nostly fine to coarse nedium plasticity.
-35			S-8	4 8 14	22	29				35 — - -	/// y	yellowis	sh browr SAND; s	า (10ÝR 4/4	1); saturate	e to dense; dark d; mostly fine to AVEL; medium
_40	_	X	R-9	15 24 30	54	48	12.6	119		40 — -		4/6); sa	ıturated;		to mediur	owish brown (10YR n SAND; some
- 45	755 									- - 45 —		(10YR		urated;̀ mó		yellowish brown ittle fine SAND;
	— — —750		R-10							-		very pa 6/6), sa	le browr turated;	n (10YR 8/4	1) to brown to coarse	SW-SM); dense; ish yellow (10YR SAND; few aceous.
	_									-		the bot	tom of th	e hollow-s	tem auger.	oundwater flow into
GR	924	5 A	ctivi	ty Roa ty Roa , Calif	ad, S	uite	103	INC	OF SU LO WI	THIS BOBSURFA CATION TH THE	ORING AND ACE COND	O AT THE T ITIONS MAY CHANGE OF TIME.	TIME OF AY DIFFE AT THIS THE DA	ER AT OTHE S LOCATION TA	ER N	FIGURE A-7 b

	30R		G F	RECC	DRD	١ ١	PROJE Rivers			unity Ho	ospital	STAF		PROJEC SD80		ER	BORING B-02 SHEET NO.
Prop	osed P	arking	g Gara	age and	Tower	Sites							/2024	4	4/1/202		3 of 3
Yello	NG COM W Jack NG EQUI	et Dri	Т	I D: //4	00			Hol BORII		ETHOD tem Au (in)	TOTAL	DEPTH (ft)		JWJ ELEV (f	ft) DEPT	M. H <i>IELEV.</i> G	CKED BY AF ROUNDWATER (ft)
	ING MET		ounte	d Rig #1	20		NOTES	8			51.5		798		¥ 3	36.0 / 76.	2.0
Ham	mer: 14	lo lbs.	., Dro	p: 30 in.	(Auton	natic)	ETR	~ 80	%, N ₆	₀ ~ 80/6	60 * N ~	1.33 * N					
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	2°	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DESC	CRIPTION	I AND CL	.ASSIFICA	TION
-	745	X	S-11							_		dense; mostly	light yellofine to m	owish bredium S	own (10 AND; fe	DED SAN IYR 6/4); w fines; r	D (SW); saturated; nonplastic.
-										_ _		Ground	dwater De dwater ini	epth: 36 tially obs	served a	at 46 feet	
55	-									55 —		when the	he botton	n of the l	borehole	heaved.	
_	_									-							
-	_									-							
-	740									-							
60										60 —							
										- 00							
-	_									_							
-	<u>735</u>									-							
-										-							
65	_									65 —							
-	-									_							
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70 2000 1418	730									-							
5 – 2 – 70										70 <i>—</i>							
										-							
	L									_							
00 K	<u>725</u>									_							
NE MINO	_									_							
TIOS XWW SHIPLE OF THE	924	5 A	ctivi	A CON ty Roa o, Calif	ad, S	uite	103	INC	OF SU LO WI PR	THIS BOURFACATION TH THE ESENTE	ORING AN ACE CONE S AND MA PASSAGE ED IS A SII	PLIES ONLY ID AT THE DITIONS MAY CHANGE OF TIME. MPLIFICAT JNTERED.	TIME OF AY DIFFE E AT THIS THE DAT	DRILLING R AT OT S LOCATI A	G. HER ION	F	FIGURE A-7 c

E	BOR	RIN	G F	RECO	ORD	١ ١	PROJE			unity Ho	ospital			PROJECT SD809	NUMBER	BORING B-03
Prop	OCATION	i arkin		age and			TAIVCIS			ETHOD	ээрнаг	STAF 4/2	rt /2024	FIN	ish /2/2024	SHEET NO. 1 of 2
	w Jack									tem Au	•			JWJ		MAF
	NG EQUI			s Rig #1	42			BORII 8	NG DIA	(in)	36.5	DEPTH (ft)	GROUN 798	D ELEV (ft	DEPTHÆI ▼ / na	<i>LEV.</i> GROUNDWATER (
AMPL	ING MET	HOD					NOTES	3								
Ham	mer: 14 ⊤	IO Ibs	., Dro ⊟	p: 30 in.	(Auton	natic) ⊺	EIR	R ~ 80 ⊤	'%, N ₆ ⊺	₀ ~ 80/0	60 * N ~	1.33 * N				
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	ž	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DES	CRIPTION	AND CLASS	SIFICATION
												PAVEN	MENT: 2	2-inches A	sphalt Cor	ncrete.
			B-1						PA EI	_		yellowis	sh browr 1 SAND:	า (10YR 5	/6); moist;	medium dense; mostly fine to GRAVEL; nonplastic;
									CR			(4% Gr	avel; 49	% Sand; 4	17% Fines))
-5										5 —						
-5			R-2	6 11	24	21	3.1	104) 5 <u> </u>						
			11-2	13	24		0.1	104		-						
	_									-						
	<u>790</u>									-						
	_									-		dense;	yellowis	h brown (10YR 5/6);	(SM); medium moist; mostly fine to
-10	_		,	4						10 —		coarse	SAND; I	ittle fines;	nonplastic) .
	_	X	S-3	6 6	12	16				_						
	_	,								_						
	785									_						
										_						
-15										15 <u> </u>						(SW-SM); medium
-10		M	R-4	5 11	23	20	4.2	106		10 —		(10YR	5/6); mo	ist; mostly	fine to co	to yellowish brown arse SAND; few
				12						_		fines; tr	ace GR	AVEL; noi	nplastic; m	icaceous.
										-						
	780									-						
	_									_						
-20	_		,	6						20 —		Few GI	RAVEL.			
	_	X	S-5	8 7	15	20				-						
	_			, ,						_						
	775									_						
										_						
GP		DE		\ CO\	16111	TAN	ITC	INIC	TH			LIES ONLY				FIGURE
GR				ty Roa			-	111/	SU	BSURF#	ACE COND	D AT THE ' DITIONS MA Y CHANGE	Y DIFFE	R AT OTH	ER	IIGUIL
				, Calif					WI	TH THE	PASSAGE	OF TIME. MPLIFICAT	THE DA	TA		A-8 a
	Ju	ال	Jyo	, Juiii	J11116	. J.	U				NS ENCOL				-	

F	30R	IN	G F	RECO	DRD	\	PROJE			unity H	osnital			ROJECT NUM SD809	/IBER	BORING B-03
SITE LO	CATION	ı					iviveis	iiu c C	OHIIII	unity 17	υομιιαι	START		FINISH		SHEET NO.
	osed Pa		g Gar	age and	Tower	Sites		ייימח	INC 1	ETUAR		4/2/2024		4/2/2		2 of 2
	ng com i w Jack		illina							tem Au	ıger			OGGED BY		CHECKED BY MAF
DRILLII	NG EQUI	PMEN	T					BORII	NG DIA		TOTAL		IND E	LEV (ft) DE		V. GROUNDWATER (
	75 Lim		Acces	s Rig #1	42		NOTE	8			36.5	798	}	Ţ	. / na	
			., Dro	p: 30 in.	(Auton	natic)	NOTES		%, N _e	_{so} ~ 80/	60 * N ~	1.33 * N				
				Ì												
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	Z°	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DE	SCR	IPTION AND	CLASSIF	ICATION
	_		R-6	11 30 40	70	62	6.2	114		-	Δ Δ Δ 	(SW-SM); ver	ry de h bro	nse; very pa wn (10YR 4	ale browr 1/6): mois	AND WITH SILT n (10YR 7/3) to st; mostly fine to
	_									-		coarse SAND	; few	/ finès; nonp — — — — —	olastic; m	nicaceous.
	770									-		CLAYEY SAN	ND (S	SC); dense;	dark yell	lowish brown um SAND; some
										-		fines; trace G				
_30				7						30 —						
		X	S-7	7 12	26	35				_						
		$\overline{}$	4	14												
										-						
	765									-						
	_									-						
_35			R-8	11 19 36	55	49	11.6	122		35		yellowish bro	wn (1	10YR 5/6); r	noist; mo	W-SM); dense; ostly fine to coarse plastic; micaceous
	_									-	-	Total Depth:	361%	Feet		
	<u>760</u>									-		Groundwater			d	
_40										40 —						
	_									-	-					
	_									-						
	755									_						
_45	_									45 —	1					
	-									-	-					
	_									-	-					
	750									-						
										-						
GR				A CON				INC	OF SU LO	THIS B IBSURF ICATION	ORING AI ACE CON IS AND M	PLIES ONLY AT THE TIME OF THE TIME OF THE TIME OF THE TIME OF THE	OF DF FER A	RILLING. AT OTHER		FIGURE
				, Calif					PR	RESENTE	ED IS A S	E OF TIME. THE D IMPLIFICATION OF UNTERED.		E ACTUAL		A-8 b

F	30R	NI (G F	RECC)RD	۱ ۱	PROJE			ınitı II	nonital			PROJE SD8	CT NUME	BER	BORING B-04
	CATION		<u> </u>	<u>,_</u>	/		Kivers	iae C	ommı	unity H	ospital	START	•		INISH		SHEET NO.
Propo	osed P	arking	g Gara	age and	Tower	Sites						4/4/2			4/4/202	24	1 of 2
RILLIN	IG COM	PANY						1		ETHOD		1		LOGG			HECKED BY
	w Jack									tem Au		DTU (C)		JW			MAF
	IG EQUI			s Rig #1	42			8 8	NG DIA	(IN)	31	:ΡΙΗ (π) Ο	794) ELEV	I .	ıнÆLEV / na	. GROUNDWATER
	NG MET		10000	ortig # i			NOTE				1 01		701		*	, ,,a	
lamr	ner: 14	l0 lbs.	, Dro	p: 30 in.	(Auton	natic)	ETF	R ~ 74	%, N ₆	₀ ~ 74/	60 * N ~ 1.	23 * N					
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	Z	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DESC	CRIPTIC	N AND C	LASSIFI	CATION
			D 4							-			owish b	rown (10ÝR 4/4	l); mois	dium dense; t; mostly fine to
5	790		B-1							- 5 —		YOUNG yellowish medium	n brown	(10YR	5/6); mo	oist; mo	M); loose; stly fine to
,	-	X	R-2	3 4 4	8	7	9.4	112	PA DS	- - -		(0% Gra	vel; 65%	% Sand	l; 35% Fi	nes)	
10	785 		S-3	2 2 2 2	4	5				- 10 — -			(4); moi	st; mos			wish brown Im SAND; some
15	—780 —	X	R-4	5 9 11	20	16	2.6	111		- 15 — -			ellowisł SAND;	h browr	1 (10YR	5/6); mo	· — — — — — — — — — — — — — — — — — — —
20	775 		S-5	14 27 43	70	86				- 20 — - -		GRAVEL	_(SW); ostly fin	very dene SAN	ense; str D; little (ong bro	AND WITH wn (7.5YR 5/8); _; few fines;
	OUP 924	5 A	ctivi	A CON	ad, S	uite	103	INC	OF SU LO WI	THIS BOBSURFACATION TH THE	MARY APPLI ORING AND ACE CONDIT S AND MAY PASSAGE C ED IS A SIMF	AT THE TI TONS MAY CHANGE A OF TIME. T	ME OF I OIFFEI AT THIS HE DAT	DRILLIN R AT O' S LOCA' FA	ng. Ther Tion		FIGURE A-9 a

	3OR		G F	RECC	RD	١ ١	PROJE Rivers			unity H	ospital	STAF		SD80	T NUMBE	ER	BORING B-04 SHEET NO.
DRILLII	vg com w Jack	PANY et Dri	lling	age and	Tower	Sites		Hol	low St	ETHOD tem Au		4/4	1/2024	LOGGE JWJ	1/4/2024 D BY	CHE M/	2 of 2 ECKED BY AF
CME	NG EQUI 75 Lim ING MET	nited A		s Rig #1	42		NOTES	8	NG DIA	(in)	31	DEPTH (ft)	GROUNE 794	ELEV (f	t) DEPTI		ROUNDWATER (ft)
Hamı	mer: 14	0 lbs.	., Dro	p: 30 in.	(Auton	natic)	ETR	? ~ 74	%, N ₆	₀ ~ 74/	60 * N ~	1.23 * N					
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	Z ^o	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DESC	CRIPTION	I AND CL	ASSIFICA	TION
- - - - 30		X	R-6	40 50 (4")	100+	100+				- - - - 30 —		GRAVI moist;	LLUVIUM EL (SW); mostly fin stic; mica	very der ne SAND	nse; stro	ng browr	D WITH n (7.5YR 5/8); few fines;
- -			5-7	50	76	93				- - -		Total D Ground	epth: 31 dwater No	Feet ot Encou	ntered		
35 - -	_									35 — -	-						
- 40 -	755 									- 40 —	-						
- - - - - -45	750									- - 45 —	-						
	 745									- - -	-						
GR	924	5 A	ctivi	A CON ty Roa , Calif	ad, S	uite	103	INC	OF SU LO WI PR	THIS BOURFACATION TH THE ESENTE	MARY APP ORING AN ACE COND IS AND MA PASSAGE ED IS A SIN NS ENCOL	D AT THE DITIONS MA Y CHANGI OF TIME. MPLIFICAT	TIME OF I AY DIFFE E AT THIS THE DAT	DRILLING R AT OTI S LOCATI FA	G. HER ON	F	FIGURE A-9 b

F	30R	IN	G F	RECC)RD	۱ ۱	PROJE			unity H	oenital			PROJECT SD809		BORING B-05
Prop	osed P	ı arkinç		age and			Nivers				озрікаі	STAF 4/4	кт /2024	FINI 4/-	s н 4/2024	SHEET NO. 1 of 2
	vg com w Jack		lling							ETHOD tem Au	ıaer			LOGGED	BY	MAF
	NG EQUI		-						NG DIA			DEPTH (ft)	GROUN		DEPTH/EL	EV. GROUNDWATER
			Acces	s Rig #1	42			8			31.5		796		▼ / na	
	ING MET		Dro	p: 30 in.	(Auton	actic)	NOTES		0/. NI	~ 71/	60 * N ~ 1	1 22 * N				
ı ıaıııı	11161. 14	o ibs	., DIO		(Auton		EIR	14	70, IN ₆	0 74/	1 1	1.23 IN				
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	ް	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DES	CRIPTION A	ND CLASS	IFICATION
	795 		B-1							-		dark ye mediur	llowish l	brown (10Ý ; some fine	'R 4/4); mo	nedium dense; oist; mostly fine to tic. Contains trash
- 5	790 		R-2	1 2 4	6	5	12.6	106	PA	5 — - -	-	yellowi fine to	sh browr medium		6); moist; r nplastic; m	(ML); loose; dark nostly fines; some icaceous.
-10	785 		S-3	6 7 10	17	21				- 10 — - -		brown	(10YR 4)		mostly fine	e; dark yellowish to medium SAND; s.
-15	780 		R-4	10 32 60	92	75	6.5	127		- 15 — - -		(SC); v mostly	ery dens	se; yellowis oarse SAN	h brown (1	WITH GRAVEL 10YR 5/6); moist; nes; little GRAVEL;
-20	775 		S-5	10 13 20	33	41				- 20 — - - -		very pa 5/8); m	ile browr oist; mo:	า (10YR 8/4	4) to yellow coarse SA	(SW-SC); dense; vish brown (10YR ND; little fines; few
GR				A CON				INC	OF SU LO	THIS BOURFACATION	MARY APPL ORING AND ACE CONDI IS AND MAY	O AT THE ITIONS MAY CHANGI	TIME OF AY DIFFE E AT THI	DRILLING. ER AT OTHE S LOCATIO	₽R	FIGURE
				, Calif					WI PR	TH THE ESENTE	PASSAGE ED IS A SIM NS ENCOU	OF TIME. IPLIFICAT	THE DA	TA		A-10 a

F	30R	IN	G F	RECO)RD	١	PROJE			unity Ho	nenital			PROJECT SD809		BORING B-05
Propo	osed P	ı arkinç		age and			Rivers				ospitai	STAF	rt /2024	FINI 4/	sн 4/2024	SHEET NO. 2 of 2
	NG COMI W Jack		illing							ETHOD	ıaor			LOGGED JWJ	BY	CHECKED BY MAF
	NG EQUI								NG DIA	tem Au		DEPTH (ft)	GROUN	1	DEPTH/EL	EV. GROUNDWATER (
			Acces	s Rig #1	42			8		. ,	31.5		796	. ,	▼ / na	
	ING MET		Dro	p: 30 in.	(Auton	actic)	NOTES		0/ NI	- 74/	60 * N -	1 22 * N				
Hami	mer: 14	Edi Ds	., Dio	p: 30 in.	(Auton	nauc)	EIR	~ 74	.%, IN _∈	₀ ~ 74/	00 " N ~	1.23 * N				
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	ž	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DES	CRIPTION A	AND CLASS	IFICATION
	770 		R-6	9 17 22	39	32	13.5	109		- - -		GRAVE yellowis	L (SW) sh browr n SAND:	; dense; ve n (10YR 5/-	ery pale bro 4); moist; r	SAND WITH own (10YR 8/2) to mostly fine to fines; nonplastic;
_30	 765	X	S-7	10 27 50	77	95				30 — -		Very pa (10YR	ale brow 6/4); ver	n (10YR 8/ y dense.	(3) to light	yellowish brown
	_									-	-	Total D Ground	epth: 31 lwater N	½ Feet ot Encount	tered	
_35	_									35 —						
	760									_	_					
										_						
	_									-	-					
	_									-	-					
_40	_									40	_					
	755									_						
	-															
										_	1					
	-									-	1					
	<u> </u>									_	-					
- 45	_									45 <u>—</u>						
-	750															
	750									-	1					
	-									-	1					
	<u> </u>									-	-					
	_									_	-					
GR	OUP	DE	LTA	A CON	ISUL	L ATAN	NTS,	INC	C. OF	THIS B	ORING AI	PLIES ONLY	TIME OF	DRILLING.		FIGURE
	924	5 A	ctivi	ty Roa , Calif	ad, S	uite	103		LO Wi PR	CATION TH THE ESENTE	S AND M. PASSAG ED IS A S	DITIONS MAY CHANGE E OF TIME. IMPLIFICAT OUNTERED.	AT THIS	S LOCATIO TA	N	A-10 b

Е	30R	IN(G F	RECO	DRD	١ ١	PROJE Rivers			unity H	ospital			PROJECT SD809			BORING B-06
	CATION		_		_					-		STAR		FINI	SH		SHEET NO.
	osed P IG COM		g Gar	age and	Tower	Sites		DRILI	ING M	ETHOD		4/3/	/2024	LOGGED	4/2024 BY	CHE	1 of 2 CKED BY
	w Jack		lling							tem Au	ıger			JWJ	٥.	MA	
	IG EQUI								NG DIA	. (in)	I	EPTH (ft)		ELEV (ft)			ROUNDWATER
	75 LIM		Acces	s Rig #1	42		NOTE	8 s			31.5		794		▼ / na	a	
Hamr	ner: 14	l0 lbs	., Dro	p: 30 in.	(Auton	natic)	ETF	? ~ 74	%, N ₆	~ 74/	60 * N ~ 1.	23 * N					
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	20	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DESC	RIPTION A	AND CLAS	SIFICA ⁻	TION
											P 5 4 P	PAVEN	IENT:4-iı	nches Por	tland Cer	ment C	oncrete.
			B-1						PA	- -		dark ye	llowish bi	ND (SM); rown (10Y dium SAN	'R 3/6); m	noist; n	m dense; nostly fines; nicaceous.
-5	790								EI CR R	5 —		dark yel medium	llowish bi SAND;		'R 4/4); m s; low pla	noisṫ; m	C); loose; nostly fine to Sample has
	_	X	S-2	2 2 2	4	5				-				ying vege 5 Sand; 4)	
	— —785									-		(10YR 8	3/4) to ye	SAND (S llowish br AND; little	own (10Y	'R 5/6)	pale brown moist; mostly fines;
-10		X	R-3	10 24 36	60	49	13.1	114		10 —		nonplas	tic.				
	_			00						-							
-15	780 									- 15 —		OLD AL	LUVIUN	<u>I</u> : SILTY	SAND W	TITH GF	RAVEL (SM);
	_		S-4	12 26 30	56	69				-		fine to n	nedium S	SAND; sor	me fines;	little G	moist, mostly RAVEL;
	— —775		B-5							_							
00										_							
-20		X	R-6	26 50 (5")	100+	100+				20 —		dense; (10YR 6	very pale 5/8); mois		0YR 8/3) fine to co	to brow arse S	wnish yellow AND; few
	_									-		, -			,		
	<u> </u>									_					· · · · ·		
GR	OUP	DE	LTA	A CON	NSUL	.TAN	NTS,	INC	ر OF	THIS B	MARY APPLI ORING AND	AT THE T	IME OF D	DRILLING.		F	IGURE
	924	5 A	ctivi	ty Roalit	ad, S	uite	103		LO	CATION TH THE	ACE CONDIT IS AND MAY PASSAGE C ED IS A SIMF	CHANGE F TIME.	AT THIS	LOCATIO A	N		A-11 a

SITE LO	CATION	ı		RECC			PROJE Rivers			unity H	ospital	STAF	RT	SD80	T NUMBE	R	BORING B-06 SHEET NO.
DRILLII Yello DRILLII CME	NG COMI W Jack NG EQUI	PANY et Dri PMENT nited A	lling	age and		Sites	NOTES	Hol BORII 8		ETHOD tem Au (in)			GROUNE 794	LOGGEI JWJ		CHE M <i>i</i> MELEV. G	2 of 2 CKED BY AF ROUNDWATER (ff)
			, Dro	p: 30 in.	(Auton	natic)			%, N ₆	₀ ~ 74/	60 * N ~	1.23 * N					
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	° Z°	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DESC	CRIPTION	AND CLA	ASSIFICA	TION
- - - - -30			S-7	7 25 25 25	50	62				- - - - 30 —		very de	ense; yello medium S	owish br	own (10)	/R 5/6);	RAVEL (SM); moist; mostly GRAVEL;
- - 35 -	760 									- - 35 — -	6.7.1.9		epth: 31½		ntered		
40	755 									- 40 — -							
45	750 									- 45 — - -							
	745									-	-						
GR	924	5 A	ctivi	A CON ity Roa o, Calif	ad, S	uite	103	INC	OF SU LO WI PR	THIS BOURFACATION TH THE ESENTE	MARY APP ORING ANI ACE COND IS AND MA PASSAGE ED IS A SIN NS ENCOL	D AT THE DITIONS MAY OF TIME. MPLIFICAT	TIME OF I AY DIFFEI E AT THIS THE DAT	DRILLING R AT OTI S LOCATI A	G. HER ON		FIGURE A-11 b

E	30F	RIN	G F	RECO	ORD)	PROJE Rivers			unity H	ospital			PROJECT SD809		BORING B-07
	CATION		. 0		T	0:4					·	STAR		FINI		SHEET NO.
	osed P NG COM		g Gar	age and	Iower	Sites		DRILI	ING M	ETHOD		4/4/	2024	LOGGED	4/2024 BY	1 of 2
	w Jack		Ilina							tem Au	ıaer			JWJ	J.	MAF
	NG EQUI								NG DIA			PTH (ft)	GROUND	ELEV (ft)	DEPTH/EL	.EV. GROUNDWATER
			Acces	s Rig #1	42			8			28		794		▼ / na	
	.ING MET		Dro	p: 30 in.	(Auton	natic)	NOTE		% N	~ 7//	60 * N ~ 1.2	23 * N				
ı ıaııı	11161. 15	lo ibs	., Dio		(Auton		L 11	\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	70, IN ₆	30 - 7 - 7	1 1.2	20 11				
et)	z	뮙	o.	PENETRATION RESISTANCE (BLOWS / 6 IN)	ż		ш	≥		et)						
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	RAT STAN	BLOW/FT "N"	Z ⁰⁹	MOISTURE (%)	IS (£	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DECC	DIDTION A	ND CLASS	IFICATION
ΞĐΤ	EV.	/PLI	MP	NET NO.	, o	_	OIS (%)	7 9	 	l H	LC LC		DESC	KIPTION	AND CLASS	IFICATION
D		SAN	\s	<u> </u>	BI		Σ	DRY DENSITY (pcf)		_ B						
		XXX	1								:/: /: /					
		\bowtie								_						o medium dense;
												fine to m	iowish bi nedium S	rown (101 SAND; soi	ne fines; li	oist to wet; mostly ittle GRAVEL; low
	_		B-1							-		plasticity		•	•	·
	_	\bowtie	D-1							-		VOLING	ΔΙΙΙΙ	IIIM: SII	TV SAND	(SM); loose to
	790											medium	dense;	yellowish	brown (10)	ÝR 4/6); moist;
	730	\bowtie								_		mostly fi	ne SANI	D; some f	ines; mica	ceous; nonplastic.
-5	_	\times		2						5 _						
	_	X	S-2	3	8	10				_						
				5												
										-						
										-	_					
	785									_						wish brown (10YR
												4/6); mo	ist; mosi RAVEL: i	tly fines; s nonplastic	some fine t c; micaceo	o medium SAND; us.
-10	_		i	2						10	1			•		
			R-3	4 5	9	7	15.7	113	PA	-		(1% Gra	ivei, 39%	o Sariu, o	0% Fines)	
				3												
										_]					
	_									-						
	780									_						(SW-SM); medium to yellowish brown
-15										15		(10YR 5	/6); mois	st; mostly	fine to coa	arse SAND; trace
-15			1 .	3						15		fines; tra	ace GRA	VEL; non	plastic.	
	_	X	S-4	7 15	22	27				-						
										_						
	_									-	اظ اما م	<u> </u>			0545	2415 (2)("
	775									-						SAND (SW); very to brownish yellow
-20										20 —		(10YR 6	/8); mois	st; mostly	fine to coa	arse SAND; few
-20			D -	16	0.4	60	F 0	110		20].	GKAVE	∟; trace 1	ıınes; non	plastic; mi	caceous.
	_		R-5	34 50	84	69	5.6	116		-						
	_]							_						
										-						
	770									-						
											Δ. Δ. Δ.					
GR	OLID	DE	T/	A CON	12111	TAP	NTS.	INC	TH		MARY APPLIE				۱	FIGURE
JIV								1147	SU	IBSURF	ACE CONDIT	IONS MA	Y DIFFER	R AT OTHE		. IOOKE
				ty Roa					WI	TH THE	S AND MAY (PASSAGE O	F TIME.	THE DAT	Ά		A-12 a
	Sa	וט ח	ego	, Calit	iornia	3 92	126				ED IS A SIMP NS ENCOUN		ON OF TH	HE ACTUA	L	~

	30R		G F	RECC	DRD	١	PROJE Rivers			unity H	ospital	STAF		PROJEC SD80		BER	BORING B-07 SHEET NO.
			g Gara	age and	Tower	Sites							/2024		4/4/20)24	2 of 2
	NG COM									ETHOD		'		LOGGE	D BY		HECKED BY
	w Jack NG EQUI		_						NG DIA	tem Au		DEPTH (ft)	GROUNE	JWJ	ft) DEF		MAF GROUNDWATER (ft
				s Rig #1	42			8		(,	28	(,	794	(.	- 1	/ na	
	ing met mer: 14		., Dro	p: 30 in.	(Auton	natic)	NOTES		%, N ₆	₀ ~ 74/	60 * N ~	1.23 * N			'		
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	2º	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DESC	CRIPTION	N AND (CLASSIFIC	CATION
-	_	X	S-6	21 29 20	49	60				-		dense; (10YR	very pale 6/8); moi	e brown st; most	(10YR ly fine	8/3) to b	ND (SW); very prownish yellow a SAND; few peous.
										_		Total D	epth: 28	Feet			
<u> </u>	765									-	1	Ground	dwater No	ot Encou	ıntered	t	
30	-									30 —							
-	_									-							
	_									_							
										_							
-	760									-	1						
<u> 35 </u>	_									35 —							
-	_									-	-						
										_							
										_							
	<u>755</u>									-							
40	_									40 —	-						
-	_									_	-						
<u> </u>	_									-							
] -	_									_							
45 45	750																
3	,50									-							
2 45										45 —	1						
ğ -	-									-							
	-									-	-						
<u> </u>	<u> </u>									_							
	745									-	-						
TION KNIMM SHIPLE OF STATE OF	924	5 A	ctivi	A CON ty Roa , Calif	ad, S	uite	103	INC	OF SU LO WI PR	THIS BOBSURFACATION TH THE ESENTE	ORING AN ACE CONI IS AND MA PASSAGE ED IS A SI	PLIES ONLY ND AT THE DITIONS MAY CHANGE OF TIME. MPLIFICAT UNTERED.	TIME OF AY DIFFE E AT THIS THE DAT	DRILLING R AT OT S LOCATI FA	G. HER ION		FIGURE A-12 b

Е	30R	IN	G F	RECC	ORD	`	PROJE Rivers			unity H	ospital		I	SD809	NUMBER	BORING B-08
	CATION				_	0					· · · · · · · · · · · · · · · · · · ·	START		FINIS		SHEET NO.
	OSECIP NG COM		g Gara	age and	Iower	Sites		DRILL	ING M	ETHOD		4/1/2	2024	0GGED	1/2024 BY	1 of 2
	w Jack		lling							tem Au	ıger			JWJ	.	MAF
RILLI	NG EQUI	PMEN	Т					1	NG DIA		TOTAL DE	EPTH (ft)		LEV (ft)	DEPTH/E	LEV. GROUNDWATER
	75 Lim		Acces	s Rig #1	42		NOTE	8			30		794		▼ / na	<u> </u>
			., Dro	p: 30 in.	(Auton	natic)	_		%, N ₆	o ~ 80/	60 * N ~ 1.	33 * N				
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	Ž	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DESCRI	PTION A	IND CLASS	SIFICATION
<u> </u>		SAN	SA	PEN RE (BL	BL		¥	DRY								medium dense;
			B-1						PA EI	-		some fin	es; little G	RAVEL	mostly fine ; nonplast 12% Fines	e to coarse SAND; tic; micaceous.
_5	790 		S-2	2 4 4	8	11			CR R	5 — -		WITH SI (10YR 5	LT (SP-SI /6); moist;	M); med mostly	ium dense	ADED SAND e; yellowish brown dium SAND; few icaceous.
-10				3						- - - 10 —						
			R-3	4 7	11	10	7.2	116		- - -						
-15			S-4	19 50 (5")	79	100+				15 — - -		(SW-SM yellowish); very de n brown (1 ew fines; t	nse; ver 0YR 5/6	y pale bro	SAND WITH SILT own (10YR 8/4) to mostly fine to coarse onplastic;
-20	775									- - 20 —						dense; very pale
		X	R-5	15 35 50	85	75	5.3	107		- -		mostly fi		se SAN		n (10YR 5/6); moist; RAVEL; trace fines;
	770									-						
GR	OUP	DE	LTA	CON	ISUL	_TAN	ITS,	INC	ر OF	THIS B	MARY APPLI	AT THE TI	ME OF DR	RILLING.		FIGURE
				ty Roa , Calif					LO WI PR	CATION TH THE	ACE CONDIT IS AND MAY PASSAGE C ED IS A SIMF	CHANGE . F TIME. T PLIFICATION	AT THIS LO	OCATIO	N	А-13 а

	BOR	RIN	G F	RECO)RD	\	PROJE			unity H	osnital				IECT N	NUMBER		BORING B-08
SITE L	OCATION	1					1111010	140 0	01111110	arricy i i	oopitai	STAF		0.0	FINIS			SHEET NO.
	osed P		g Gara	age and	Iower	Sites		DRILL	ING M	ETHOD		4/1	/2024	LOG	4/1 GED E	I/2024 BY	CHE	2 of 2 CKED BY
Yello	w Jack	et Dri						Hol	low S	tem Au				J۷	٧J		MA	\F
	NG EQUI			s Rig #1	42			BORII 8	NG DIA	(in)	TOTAL 30	DEPTH (ft)	GROUNI 794	D ELE	V (ft)	DEPTH/E		ROUNDWATER (fi
SAMPL	ING MET	THOD		p: 30 in.		natic)	NOTES	3	%, N ₆	o ~ 80/		1.33 * N	701			¥ ///		
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	Z º	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DESC	CRIPT	ION A	ND CLAS	SIFICA ⁻	TION
-		X	R-6	24 50 22 50 (5")	74	66	3.2	112		-		SAND	(GW); ve	ery de /4): m	nse; v oist: n	white (10	YR 8/8 RAVEL	/EL WITH 8) to very pale .; some fine to
30 _ _ _	_ _ _			(0)						30 — - -	-	Total D Ground	epth: 30 dwater N	Feet ot End	counte	ered		
- 35 -	760 									35 — -	-							
- - -										- - -	-							
40 _	_									40 —	-							
-	750									-								
45 - -	_									45 — -	-							
-	745									- -								
GR				A CON				INC	OF SU	THIS BOURFA	ORING AN ACE CONI	PLIES ONLY ID AT THE DITIONS MA	TIME OF AY DIFFE	DRILI R AT	LING. OTHE	R	F	IGURE
				ty Roa , Calif					WI [*]	TH THE ESENTE	PASSAGE	AY CHANGI E OF TIME. MPLIFICAT	THE DA	TA				A-13 b

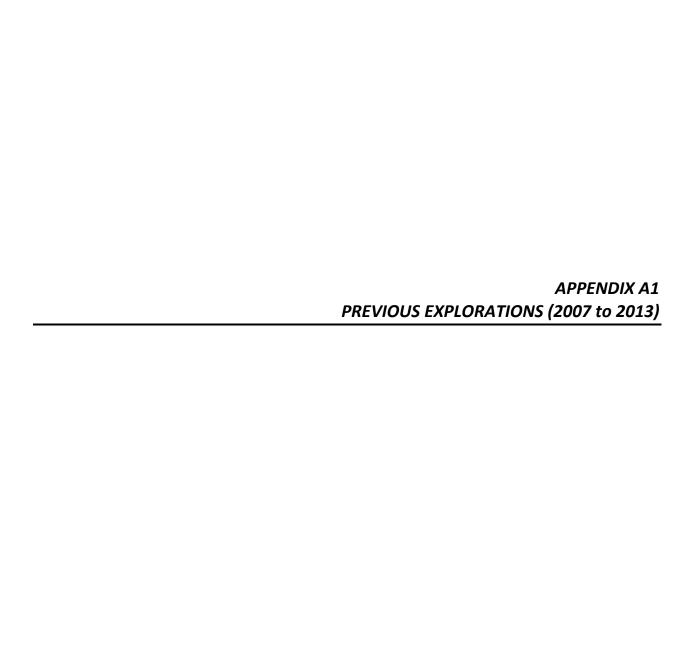
F	30R	IN	G F	RECC)RD	۱ ۱	PROJE			unity Ho	nenital			PROJECT SD809		BORING B-09
Propo	osed Pa	ı arkinç		age and			Rivers				ospitai	STAF	RT 5/2024	FINI		SHEET NO. 1 of 2
	NG COMP		lline							ETHOD	gor	•		LOGGED	BY	CHECKED BY MAF
	w Jack NG EQUII		_						NG DIA	tem Au		DEPTH (ft)	GROUN		DEPTH/EL	EV. GROUNDWATER
			Acces	s Rig #1	42			8			31.5		832		▼ / na	
	ING MET		Dro	p: 30 in.	(Auton	natic)	NOTES		% N	~ 7//	30 * N ~	1.23 * N				
T IGITII	1101. 14	0 103.	., 510		(7 taton			'-	170, 146	50 7-77		1.20 14				
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	Z ⁰	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DES	CRIPTION A	AND CLASSI	IFICATION
	 830		B-1							-		dark ye mediur	ellowish b n SAND;	orown (10Y some fine	'R 4/4); mo	nedium dense; oist; mostly fine to RAVEL and ts.
_5	825		R-2	4 7 7	14	11	10.1	113		5 —		dense; fine to	dark vel	lowish brov SAND; sor	wn (10YR 4	(SM); medium 4/4); moist; mostly ace GRAVEL and
-10			S-3	2 2	4	5			PA	- - 10 —		moist; i nonpla	mostly fir stic.		fine SAND	wn (10YR 5/6); ; micaceous;
	820 			2						- - -						
-15	 815		R-4	11 12 14	26	21	6.5	101		15 — - -						strong brown (10YR ND; nonplastic.
-20	 810		S-5	9 13 26	39	48				20 —		vellowi	sh browr	10YR 4/4); dense; dark nostly fine to tic.
GR	924	5 A	ctivi	\ CON ty Roa	ad, S	uite	103	INC	OF SU LO	THIS BO BSURFA CATION	ORING AN ACE CONI S AND M/	PLIES ONL' ND AT THE DITIONS M AY CHANGI E OF TIME.	TIME OF AY DIFFE E AT THIS	DRILLING. R AT OTHE S LOCATIO	≣R	FIGURE A-14 a

F	30R	IN	G F	RECO)RD	1	PROJE			unity LL	nenital			PROJECT SD809		BORING B-09
SITE LO	CATION	l		age and			Rivers	ide C	omml	unity Ho	ospitai	STAF	rt /2024	FINI		SHEET NO. 2 of 2
	IG COM		y Juli	ago ana				DRILL	ING M	ETHOD			,_027	LOGGED		CHECKED BY
	w Jack		_							tem Au				JWJ		MAF
	NG EQUI			s Rig #1	42			BORII 8	NG DIA	. (in)	31.5		GROUN 832	D ELEV (ft)	DEPTHÆL	EV. GROUNDWATER
	ING MET		10000	33 T (19 # 1	72		NOTES				01.0		002		<u> </u>	
Hamr	ner: 14	0 lbs	., Dro	p: 30 in.	(Auton	natic)	ETR	~ 74	%, N ₆	₀ ~ 74/	60 * N ~	1.23 * N				
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	z	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DES	CRIPTION A	AND CLASSI	IFICATION
	805 		R-6	8 14 22	36	30	2.5	114		- - -		SILT (S moist; r	P-SM);	dense; brone ne to medi	wnish yello	ED SAND WITH ow (10YR 6/8); little GRAVEL; few
_30	_	X	S-7	7 12 16	28	34				30 — -		Grades brown (from ve (10YR 5/	ery pale bro /6); micaeo	own (10YR ous; trace 0	7/4) to yellowish GRAVEL.
	800 									-		Total D Ground	epth: 31 lwater N	½ Feet ot Encoun	tered	
_35	_									35 —						
	_									_						
	795									_						
										_						
	_									-						
_40	_									40 —						
	_									_						
	790															
	1 30									_						
	_									-						
	_									_						
- 45	_									45						
										_						
	705															
	<u> </u>									-						
	-									-						
	_									_						
GR	OUP	DE	LT/	A CON	ISUL	LTAN	NTS.	INC	TH OF			PLIES ONLY				FIGURE
	924	5 A	ctivi	ty Roalif , Calif	ad, S	uite	103		LO Wi PR	BSURFA CATION TH THE	ACE CON S AND M PASSAG ED IS A S	DITIONS MA AY CHANGE E OF TIME. IMPLIFICAT	AY DIFFE AT THIS THE DA	ER AT OTHI S LOCATIO TA	ER N	A-14 b

			G F	RECC	RD	۱ ۱	PROJE Rivers			unity Ho	ospital				809		R	BORING B-10
	CATION		. Car	222 224	Tower	Citoo						STAI			FINIS			SHEET NO.
	NG COM		Gara	age and	Tower	Sites		DRILL	ING M	ETHOD		4/2	2/2024	LOG	4/2 GED E	2/2024 RV		1 of 3
	w Jack		llina							tem Au	ger			JW		5 1	M	
	NG EQUI								NG DIA			DEPTH (ft)	GROUN	_		DEPTH		ROUNDWATER (ft)
				d Rig #1	20			8		()	51.5	(,	838		(,	y / i		(,
	ING MET						NOTES	_			1 4 114							
Ham	mer: 14	0 lbs	, Dro	p: 30 in.	(Auton	natic)	ETR	R ~ 80	%, N ₆	₁₀ ~ 80/6	60 * N ~	1.33 * N						
(feet)	TION	TYPE	E NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	o ₂	.ure	NSITY ()	ER TS	EPTH (feet)	HIC							
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETI RESIS (BLOW	BLOW,	Z°	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH	GRAPHIC LOG		DES	SCRIPTI	ON A	ND CLA	SSIFICA	TION
_	_									-	000		MENT: inches					ete
-	835		B-1						PA	-		brown		5/6); mc	oisť; n	nostly f	ines; so	; yellowish me fine to
-									EI CR	-			ravel; 33	•		•		
5 -	_		S-2	8 9 9	18	24				5 —								
-	830									-								
										_				. — — —				
10 _	_		R-3	8 14 20	34	30	5.8	99	DS	10 — -		moist;	mostly fi	ine to n	nediu	; yellow ım SAN	ID; som	vn (10YR 5/6); e fines; low
-	825									_								
- 15				8						- 15 —		yellow	(10YR 6	3/8); ma	oist; r	nostly f	ines; litt	se; brownish
- 601 - 4/18 	_	X	S-4	9 11	20	27				_		mealur	n SAND	; iow p	iastic	city; mic	aceous	
	820									-								
20 20	_			7						20 —								
			R-5	13 24	37	33	11.2	99	DS	_								
AMA SOIL	815									_								
GLC LOG, BOKING MMX SOIL SD SD809 LOGS; GFJ GDCLOG, GD 1,4/19/24										_						ı		
GR				ty Roa			-	INC	OF SU	THIS BOBSURFA	ORING AN	PLIES ONL' D AT THE DITIONS M XY CHANG	TIME OF AY DIFFI	F DRILL ER AT (ING. OTHE	R	F	FIGURE
الا				, Calif					WI [*] PR	TH THE ESENTE	PASSAGE	OF TIME. MPLIFICAT	THE DA	ATA				A-15 a

F	30R		GF	RECO)RD	۱ ۱	PROJE			ınit. I I	oonital			PROJECT		BORING B-10
ITE LO	CATION	l		age and			Kivers	iae C	ommı	unity H	ospitai	START 4/2/2		SD809		SHEET NO. 2 of 3
	NG COM		y Gara	age and	Tower	Siles		DRILL	ING M	ETHOD		4/2/2	2024	LOGGED		CHECKED BY
	w Jack		_							tem Au				JWJ		MAF
	NG EQUI			LD: //4	00				NG DIA	. (in)	I	PTH (ft)		D ELEV (ft)		EV. GROUNDWATER
	85 Tru ING MET		ounte	d Rig #1	20		NOTES	8			51.5		838		▼ / na	
			Dro	p: 30 in.	(Auton	natic)			%. Na	. ₀ ~ 80/	60 * N ~ 1.	33 * N				
			ĺ	<u>. </u>					, <u>, , , , , , , , , , , , , , , , , , </u>							
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	Z ^O	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DESC	CRIPTION A	ND CLASS	IFICATION
	_	X	S-6	5 9 12	21	28				-	-		n brown	ı (10YR 4/4		dense; light nostly fines; little
-30	810 	X	R-7	7 30 50	80	71	7.5	113		30 —		very den	se; stro		(7.5YR 5/8	FH GRAVEL (SM); 3); moist; mostly fine onplastic;
-35	805 		S-8	8 17 18	35	47				- 35 -	8.0	5/8); mo	ist; mòs		ND; some	- — — — — — — ng brown (7.5YR fines; few
. 40	800 795	X	R-9	15 30 42	72	64	5.6	115		- 40 — -		brown (7	'.5YR 5	//TH GRAV /8); moist; VEL; nonp	mostly fine	very dense; strong e SAND; some aceous.
- 45			,							- 45 —	-	(10YR 4)	/4); mòi		fines; little	ight yellowish brown fine SAND;
	790 	X	S-10	10 27 33	60	80				- - - -		7/4) to b	rownish SAND;	yellow (10	OYR 6/6), r	y pale brown (10YR moist; mostly fine to AVEL; nonplastic;
GR				CON				INC	C. OF	THIS B	MARY APPLI ORING AND ACE CONDIT	AT THE TI	ME OF	DRILLING.		FIGURE
				ty Roa , Calif					LO WI PR	CATION TH THE ESENTE	IS AND MAY PASSAGE C ED IS A SIMF NS ENCOUN	CHANGE . F TIME. T PLIFICATION	AT THIS	S LOCATIO TA	N	A-15 b

	3OR		G F	RECC	RD	1	PROJEC Rivers			unity Ho	ospital	STAF		PROJEC SD80		BER	BORING B-10 SHEET NO.
Prop	osed Pa	arking	g Gara	age and	Tower	Sites							2/2024		4/2/20		3 of 3
Yello	w Jack	et Dri						Holl	low St	ETHOD tem Au				JWJ		M	ECKED BY AF
	iG EQU II 85 Tru			d Rig #1	20			8 8	NG DIA	(in)	51.5		838) ELEV ('ιнÆLEV. ∣na	GROUNDWATER (ft)
SAMPL	ING MET	HOD		p: 30 in.		natic)	NOTES		%, N ₆	₀ ~ 80/6	60 * N ~	1.33 * N					
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	, Z	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DESC	CRIPTION	N AND C	CLASSIFICA	ATION
-		X	R-11	17 27 50	87	77	2.0			_		SILT (S to light mediur	SP-SM); v yellowish n SAND;	very den n brown few fine	ise; ver (10YR	y pale bro 6/4); mois	SAND WITH own (10YR 8/3) st; mostly fine to nonplastic.
-	_									_			epth: 511 dwater No		untered		
<u> 55 </u>	_									55 —							
}										-							
<u> </u>										-							
<u> </u>	 780									-							
60										60 —							
-	_									_							
-										_							
-	<u></u> 775									-							
-	_									- 65 —							
65	_									00 —							
<u> </u>	_									_							
<u> </u>	<u>770</u>									_							
<u>-</u>	_									_							
70	_									70 —							
										_							
	<u> </u>									_							
	_									_							
GR	924	5 A	ctivi	L CON ty Roa , Calif	ad, S	uite	103	INC	SU LO WI	THIS BOBSURFACATION TH THE	ORING AN ACE CONI S AND MA PASSAGE	PLIES ONL' ND AT THE DITIONS MAY AY CHANGI E OF TIME.	TIME OF AY DIFFE E AT THIS THE DAT	DRILLIN R AT OT S LOCAT FA	G. HER ION		FIGURE A-15 c



Date Drilled: 12/20/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 799.0

Logged by: VJR

Measured Depth to Water(ft): 39.0

	DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION (SM) Silty Sand, fine to coarse with gravel to 1", brown	REMARKS	DRIVE BULK	BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
-					X	8 14 19	13.4	120	Ring
-	- 5	-	(ML) Sandy Silt, fine with medium, light brown	Native		3 4 5	6.7	101	SA Ring, Consol.
	10	-				4 7 9	11.5	120	Ring, DS
	15		(SP-SM) Sand, medium to coarse with fine and silt, brown		X	7 12 18	2.3	114	Ring
CHJ.GDT 1/18/08	- 20		(SW-SM) Sand with silt, fine to coarse with clay and gravel to 3", brown		X	38 50/5"	3.6	117	SA Ring
BORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ.GDT 1/18/08	- 25					40 50/5"	4.1	105	Ring, DS
OG - NO EQUIV & BLOW	30				*	50/4"	3.5	104	Ring
BORINGL		-	(SP) Sand, medium to coarse with fine, gravel and cobbles to 4", brown						



PROPOSED HOSPITAL EXPANSION AND PARKING GARAGEOb No. RIVERSIDE, CALIFORNIA 07881-3

Enclosure B-1a

Date Drilled: 12/20/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 799.0

Logged by: VJR

Measured Depth to Water(ft): 39.0

	t)			SS	SAM	IPLES	6 IN.	RE (%)	IT WT.	LD
	DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	DRIVE	BULK	BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
-		-	(SP) Sand, medium to coarse with fine, gravel and cobbles to 4", brown		><		50/4"	6.0	111	Ring
	40 -	-		Groundwater	><		50/4"	17.2	Dist.	Ring
	45 -	-			X		21 50/5"	10.5	123	Ring, DS
-	50 -	- - - -			×		50	11.8	128	Ring
CHJ.GDT 1/18/08	55 -	- - - - -			×		50	12.4	125	Ring
ER 6 IN 07881-3.GPJ	60 -	-			><		50/4"	15.4	123	Ring
BORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ.GDT 1/18/08	65 -	-	NO BEDROCK REFUSAL AT 70.0' FILL TO 4.0' SLIGHT CAVING GROUNDWATER AT 39.0'		×		50/4"	9.6	134	Ring
BORING		-	END OF BORING		_		50/1"	N.R.	N.R.	Ring

PROPOSED HOSPITAL EXPANSION AND PARKING GARAGEOD No. RIVERSIDE, CALIFORNIA

07881-3

Enclosure B-1b

Date Drilled: 12/20/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 800.0

Logged by: VJR

Measured Depth to Water(ft): 36.0

	DEPTH (ft)	GRAPHIC	VISUAL CLASSIFICATION	REMARKS	DRIVE	BULK	BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
-			(ML) Sandy Silt, fine to medium with coarse and gravel to 1", light brown		X		5 6 4			SA, MDE, SE SPT
-	5 -		(ML) Sandy Silt, fine with medium, light brown	Native	X		3 2 2	7.9		SPT
-	10 - - - - - 15 -		(SP-SM) Sand, medium to coarse with fine, silt and gravel to 1", brown	-	X	***	3 3 5	3.3		SPT
	-	-	(SP) Sand, medium to coarse with fine and gravel to 1",		X		5 6 6			SPT
GPJ CHJ.GDT 1/18/08	20 -	- - - - -	brown		X		19 24 19			SPT
BLOW PER 6 IN 07881-3	25 - - - -				\times		41 50/5"			SPT
BORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3, GPJ CHJ, GDT 1/18/08	30 -		(SP-SM) Sand, fine to coarse with silt and gravel to 1", light brown		><		50			SPT



PROPOSED HOSPITAL EXPANSION AND PARKING GARAGEOD No. 07881-3 RIVERSIDE, CALIFORNIA

Enclosure B-2a

Date Drilled: 12/20/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 800.0

Logged by: VJR

Measured Depth to Water(ft): 36.0

DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	DRIVE	BULK	BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
- - - 40	-	(SP-SM) Sand, fine to coarse with silt and gravel to 1", light brown	Groundwate	X		19 23 32			SPT
- 45		END OF BORING NO BEDROCK REFUSAL AT 43.0' FILL TO 4.0' SLIGHT CAVING		×		50/5"			SPT
- - 50		GROUNDWATER AT 36.0'							
143.6PJ CHJ.GDT 1/18/08									
BORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ.GDT 1/1808		S4							
BORING LO									

C.H.J.

PROPOSED HOSPITAL EXPANSION AND PARKING GARAGEOb No. RIVERSIDE, CALIFORNIA 07881-3

6. Enclosure B-2b

Date Drilled: 12/26/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 839.0

Logged by: JMZ

Measured Depth to Water(ft): 76.7

	DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	DRIVE	BULK BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
	12	- - -	Asphalt Base (SM) Silty Sand, fine with gravel to 3", red brown	Fill	X	7 6 12	7.3 7.8	107	Ring
-	5 -	-	(SM) Silty Sand, fine, light brown	Native	X	12 20 31	14.1	106	Ring
-	10 -				8	9 13 17	10.8	120	Ring
	15 -						10.0	120	King
18/08	20 -				X	9 16 39	15.0	117	Ring
31-3.GPJ CHJ.GDT 1	25 -	-	(ML) Sandy Silt, fine, brown		X	11 13 18	12.8	113	Ring
BLOW PER 6 IN 078	3	-	(1712) Salidy Sile, Ilito, Storia		X	13 19 23	24.4	102	Ring
BORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ.GDT 1/18/08	30 -	-	(ML) Sandy Silt, fine with clay, red brown		X	13 18 30	13.6	122	Ring
BORIN									

€ C.H.J.

PROPOSED HOSPITAL EXPANSION AND PARKING GARAGEO No.
RIVERSIDE, CALIFORNIA 07881-3

Enclosure B-6a

Date Drilled: 12/26/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 839.0

Logged by: JMZ

Measured Depth to Water(ft): 76.7

DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	DRIVE	BULK	BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
-		(SC) Clayey Sand, fine to medium with silt, red brown		X	*****	10 18 35	10.8	125	Ring
- 40		(SM) Silty Sand, fine to medium with coarse and gravel to 3/4", light brown		X		17 30 50	10.0	126	Ring
- 45	_	(SP-SM) Sand, fine to coarse with silt and gravel to 1", light brown		X		48 43	1.8	Dist.	Ring
- 50		(SP) Sand, fine to medium with coarse and gravel to 1", light brown		X		40	N.R.	N.R.	Ring
- 55		(SP) Sand, fine to coarse with gravel and cobbles to 4", light brown	ž.			50/5"		0.212004	
07881-3.GPJ CHJ.GD						50	N.R.	N.R.	Ring
S C C C C C C C C C C C C C C C C C C C	-			×		50/4"	3.0	112	Ring
BORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ.GDT 1/18/08				×		50	3.3	96	Ring

PROPOSED HOSPITAL EXPANSION AND PARKING GARAGEOD No. 07881-3 RIVERSIDE, CALIFORNIA

Enclosure B-6b

Date Drilled: 12/26/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 839.0

Logged by: JMZ

Measured Depth to Water(ft): 76.7

DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	DRIVE	BULK	BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
-		(SP) Sand, fine to coarse with gravel and cobbles to 4", light brown		×		50/4"	4.2	105	Ring
- 75 -		(SM) Silty Sand, fine to coarse with clay and gravel 3", brown	Ğroundwate	×		50/5"	9.6	114	Ring
- 80 -		END OF BORING	_	×		50/2"	N.R.	N.R.	Ring
- 85 - -		NO BEDROCK REFUSAL AT 83.0' FILL TO 5.0' SLIGHT CAVING GROUNDWATER AT 76.7'							
SPJ CHJ.GDT 1/18/08									
LOW PER 6 IN 07881-3.									9
BORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ.GDT 1/18/08	-								



PROPOSED HOSPITAL EXPANSION AND PARKING GARAGEOD No. RIVERSIDE, CALIFORNIA

07881-3

B-6c

Enclosure

Date Drilled: 12/26/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 825.0

Logged by: JMZ

Measured Depth to Water(ft): N.A.

DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	DRIVE	BULK	BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
-		Asphalt Base (SM) Silty Sand, fine to medium with coarse and gravel to 3", brown	Fill				6.5		
- - 5 -		(SM) Silty Sand, fine to coarse with gravel to 1", brown	Native	X		6 8 8	5.8		Cor. SPT
- 10 -	-			X		3 3 3			SPT
- 15 -				X		2 4 3			SPT
- 20 -		(SP-SM) Sand, fine to medium with coarse and silt, light brown		X		2 3 5			SPT
ER 6 IN 07881-3.GPJ CH.		(SP-SM) Sand, fine to coarse with silt and gravel to 1", light brown		X		6 8 11			SPT
BORING LOG-NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ.GDT 1/18/08		(SP) Sand, fine to coarse, light brown		X		8 12 19			SPT
BORING LO		(SP) Sand, fine to coarse with gravel to 1", light brown							

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PROPOSED HOSPITAL EXPANSION AND PARKING GARAGEOb No. RIVERSIDE, CALIFORNIA 07881-3

Enclosure B-7a