Appendix D

Percolation Testing

GEOTECHNICAL PROFESSIONALS INC.

December 20, 2019

Magnolia Partnership LLC
1201 Dove Street, Suite 520
Newport Beach, California 92660

Attention: Mr. Todd Cadwell

Subject: Percolation/Infiltration Testing
Proposed Apartment and Retail Development
Magnolia Flats
NEC Magnolia Avenue and Banbury Drive
Riverside, California
GPI Project No. 2917.1I

Dear Mr. Cadwell:

This report presents the results of planning phase percolation testing performed by Geotechnical Professionals Inc. (GPI) for stormwater disposal systems for the subject site. Our scope of services was presented in our proposal dated October 17, 2019. The testing was performed in accordance with the County of Riverside guidelines (Reference 1). We provided a Feasibility-Level Investigation report for the subject project site (Reference 2), dated February 18, 2019.

The location of the site is shown on the Site Location Map, Figure 1. The fieldwork described in this report was performed as part of our concurrent geotechnical investigation for the planned development.

Project Description

The project includes constructing a new mixed-use apartment development at the subject site. The apartment portion of the development consists of 4-story wood framed structures surrounded by at-grade parking. Carports are planned for a substantial portion of the at-grade parking. The apartment buildings will be at-grade surrounding courtyards and a swimming pool area. A shops building with retail and restaurants will be located in front of the development adjacent to Magnolia Avenue. A park is planned at the north portion of the site beyond at-grade apartment parking. The preliminary site configuration is shown on Figure 2, Site Plan.

Based on data from our feasibility investigation, we determined that infiltration of storm water is not feasible in the upper 30 feet of the soil profile due to the anticipated adverse impacts to the site soils or mounding of groundwater. Based on discussions

5736 Corporate Avenue • Cypress, CA 90630 • (714) 220-2211 , FAX (714) 220-2122

with KHR, three potential locations for the dry well stormwater infiltration systems have been identified.

Scope of Services

Our scope of services for the percolation/infiltration testing consisted of installation of six test wells at the three different locations, field percolation testing, and the preparation of this summary report. The wells were installed as part of a comprehensive geotechnical investigation that included ten exploratory borings and preparation of a full geotechnical report to be issued at a later date.

Subsurface Exploration

The borings were drilled using truck-mounted hollow-stem auger drill equipment at the locations shown on Figure 2, Site Plan. Relatively undisturbed samples were obtained using a brass-ring lined sampler (ASTM D 3550). Bulk (disturbed) samples were also obtained using a split-spoon sampler by means of the Standard Penetration Test (SPT, ASTM D 6066). The field explorations for the investigation were performed under the continuous technical supervision of GPI's representative, who visually inspected the site, maintained detailed logs of the borings, classified the soils encountered, and obtained relatively undisturbed samples for examination and laboratory testing.

The soils encountered in the borings were classified in the field and through further examination in the laboratory in general accordance with the Unified Soils Classification System. The logs of Borings B-4, B-7, and B-10, drilled within the limits of the proposed infiltration areas and near percolation test wells P-1 through P-6, are attached.

Laboratory Testing

We performed laboratory tests to determine the percent fines (silts and clays) for selected samples of the on-site soils in accordance with ASTM D 422. The results are shown in the table below.

Fines Content of Selected Samples

BORING NO.	DEPTH (ft)	SOIL DESCRIPTION	PERCENT PASSING No. 200 SIEVE
B-4	37	Sandy Silt (ML)	59
B-4	41	Sand with Silt (SP-SM)	6
B-7	37	Sand with Silt (SP-SM)	8
B-7	41	Silty Sand (SM)	42
B-7	45	Sand (SP)	4
B-10	37	Sandy Silt (SP-SM)	45
B-10	41	Silt (ML)	96
B-10	45 (Upper)	Clayey Sand (SC)	42
B-10	45 (Lower)	Sand with Silt (SP-SM)	7

Subsurface Soil Conditions

In the exploratory borings adjacent to the percolation wells, we predominately encountered layered sands, and silty sands at depths below 35 feet from existing grade. The sands and silty sands were generally medium dense to dense and dry to slightly moist. A layer of silt was encountered in B-7 and B-10 from at a depth of approximately 40 feet below existing grade. The silt was very stiff and wet. Detailed descriptions of the subsurface conditions encountered are shown on the Logs of Borings, Figures 3 to 6.

Groundwater was encountered in one of our exploratory borings at a depth of approximately 57 feet below existing ground surface. The historical high groundwater has not been determined in the area by the State of California.

Percolation Test Wells

Test wells, P-1 thru P-6, were installed in boreholes drilled using truck-mounted hollow-stem auger drill equipment. The wells consisted of 2-inch diameter PVC casing installed in an 8-inch diameter borehole. The casing was perforated in the lower 2 feet of the wells. Packing material around the slotted sections of the well casing consisted of #3 sand. The test wells were constructed to depths of approximately 35 to 40 feet below existing grade. The percolation testing was performed in general accordance with the County of Riverside requirements (Reference 1).

The test wells were filled with water the day before testing and presoaked at least 15 hours. Prior to running the tests in the wells, we conducted the Sandy Soil Criteria Test by filling the test holes as described in the Reference 1 and taking measurements of the water levels. The Sandy Soil Criteria was achieved in each of the wells. Two consecutive measurements showed that at least 6 inches of water seeped away in less than 25 minutes, therefore the test was continued for at least 1 hour as described in the criteria.

The percolation testing was conducted as outlined in the County guidelines for ‘sandy soils’ for at least six consecutive 10-minute readings. The initial water heights in the test wells were on the order of 2 to 4 feet above the bottom of the test well. After each reading, the water level was raised to approximately the same height. Details of the percolation tests are presented in the attached Tables 1 through 6, Borehole Infiltration Test Results.

The measured infiltration rates were calculated using the drop in water level over the test increment time and corrected using the Porchet Method. The final measured rates for each well, corrected as indicated above are presented in the following table and should be used with an appropriate factor of safety.

Infiltration Test Results Summary

TEST WELL	APPROXIMATE DEPTH OF TEST WELL (feet)	CORRECTED INFILTRATION RATE (in./hr.)
P-1	41	5.5
P-2	40	6.0
P-3	35	3.9
P-4	35	2.5
P-5	40	0.7
P-6	40	4.6

After completion of the infiltration testing, a portion of the well casings were removed, and the holes were backfilled with the on-site soils.

Conclusions and Recommendations

Based on the results of the percolation testing, the subsurface soils at the depths tested (35 to 40 feet below grade) at the site are suitable for dry well infiltration based on the measured field permeability. At the northwest portion of the site, we observed infiltration rates ranging from 5.5 to 6.0 inches per hour. At the center portion of the site, we observed infiltration rates ranging from 2.5 to 3.9 inches per hour, and at the southwest portion of the site, we observed infiltration rates ranging from 0.7 to 4.6 inches per hour.

Before performing percolation testing at P-5, a water line was broken causing significant flooding of the well. We assume that fine grained soils may have flowed into the well which negatively impacted the infiltration rate. We believe this may have skewed the results of our infiltration testing, and that the rate for P-5 may be excluded when designing the infiltration system.

The County of Riverside guidelines require a factor of safety of 2 be applied to the measured infiltration rates. If the corrected measured infiltration rate is greater than 0.3 inches per hour, the soils are considered potentially feasible.

The Project Civil Engineer should determine a “design infiltration rate” using the technical guidelines of the County of Riverside. The County requires a factor of safety to determine the design infiltration rate of not less than 2.0, but it may be higher at the discretion of the design engineer and acceptance of the plan reviewer.

Infiltration of storm water shallower than the depths tested is not acceptable due to collapsible soils and potential mounding due to clay layers as discussed in Reference 2.

It should be noted that this infiltration rate is for clean, clear water and does not include any effects of sediment, fines, dissolved solids or any other debris as the materials will significantly reduce the percolation rates of the subsurface soils.

LIMITATIONS

The report, exploration logs, and other materials resulting from GPI's efforts were prepared exclusively for use by Magnolia Partnership LLC and their consultants in designing the proposed development. The report is not suitable for a project other than the currently proposed development.

Soil deposits may vary in type, strength, and many other important properties between points of exploration due to non-uniformity of the geologic formations or to man-made cut and fill operations. While we cannot evaluate the consistency of the properties of materials in areas not explored, the conclusions drawn in this report are based on the assumption that the data obtained in the field and laboratory are reasonably representative of field conditions and are conducive to interpolation and extrapolation.

Our investigation and evaluations were performed using generally accepted engineering approaches and principles available at this time and the degree of care and skill ordinarily exercised under similar circumstances by reputable Geotechnical Engineers practicing in this area. No other representation, either express or implied, is included or intended in our report.

Respectfully submitted,
Geotechnical Professionals Inc.

Patrick I.F. McGervey, P.E.
Staff Engineer

Donald A. Cords, G.E.
Principal

Enclosures:
Figure 1	-	Site Location Plan
Figure 2	-	Site Plan
Figures 3 to 5	-	Logs of Borings
Table 1	-	Borehole Infiltration Test Results

Distribution: Addressee (e-mail only)
Gave Uribe, KHR Associates (e-mail only)

REFERENCES

1. County of Riverside, “Infiltration Testing”, Appendix A, Low Impact Development BMP Design Handbook, rev. September 2011

2. Geotechnical Professionals, Inc., “Feasibility-Level Investigation, Proposed Mixed-Use Apartment Development, NEC Magnolia Avenue and Banbury Drive, Riverside, California,” GPI Project No. 22924.I, dated February 18, 2019.

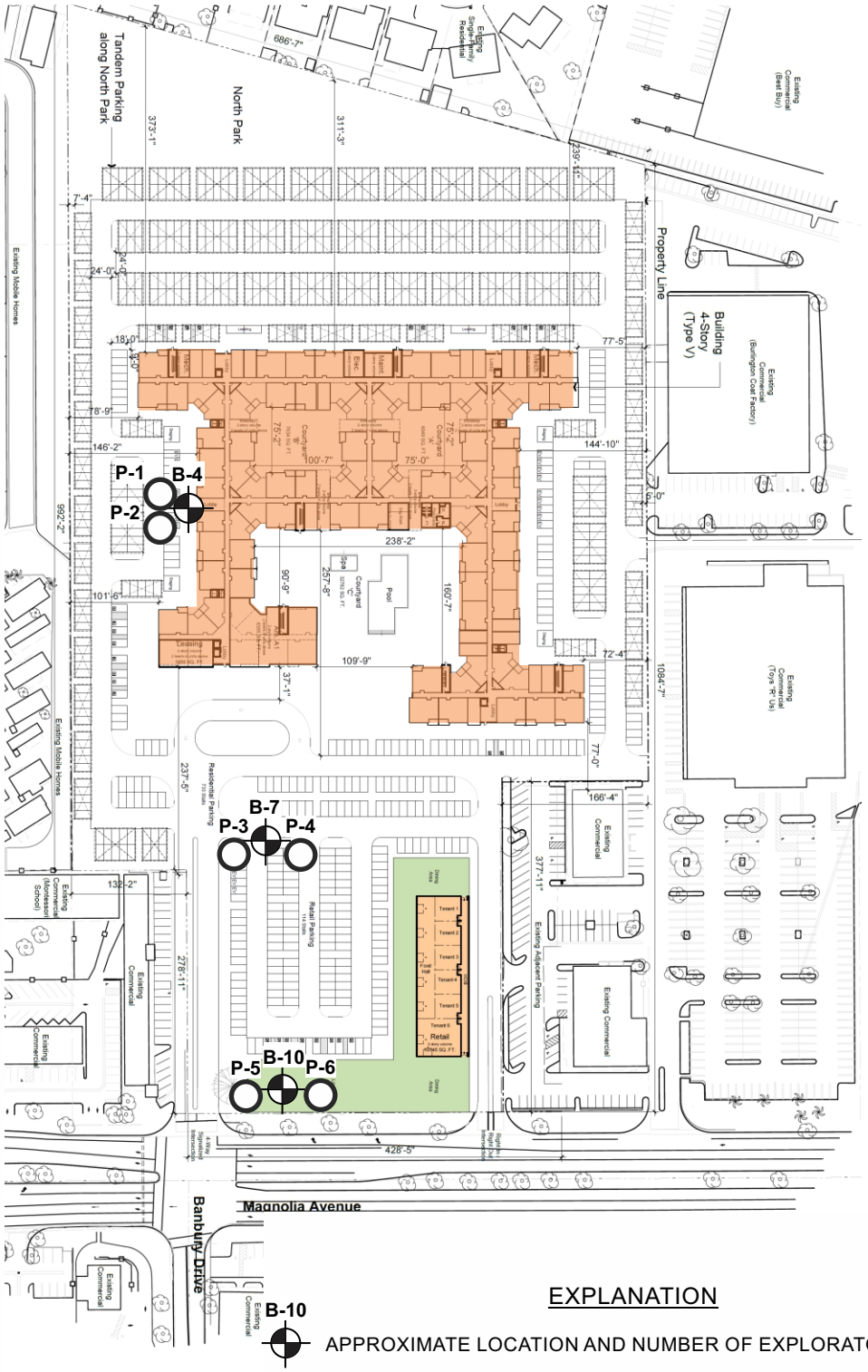
GEOTECHNICAL PROFESSIONALS, INC.

MAGNOLIA AVENUE & BANBURY DRIVE

GPI PROJECT NO.:2924.I | SCALE: 1" = 2000'

SITE LOCATION MAP

FIGURE 1



EXPLANATION

B-10 — APPROXIMATE LOCATION AND NUMBER OF EXPLORATORY BORING

P-6 — APPROXIMATE LOCATION AND NUMBER OF PERCOLATION TEST

N ACTUAL NORTH

N PROJECT NORTH

0 200 400 FEET

BASE PLAN REPRODUCED FROM SITE PLAN, CONCEPTUAL DESIGN, PRODUCED BY KTGY ARCHITECTURE, DATED SEPTEMBER 18,2019

GPI GEOTECHNICAL PROFESSIONALS, INC.

MAGNOLIA FLATS

GPI PROJECT NO.:2924.I | SCALE: 1" = 200'

SITE PLAN

FIGURE 2

MOISTURE (%)	DRY DENSITY (PCF)	PENETRATION RESISTANCE (BLOWS/FOOT)	SAMPLE TYPE	DEPTH (FEET)	*DESCRIPTION OF SUBSURFACE MATERIALS* This summary applies only at the location of this boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.	ELEVATION (FEET)
			B	0	Pulverized concrete 8-Inch over silty sand, brown, moist Fill: **SILTY SAND (SM)** brown, moist	
				5	5 to 35 feet, not sampled	730
				10		725
				15		720
				20		715
				25		710
				30		705
21.9		22	S	35	Natural: **CLAYEY SILT (ML)** brown, very moist, very stiff	700
19.9		19	S		**SANDY SILT (ML)** brown, wet, very stiff	
2.4		42	S		**SAND (SP)** light brown, dry to slightly moist, dense	

SAMPLE TYPES
- C Rock Core
- S Standard Split Spoon
- D Drive Sample
- B Bulk Sample
- T Tube Sample

DATE DRILLED: 10-31-19

EQUIPMENT USED: 8 " Hollow Stem Auger

GROUNDWATER LEVEL (ft): Not Encountered

GPI

PROJECT NO.: 2924.1I

MAGNOLIA FLATS

LOG OF BORING NO. B-4

FIGURE 3

	MOISTURE (%)	DRY DENSITY (PCF)	PENETRATION RESISTANCE (BLOWS/FOOT)	SAMPLE TYPE	DEPTH (FEET)	DESCRIPTION OF SUBSURFACE MATERIALS This summary applies only at the location of this boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.	ELEVATION (FEET)
					40		
	4.0		29	S		**SAND WITH SILT (SP-SM)** light brown, dry, dense, medium to coarse grained @ 41 feet, medium dense	
	2.3		28	S			
	2.7		34	S	45	@ 45 feet, dense, coarse grained	690
	9.9		38	S		@ 47 feet, fine to medium grained, trace gravel, 3-Inch lens of silt	
	9.4	99	77	D	50	@ 50 feet, very dense, sandy silt @ tip	685
						Total Depth 51 feet	

SAMPLE TYPES
- C Rock Core
- S Standard Split Spoon
- D Drive Sample
- B Bulk Sample
- T Tube Sample

DATE DRILLED: 10-31-19

EQUIPMENT USED: 8 " Hollow Stem Auger

GROUNDWATER LEVEL (ft): Not Encountered

GPI

PROJECT NO.: 2924.1I

MAGNOLIA FLATS

LOG OF BORING NO. B-4

FIGURE 3

MOISTURE (%)	DRY DENSITY (PCF)	PENETRATION RESISTANCE (BLOWS/FOOT)	SAMPLE TYPE	DEPTH (FEET)	DESCRIPTION OF SUBSURFACE MATERIALS	ELEVATION (FEET)
					This summary applies only at the location of this boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.	
			B	0	1-Inch AC Fill: **SILTY SAND (SM)** brown, moist	730
				5	5 to 35 feet, not sampled	725
				10		720
				15		715
				20		710
				25		705
				30		700
2.0		27	S	35	**SAND WITH SILT (SP-SM)** light brown, dry to slightly moist, medium dense, coarse grained	
3.4		28	S		@ 37 feet, medium to coarse grained	695
3.9		19	S			

SAMPLE TYPES
- C Rock Core
- S Standard Split Spoon
- D Drive Sample
- B Bulk Sample
- T Tube Sample

DATE DRILLED: 10-30-19

EQUIPMENT USED: 8 " Hollow Stem Auger

GROUNDWATER LEVEL (ft): 57

GPI

PROJECT NO.: 2924.1I

MAGNOLIA FLATS

LOG OF BORING NO. B-7

FIGURE 4

MOISTURE (%)	DRY DENSITY (PCF)	PENETRATION RESISTANCE (BLOWS/FOOT)	SAMPLE TYPE	DEPTH (FEET)	DESCRIPTION OF SUBSURFACE MATERIALS	ELEVATION (FEET)
					This summary applies only at the location of this boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.	
23.8				40	**SILT (ML)** brown, wet, very stiff	
20.0		22	S		**SILTY SAND (SM)** light brown, wet, medium dense, fine to medium grained	
						690
4.2		31	S		**SAND (SP)** light brown, dry to slightly moist, dense, coarse grained	
				45	@ 44 feet, 4-Inch lens of silty sand	
2.3		43	S			
2.6		31	S			
						685
				50		
3.7		43	S			
						680
				55		
5.2	125	71	D		**SANDY SILT (ML)** brown, dry, hard	
					SAND (SP) light brown, slightly moist, very dense, coarse grained	
						675
				60		
32.8	93	88/9"	D		**SILT (ML)** brown, wet, hard	
					SAND (SP) orange brown, wet, very dense	
					Total Depth 61 feet	

SAMPLE TYPES
- C Rock Core
- S Standard Split Spoon
- D Drive Sample
- B Bulk Sample
- T Tube Sample

DATE DRILLED: 10-30-19

EQUIPMENT USED: 8 " Hollow Stem Auger

GROUNDWATER LEVEL (ft): 57

GPI

PROJECT NO.: 2924.1I

MAGNOLIA FLATS

LOG OF BORING NO. B-7

FIGURE 4

MOISTURE (%)	DRY DENSITY (PCF)	PENETRATION RESISTANCE (BLOWS/FOOT)	SAMPLE TYPE	DEPTH (FEET)	DESCRIPTION OF SUBSURFACE MATERIALS This summary applies only at the location of this boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.	ELEVATION (FEET)
			B	0	3-Inch AC over silty sand, brown, slightly moist Fill: **SILTY SAND (SM)** brown, slightly moist	
				5	5 to 35 feet, not sampled	730
				10		725
				15		720
				20		715
				25		710
				30		705
				35		700
2.8		23	S		Natural: **SAND (SP)** light brown, dry to slightly moist, medium dense, medium to coarse grained	
16.5		15	S		**SILTY SAND (SM)** brown, wet, medium dense	
1.7		19	S		**SAND (SP)** light brown, dry, medium dense	695

SAMPLE TYPES
- C Rock Core
- S Standard Split Spoon
- D Drive Sample
- B Bulk Sample
- T Tube Sample

DATE DRILLED: 10-30-19

EQUIPMENT USED: 8 " Hollow Stem Auger

GROUNDWATER LEVEL (ft): Not Encountered

GPI

PROJECT NO.: 2924.1I

MAGNOLIA FLATS

LOG OF BORING NO. B-10

FIGURE 5

MOISTURE (%)	DRY DENSITY (PCF)	PENETRATION RESISTANCE (BLOWS/FOOT)	SAMPLE TYPE	DEPTH (FEET)	DESCRIPTION OF SUBSURFACE MATERIALS	ELEVATION (FEET)
				40	This summary applies only at the location of this boring and at the time of drilling. Subsurface conditions may differ at other locations and may change at this location with the passage of time. The data presented is a simplification of actual conditions encountered.	
34.2		10	S		**SILT (ML)** grey, wet, very stiff	
11.0		21	S		@ 43 feet, 4-Inch lens of grey sand	690
18.6		35	S	45	**CLAYEY SAND (SC)** grey, wet, dense	
4.2					**SAND WITH SILT (SP-SM)** grey, slightly moist, dense, coarse grained	
3.4		39	S		@ 47 feet, trace gravel	685
3.1	112	64	D	50		
					Total Depth 51 feet	

SAMPLE TYPES
- C Rock Core
- S Standard Split Spoon
- D Drive Sample
- B Bulk Sample
- T Tube Sample

DATE DRILLED: 10-30-19

EQUIPMENT USED: 8 " Hollow Stem Auger

GROUNDWATER LEVEL (ft): Not Encountered

GPI

PROJECT NO.: 2924.1I

MAGNOLIA FLATS

LOG OF BORING NO. B-10

FIGURE 5

TABLE 1

BOREHOLE INFILTRATION TEST RESULTS (corrected with Porchet Method)

Riverside County Method-TGD, 2011

Project No. 2924.I Project Name: Magnolia Flats Date: 12/20/2019

NOTE: Slowest rate from percolation testing used to calculate infiltration rate

Test Well	Test Duration (min)	Water Depth Initial (ft)	Water Depth Final (ft)	Total Depth of Hole (ft)	Hole Diameter (inches)	Initial Water Height (ft)	Final Water Height (ft)	Change in Height of Water (ft)	Average Height of Water (ft)	Infiltration Rate (in/hr)
	Δt	D_o	D_f	D_T	d	H_o	H_f	$\Delta H=\Delta D$	H_{avg}	I_t
P-1	10	36.80	38.37	40.80	8	4.00	2.43	1.57	3.22	5.6
P-1	10	36.80	38.37	40.80	8	4.00	2.44	1.565	3.22	5.5
P-1	10	36.80	38.36	40.80	8	4.00	2.44	1.56	3.22	5.5
P-1	10	36.80	38.36	40.80	8	4.00	2.44	1.56	3.22	5.5
P-1	10	36.80	38.37	40.80	8	4.00	2.44	1.565	3.22	5.5
P-1	10	36.80	38.36	40.80	8	4.00	2.45	1.555	3.22	5.5

Test Date 10/24/2018 NOTE: Slowest rate from percolation testing used to calculate infiltration rate

Test Well	Test Duration (min)	Water Depth Initial (ft)	Water Depth Final (ft)	Total Depth of Hole (ft)	Hole Diameter (inches)	Initial Water Height (ft)	Final Water Height (ft)	Change in Height of Water (ft)	Average Height of Water (ft)	Infiltration Rate (in/hr)
	Δt	D_o	D_f	D_T	d	H_o	H_f	$\Delta H=\Delta D$	H_{avg}	I_t
P-2	10	36.45	38.11	40.45	8	4.00	2.34	1.66	3.17	6.0
P-2	10	36.45	38.11	40.45	8	4.00	2.35	1.655	3.17	5.9
P-2	10	36.45	38.10	40.45	8	4.00	2.35	1.65	3.18	5.9
P-2	10	36.45	38.11	40.45	8	4.00	2.34	1.66	3.17	6.0
P-2	10	36.45	38.11	40.45	8	4.00	2.35	1.655	3.17	5.9
P-2	10	36.45	38.12	40.45	8	4.00	2.34	1.665	3.17	6.0

TABLE 1 (Continued)

BOREHOLE INFILTRATION TEST RESULTS (corrected with Porchet Method)

Riverside County Method-TGD, 2011

Project No. 2924.I Project Name: Magnolia Flats Date: 12/20/2019

NOTE: Slowest rate from percolation testing used to calculate infiltration rate

Test Well	Test Duration (min)	Water Depth Initial (ft)	Water Depth Final (ft)	Total Depth of Hole (ft)	Hole Diameter (inches)	Initial Water Height (ft)	Final Water Height (ft)	Change in Height of Water (ft)	Average Height of Water (ft)	Infiltration Rate (in/hr)
	Δt	D_o	D_f	D_T	d	H_o	H_f	$\Delta H=\Delta D$	H_{avg}	I_t
P-3	10	33.42	34.07	35.40	8	1.98	1.33	0.65	1.66	4.3
P-3	10	33.40	33.97	35.40	8	2.00	1.44	0.565	1.72	3.6
P-3	10	33.40	34.01	35.40	8	2.00	1.39	0.61	1.70	3.9
P-3	10	33.40	34.01	35.40	8	2.00	1.39	0.61	1.70	3.9
P-3	10	33.40	34.01	35.40	8	2.00	1.39	0.61	1.70	3.9
P-3	10	33.40	34.01	35.40	8	2.00	1.39	0.61	1.70	3.9

Test Date 10/24/2018 NOTE: Slowest rate from percolation testing used to calculate infiltration rate

Test Well	Test Duration (min)	Water Depth Initial (ft)	Water Depth Final (ft)	Total Depth of Hole (ft)	Hole Diameter (inches)	Initial Water Height (ft)	Final Water Height (ft)	Change in Height of Water (ft)	Average Height of Water (ft)	Infiltration Rate (in/hr)
	Δt	D_o	D_f	D_T	d	H_o	H_f	$\Delta H=\Delta D$	H_{avg}	I_t
P-4	10	33.25	33.63	35.25	8	2.00	1.62	0.38	1.81	2.3
P-4	10	33.25	33.63	35.25	8	2.00	1.63	0.375	1.81	2.3
P-4	10	33.25	33.66	35.25	8	2.00	1.59	0.41	1.80	2.5
P-4	10	33.25	33.64	35.25	8	2.00	1.61	0.385	1.81	2.3
P-4	10	33.25	33.66	35.25	8	2.00	1.59	0.41	1.80	2.5
P-4	10	33.25	33.67	35.25	8	2.00	1.59	0.415	1.79	2.5

TABLE 1 (Continued)

BOREHOLE INFILTRATION TEST RESULTS (corrected with Porchet Method)

Riverside County Method-TGD, 2011

Project No. 2924.I Project Name: Magnolia Flats Date: 12/20/2019

NOTE: Slowest rate from percolation testing used to calculate infiltration rate

Test Well	Test Duration (min)	Water Depth Initial (ft)	Water Depth Final (ft)	Total Depth of Hole (ft)	Hole Diameter (inches)	Initial Water Height (ft)	Final Water Height (ft)	Change in Height of Water (ft)	Average Height of Water (ft)	Infiltration Rate (in/hr)
	Δt	D_o	D_f	D_T	d	H_o	H_f	$\Delta H=\Delta D$	H_{avg}	I_t
P-5	10	37.73	37.94	39.80	8	2.07	1.86	0.205	1.97	1.2
P-5	10	37.68	37.87	39.80	8	2.12	1.93	0.19	2.03	1.0
P-5	10	37.66	37.82	39.80	8	2.14	1.98	0.16	2.06	0.9
P-5	10	37.73	37.85	39.80	8	2.07	1.95	0.12	2.01	0.7
P-3	10	37.74	37.86	39.80	8	2.06	1.94	0.12	2.00	0.7
P-5	10	37.70	37.82	39.80	8	2.10	1.98	0.12	2.04	0.7

Test Date 10/24/2018 NOTE: Slowest rate from percolation testing used to calculate infiltration rate

Test Well	Test Duration (min)	Water Depth Initial (ft)	Water Depth Final (ft)	Total Depth of Hole (ft)	Hole Diameter (inches)	Initial Water Height (ft)	Final Water Height (ft)	Change in Height of Water (ft)	Average Height of Water (ft)	Infiltration Rate (in/hr)
	Δt	D_o	D_f	D_T	d	H_o	H_f	$\Delta H=\Delta D$	H_{avg}	I_t
P-6	10	37.70	38.50	39.97	8	2.27	1.47	0.8	1.87	4.7
P-6	10	37.80	38.55	39.97	8	2.17	1.42	0.75	1.80	4.6
P-6	10	37.80	38.56	39.97	8	2.17	1.42	0.755	1.79	4.6
P-6	10	37.80	38.56	39.97	8	2.17	1.42	0.755	1.79	4.6
P-6	10	37.80	38.55	39.97	8	2.17	1.42	0.75	1.80	4.6
P-6	10	37.80	38.55	39.97	8	2.17	1.42	0.75	1.80	4.6