

Scour Analysis of Box Springs Canyon at Quail Run

Prepared for:

SDH & Associates, Inc.

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Prepared by:



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TORY R. WALKER ENGINEERING

RELIABLE SOLUTIONS IN WATER RESOURCES

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CHAPTER 1 - BACKGROUND

1.1 Project Description

The proposed Project is the Quail Run Apartments, a multi-family residential development generally located at the northwest corner of Central Avenue and Quail Run Court. See Figure 1 below. The total project site encompasses approximately 31 acres, with the apartment complex being developed on approximately 16 acres of the southerly portions of APNs 253-240-020 and 253-240-028. The apartment complex will consist of 13 apartment buildings. The Project also includes 9.9 acres of common open space areas, of which approximately 7.1 acres are set aside as scenic open space. The remaining approximately 2.8 acres of common open space will be located throughout the complex for resident enjoyment. See Figure 2.

The Project is designed to integrate not only with the surrounding urban development, but also with the existing on-site biological resources, which will be preserved and enhanced. Amenities include two raised viewing decks and space with interpretive signage explaining the wildlife viewshed, a walking nature trail, bench seating and picnic tables throughout the apartment complex, a large open space turf area, pool and spa, a recreation center with a separate BBQ area, a covered pavilion, par-course exercise stations, an indoor workout room, and a business center at the project's leasing building..

The northern portion of the project site will be used as a borrow area for this Project. The Project's grading plan has been designed to completely avoid sensitive riparian vegetation as well as provide full mitigation for temporary impacts to willow-mulefat, woodland, mulefat scrub, riverine, and alluvial fan sage scrub habitat.

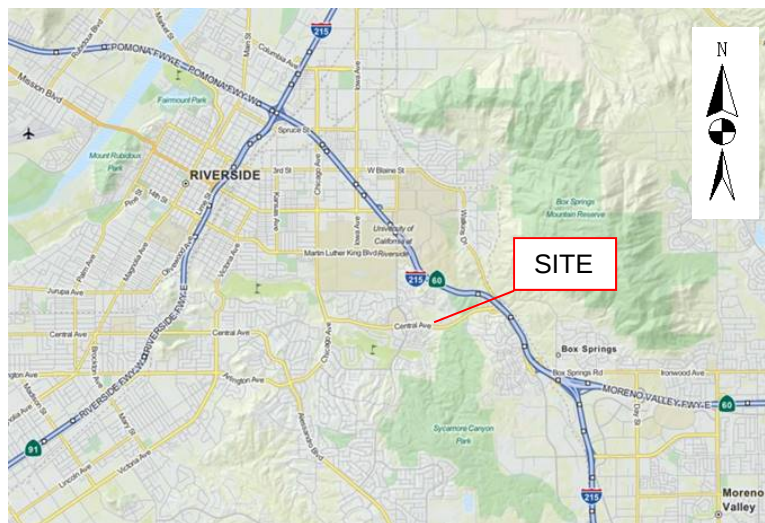


Figure 1. Vicinity Map

1.2 – Purpose of Study

Proposed grading associated with the Project would modify Box Springs Canyon topography, and thus flow patterns, within the flood detention area of the Box Springs Dam (see Figure 2). The purpose of this study is to determine the hydraulic impacts and potential scour due to the proposed grading within the detention area of the dam.



Figure 2. Proposed Site Exhibit

CHAPTER 2 – HYDRAULIC & SCOUR ANALYSIS

2.1 – Hydraulic Analysis

In the “Hydrologic and Hydraulic Analysis of Box Springs Dam” prepared by Tory R. Walker Engineering (TRWE) on March 30, 2015, a hydrology study of the 4 square mile tributary watershed analyzed 10-year 6-hour and 10-year 24-hour storms to determine peak flowrates into the Box Springs Dam. These are summarized in Table 1 below.

TABLE 1 – Peak Flows per Storm Event

Storm Event	Peak Flow (cfs)
10-year 6-hour	1554
10-year 24-hour	806

In the current study, we used these flowrates to hydraulically model flows in both the existing and proposed (with grading) conditions. We assumed worst-case scenarios associated with each storm event: for the 10-year 6-hour storm, we assumed the Box Springs Dam detention basin was empty; for the 10-year 24-hour storm, we assumed the detention basin was at the flood stage associated with that storm event.

We prepared HEC-RAS models for each condition based on current (2008) topography and the proposed grading plan for the Project. HEC-RAS cross sections were cut through the area of potential impact (see Figures 3 and 4), and appropriate hydraulic characteristics were assigned to each cross section.

Consistent with assumptions noted above, normal depth was assumed for the most downstream cross section in the 10-year 6-hour storms, and flood stage was assumed for the 10-year 24-hour storms.

This current study only analyzes the 10-year storms, as those are generally understood to be channel-forming events. Also, the 100-year storm events, modeled in the March 30, 2015 report, will result in the basin area being inundated prior to the arrival of peak flows, so potential scour will be better represented by the 10-year storms.

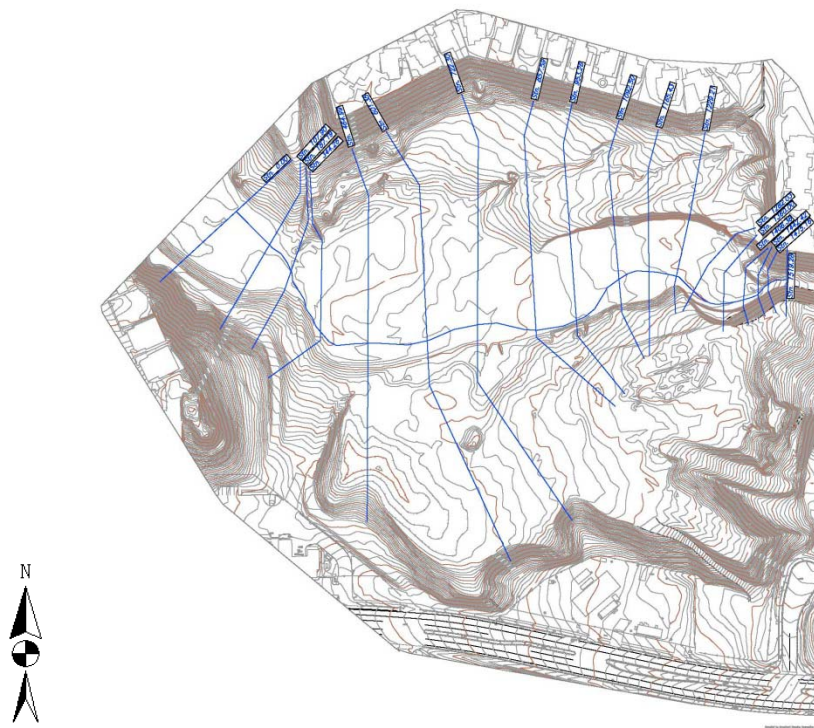


Figure 3. Pre-Project HEC-RAS Cross Sections

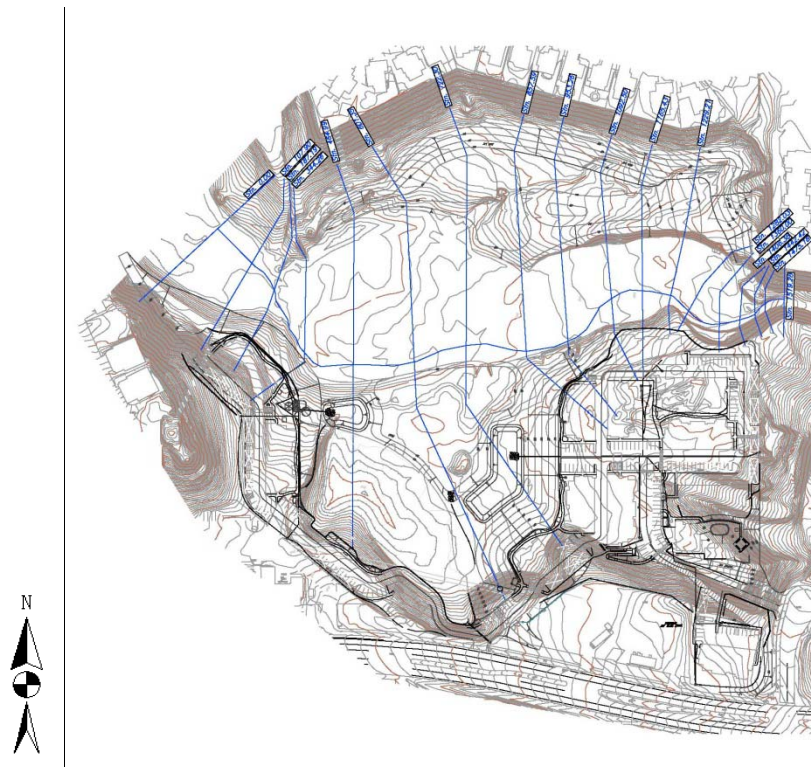


Figure 4. Post-Project HEC-RAS Cross Sections

2.2 – Results of Hydraulic Analysis

Results of the HEC-RAS hydraulic models are summarized in Tables 2 and 3 below. Detailed HEC-RAS results are contained in Appendix 2 and inundation maps are included in Appendix 1.

TABLE 2 – 10-year 6-hour HEC-RAS Results

River Stations	Water Surface Elevation (ft)		Average Channel Velocity (ft/s)	
	Existing	Proposed	Existing	Proposed
1519.28	1125.54	1125.54	15.67	15.67
1475.16	1121.65	1121.65	18.78	18.78
1442.42	1120.23	1120.23	18.10	18.10
1406.38	1118.97	1118.97	13.06	13.06
1360	1118.86	1118.90	5.83	5.72
1282.03	1117.66	1117.59	6.41	6.66
1229.21	1116.69	1116.39	6.23	6.46
1165.43	1115.63	1107.13	5.61	18.90
1092.5	1114.68	1108.83	5.09	4.35
953.26	1112.93	1107.55	5.02	6.13
857.59	1111.94	1107.29	4.51	3.30
722.3	1110.36	1107.18	3.38	2.07
601.35	1108.17	1107.09	2.81	1.34
464.99	1106.91	1106.83	1.78	1.45
344.26	1106.06	1106.13	3.06	2.96
187.19	1104.31	1104.35	6.56	6.40
107.9	1103.59	1103.52	6.26	6.63
0	1102.84	1102.68	4.90	5.43

TABLE 3 – 10-year 24-hour HEC-RAS Results

River Stations	Water Surface Elevation(ft)		Average Channel Velocity (ft/s)	
	Existing	Proposed	Existing	Proposed
1519.28	1124.29	1124.29	12.43	12.43
1475.16	1125.38	1125.38	3.93	3.93
1442.42	1125.47	1125.47	2.39	2.39
1406.38	1125.49	1125.49	1.02	1.02
1360	1125.50	1125.50	0.53	0.53
1282.03	1125.50	1125.50	0.51	0.46
1229.21	1125.50	1125.50	0.46	0.25
1165.43	1125.50	1125.50	0.37	0.14
1092.5	1125.50	1125.50	0.32	0.12
953.26	1125.50	1125.50	0.22	0.09
857.59	1125.50	1125.50	0.17	0.07
722.3	1125.50	1125.50	0.07	0.03
601.35	1125.50	1125.50	0.05	0.05
464.99	1125.50	1125.50	0.05	0.05
344.26	1125.50	1125.50	0.10	0.11
187.19	1125.50	1125.50	0.17	0.18
107.9	1125.50	1125.50	0.15	0.16
0	1125.50	1125.50	0.13	0.13

2.3 – Analysis of Results for Scour Potential

It is evident from the results in Tables 2 and 3 that the hydraulic impacts and potential scour due to the proposed grading are insignificant for all but Section 1165.43. One of the potential concerns with the proposed grading is the increased scour potential, especially along the north edge of the project area; the (one-dimensional) HEC-RAS model was prepared to hydraulically model the approximate behavior of the flow, including average velocities, which give an indication of scour potential.

A closer examination of the proposed grading at Sections 1165.43 and 1229.21 (immediately upstream) illustrates the limitation of a one-dimensional model, in that it cannot accurately represent the very localized unstable flow condition that will exist between these sections at the interface of the proposed grading and the existing topography, where the grade will drop several feet. Fortunately, a closer look at downstream sections, beginning with Section 1092.5 immediately downstream, gives a clearer picture of the actual flow characteristics at Section 1165.43. The average velocities of these sections are moderately erosive to non-erosive, indicating no need for scour protection along the north edge of the project area.

Another limitation of the one-dimensional hydraulic model is the inability to accurately represent the continuity of the flow in the existing channel. The model indicates all flow would take a sharp right turn, which of course it would not. Due to momentum, much of the flow will continue in the same direction, spreading out and spilling into the lower graded area over several sections.

Finally, as noted in the “Hydrologic and Hydraulic Analysis of Box Springs Dam” report, the gated outlet of the dam is at elevation 1106 feet, the same as the lowest elevation in Cross Section 1165.43. As there will very likely be several feet of ponded water in the detention basin when the 10-year 6-hour peak flow arrives (the report indicates an elevation of 1120 feet, or a depth of 14 feet), that ponded water will serve to dissipate the energy of potentially erosive flows.

CHAPTER 3 – CONCLUSION

As previously noted, the intent of this study was to model worst-case scenarios; in taking this approach, the modeling anomaly at HEC-RAS Cross Section 1165.43 therefore requires some reasonable explanation. A more complex two-dimensional model could more accurately represent these flow characteristics, but such a model is not warranted for this type of analysis. Indeed, in light of the mitigating factors discussed above, a more complex model is not necessary. The above analysis of results, which considers results from the upstream and downstream cross sections, the understanding of flow characteristics, and the operations of the dam is adequate evidence to conclude that the proposed grading will not pose a significant threat to scour within the detention basin area, including the north edge of the project area.

References

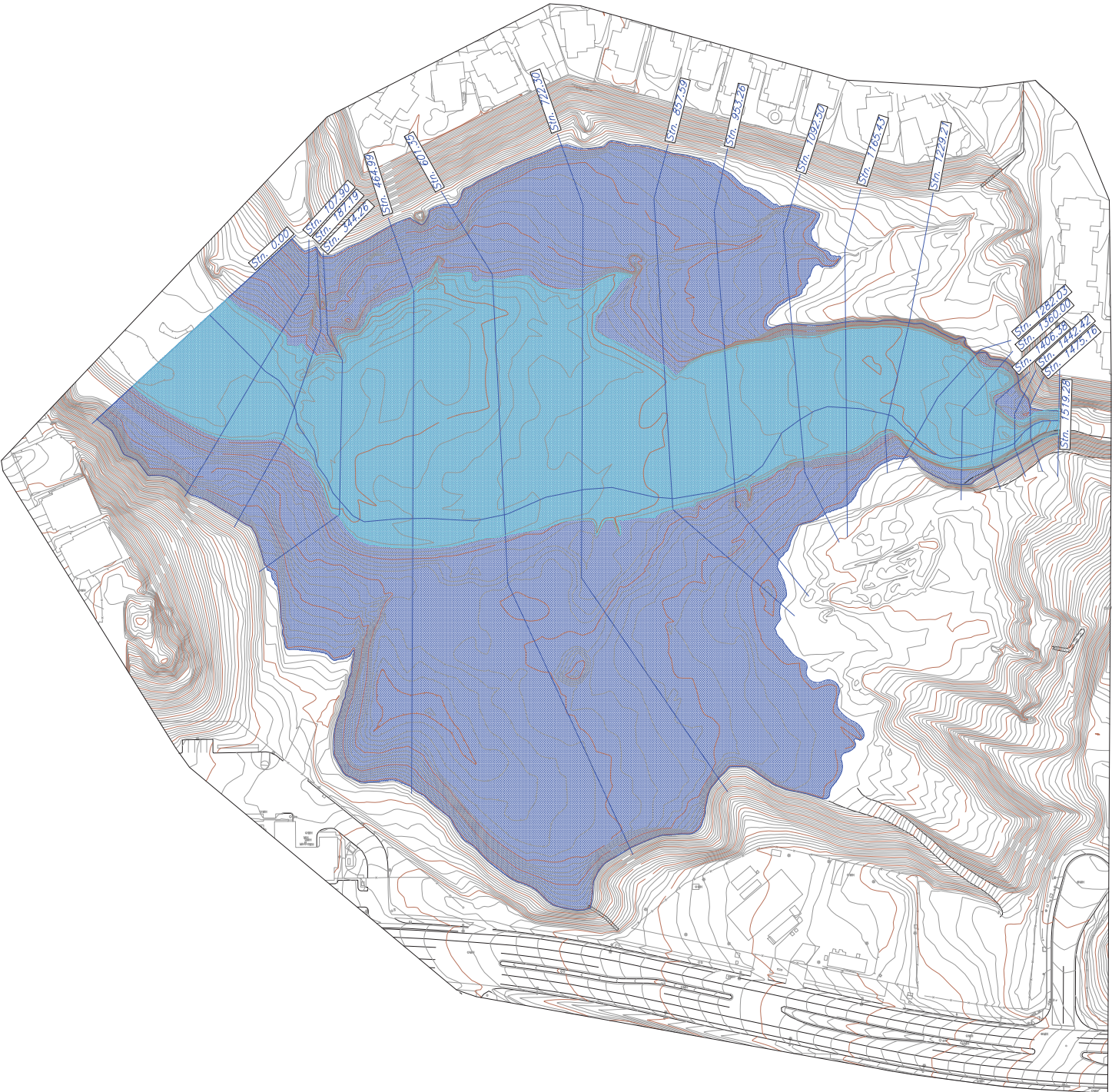
“Hydrologic and Hydraulic Analysis of Box Springs Dam” prepared by Tory R. Walker Engineering (TRWE) on March 30, 2015

APPENDIX 1

HEC-RAS Workmaps

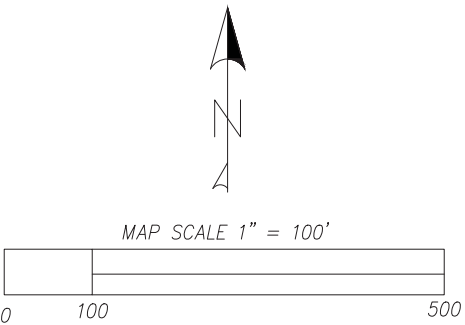
PRE-PROJECT HEC-RAS WORKMAP

TOPOGRAPHIC SOURCE:
1. Arrowhead Mapping Corporation – NOV 2014 – NAVD88



LEGEND

- HEC-RAS X-SECTION
- 10-YR, 6-HR INUNDATION
- 10-YR, 24-HR INUNDATION



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APPROVED CHANGES			
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Location: _____

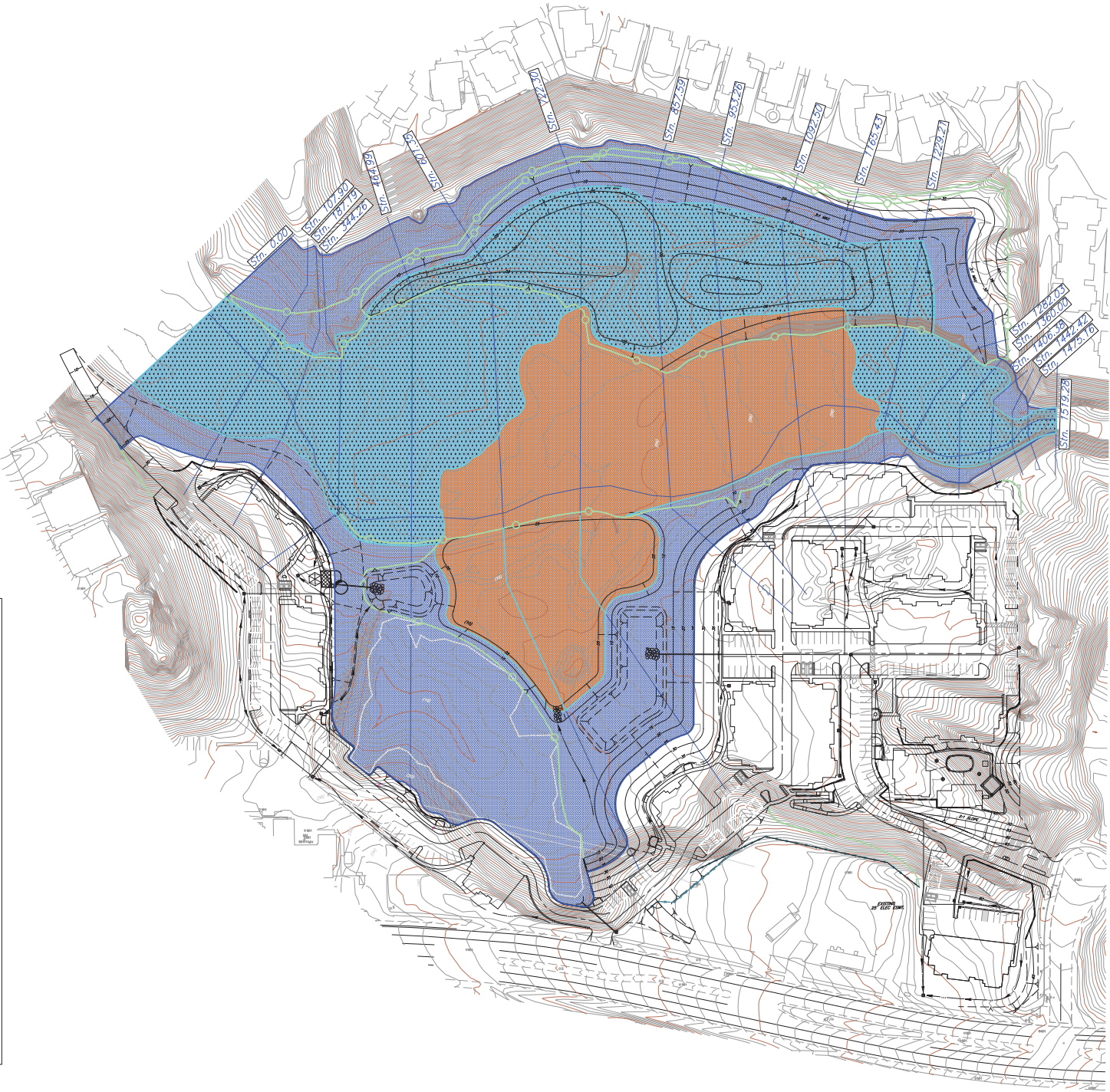
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Approved _____		
ENGINEER OF WORK ENGINEER RCE	Checked by	Approval date

POST-PROJECT HEC-RAS WORKMAP

TOPOGRAPHIC SOURCE:
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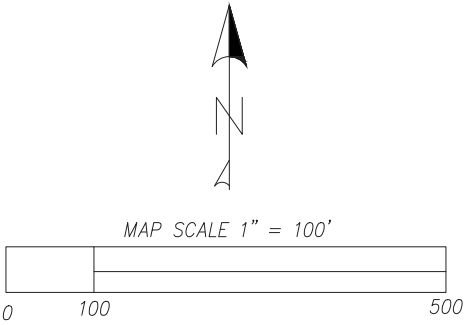
LEGEND

HEC-RAS
X-SECTION

10-YR, 6-HR
INUNDATION

10-YR, 24-HR
INUNDATION

10-YR, 6-HR
MODERATE FLOODING



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BENCH MARK

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Location: _____

Record From: _____

Elev: _____ Datum: _____

SHEET
1

CITY OF RIVERSIDE

1
SHEETS

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QUAIL RUN

Approved

ENGINEER OF WORK

ENGINEER RCE

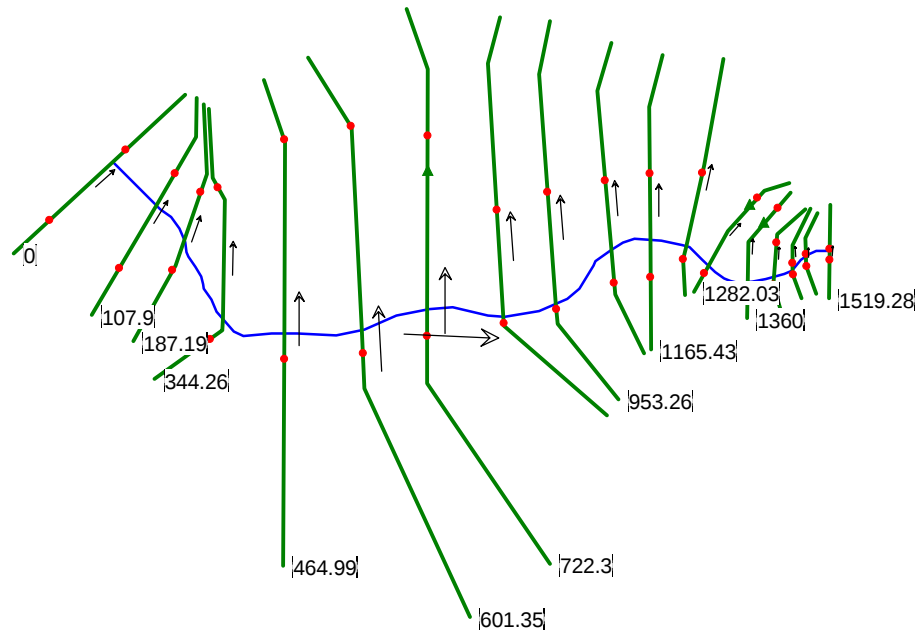
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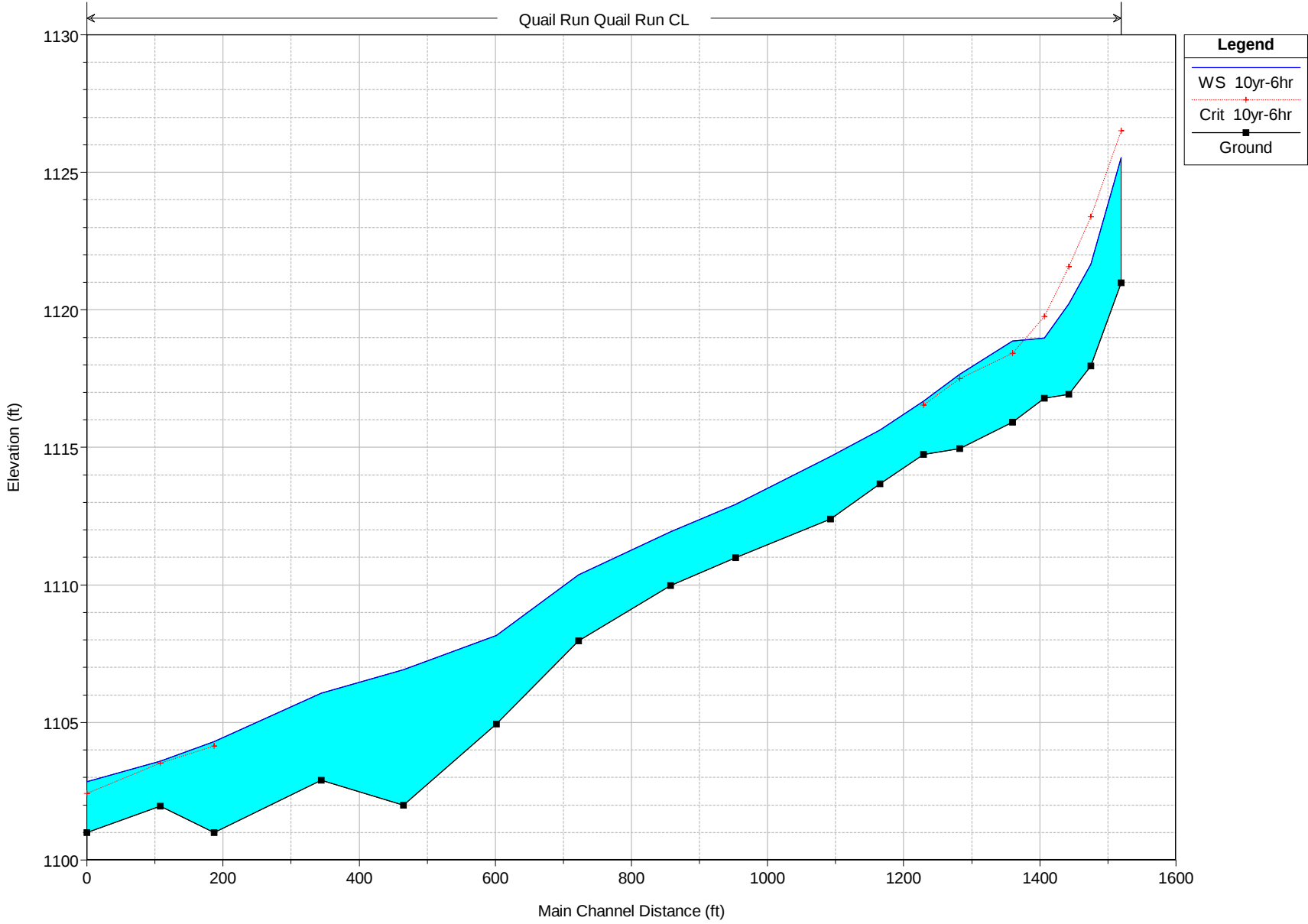
APPENDIX 2

HEC-RAS Schematic, Output Tables, Cross Sections & Profiles

HEC-RAS Model

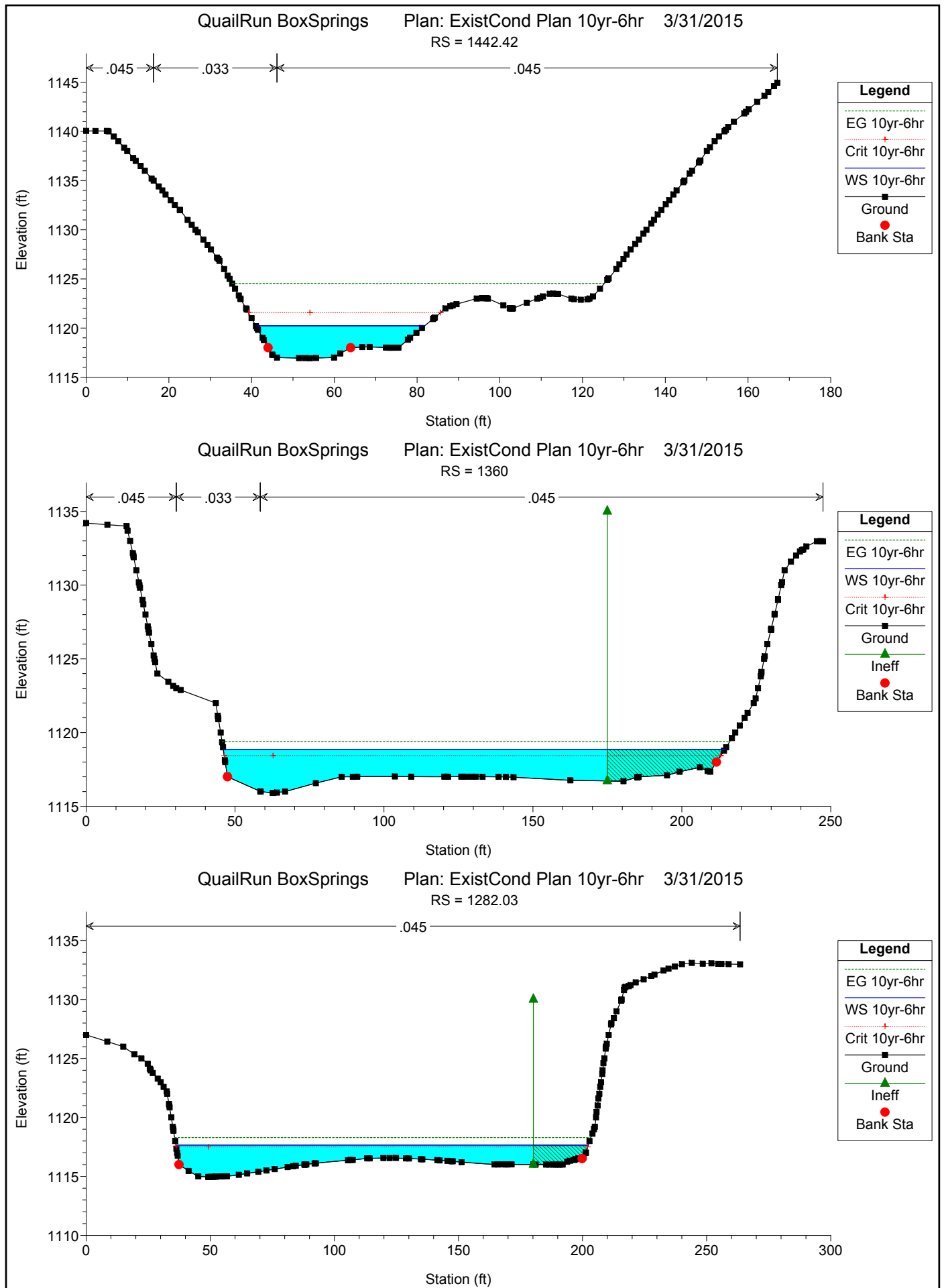


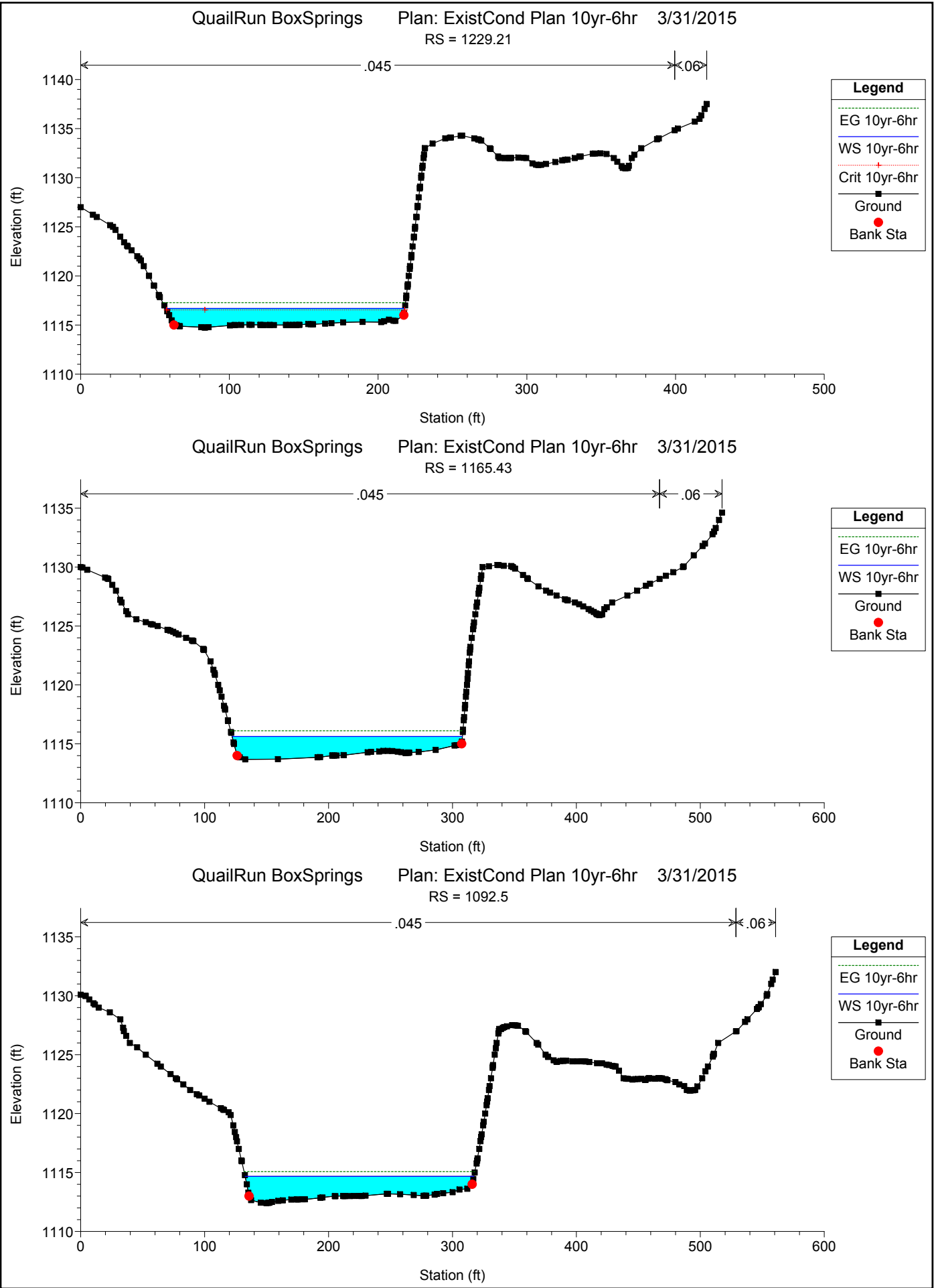
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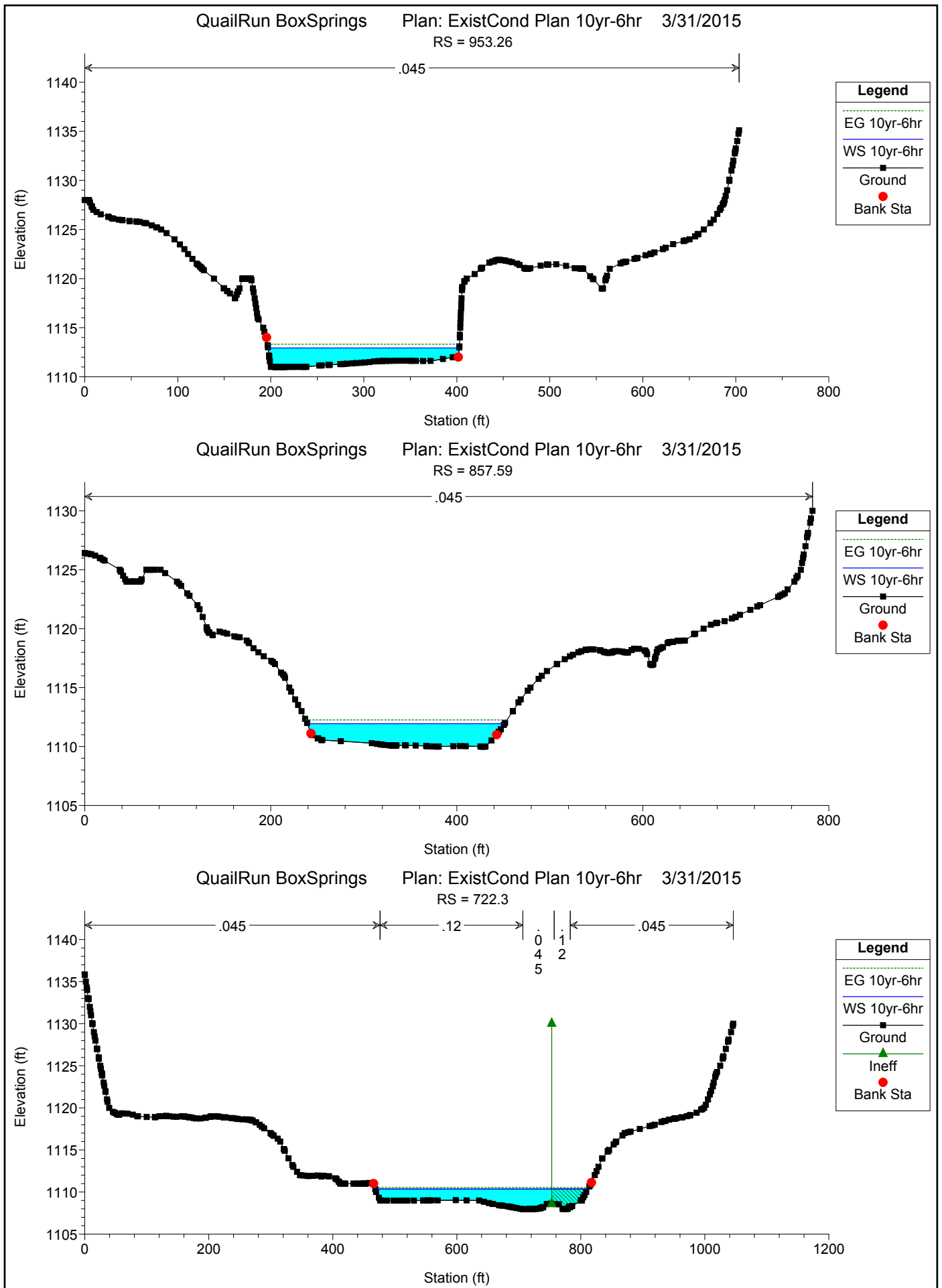


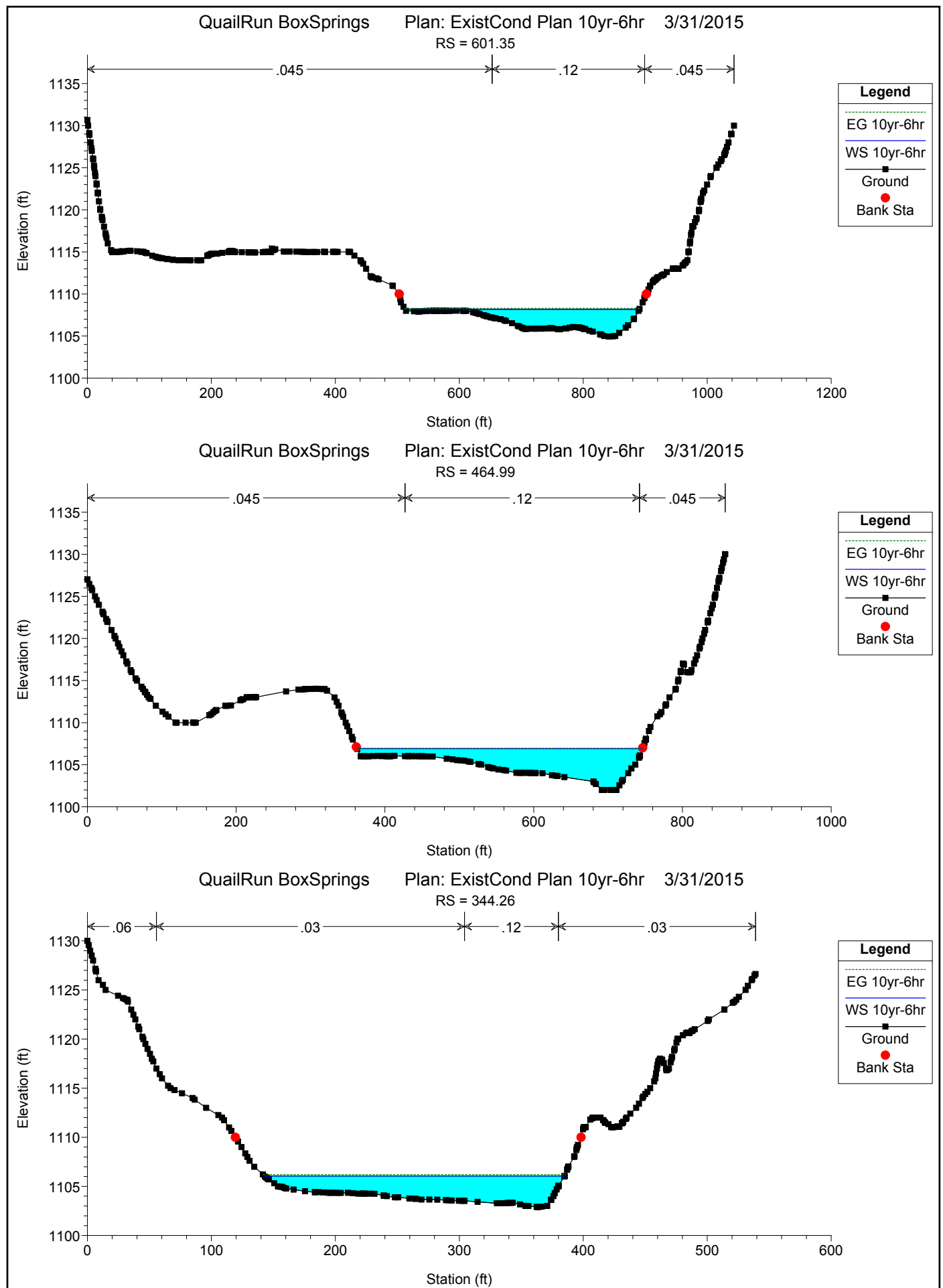
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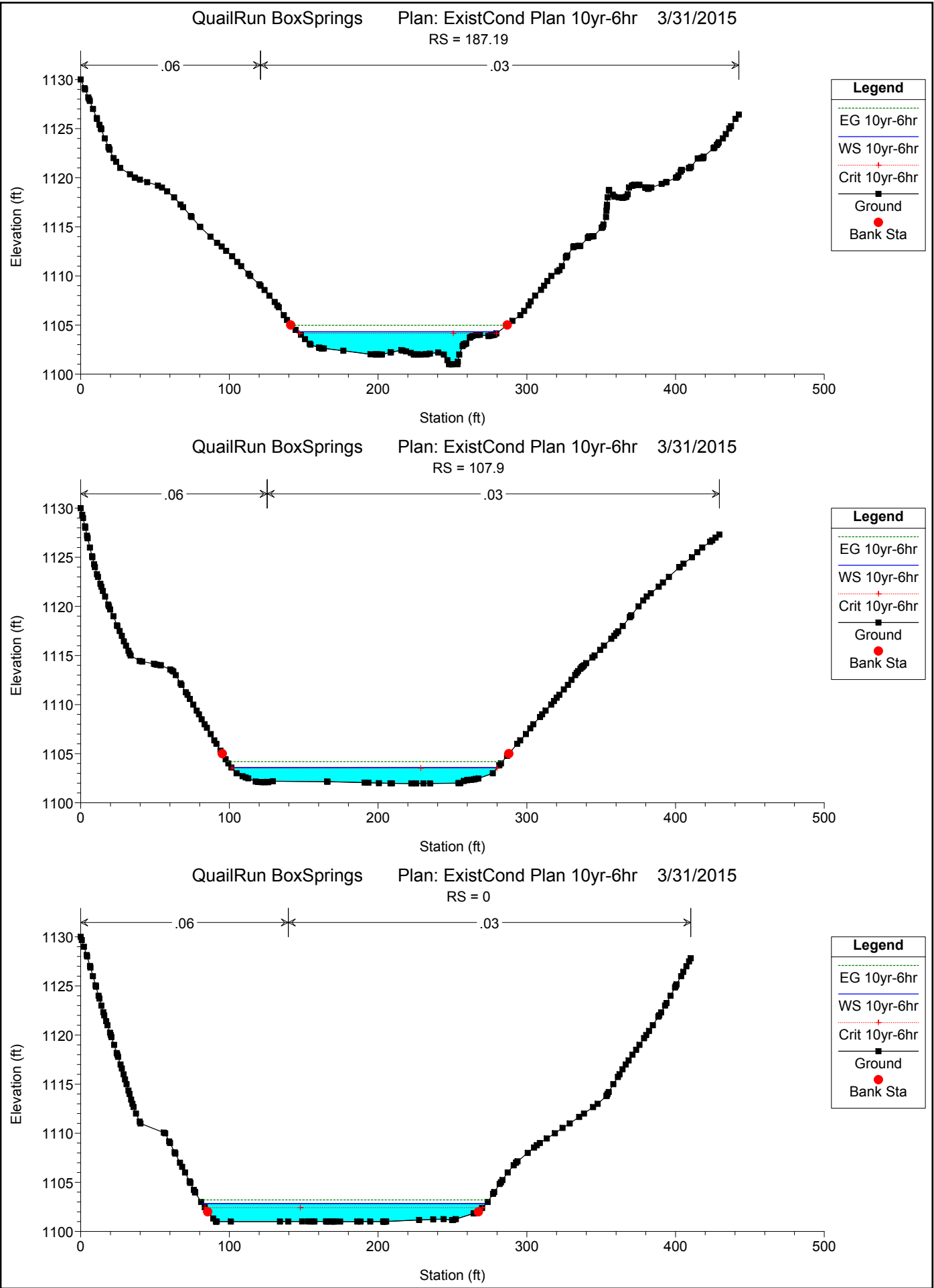
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Quail Run CL	1519.28	10yr-6hr	1553.60	1120.98	1125.54	1126.52	1128.93	0.033022	15.67	111.35	35.84	1.34
Quail Run CL	1475.16	10yr-6hr	1553.60	1117.96	1121.65	1123.39	1126.83	0.060282	18.78	88.24	30.76	1.76
Quail Run CL	1442.42	10yr-6hr	1553.60	1116.93	1120.23	1121.57	1124.53	0.063923	18.10	97.74	40.84	1.81
Quail Run CL	1406.38	10yr-6hr	1553.60	1116.79	1118.97	1119.76	1121.60	0.071063	13.06	120.16	79.85	1.84
Quail Run CL	1360	10yr-6hr	1553.60	1115.91	1118.86	1118.43	1119.39	0.010829	5.83	267.27	168.40	0.71
Quail Run CL	1282.03	10yr-6hr	1553.60	1114.96	1117.66	1117.50	1118.29	0.018631	6.41	243.13	166.16	0.87
Quail Run CL	1229.21	10yr-6hr	1553.60	1114.75	1116.69	1116.54	1117.28	0.019117	6.23	251.27	160.66	0.87
Quail Run CL	1165.43	10yr-6hr	1553.60	1113.68	1115.63		1116.12	0.016562	5.61	278.36	185.85	0.80
Quail Run CL	1092.5	10yr-6hr	1553.60	1112.40	1114.68		1115.08	0.011811	5.09	306.94	184.54	0.69
Quail Run CL	953.26	10yr-6hr	1553.60	1110.99	1112.93		1113.32	0.013392	5.02	309.62	205.59	0.72
Quail Run CL	857.59	10yr-6hr	1553.60	1109.98	1111.94		1112.25	0.009099	4.51	347.90	212.07	0.61
Quail Run CL	722.3	10yr-6hr	1553.60	1107.97	1110.36		1110.54	0.017784	3.38	460.29	343.12	0.47
Quail Run CL	601.35	10yr-6hr	1553.60	1104.94	1108.17		1108.29	0.019217	2.81	553.87	377.76	0.41
Quail Run CL	464.99	10yr-6hr	1553.60	1102.00	1106.91		1106.96	0.005718	1.78	870.99	383.57	0.21
Quail Run CL	344.26	10yr-6hr	1553.60	1102.89	1106.06		1106.20	0.006699	3.06	507.66	242.36	0.37
Quail Run CL	187.19	10yr-6hr	1553.60	1101.00	1104.31	1104.14	1104.98	0.008383	6.56	236.70	135.02	0.87
Quail Run CL	107.9	10yr-6hr	1553.60	1101.96	1103.59	1103.52	1104.20	0.011165	6.26	248.22	178.98	0.94
Quail Run CL	0	10yr-6hr	1553.60	1101.00	1102.84	1102.42	1103.21	0.006603	4.90	319.56	191.01	0.66





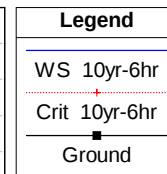
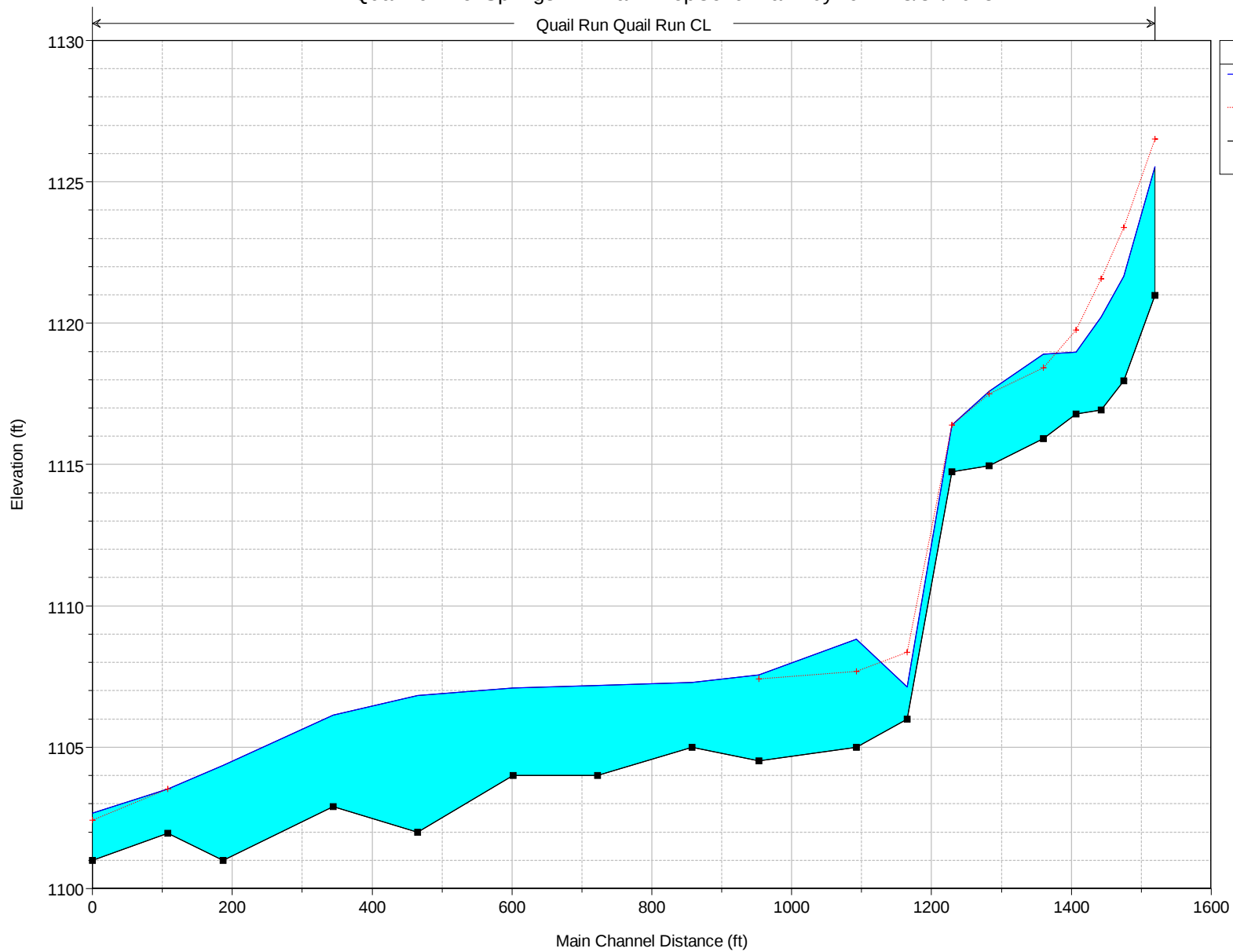






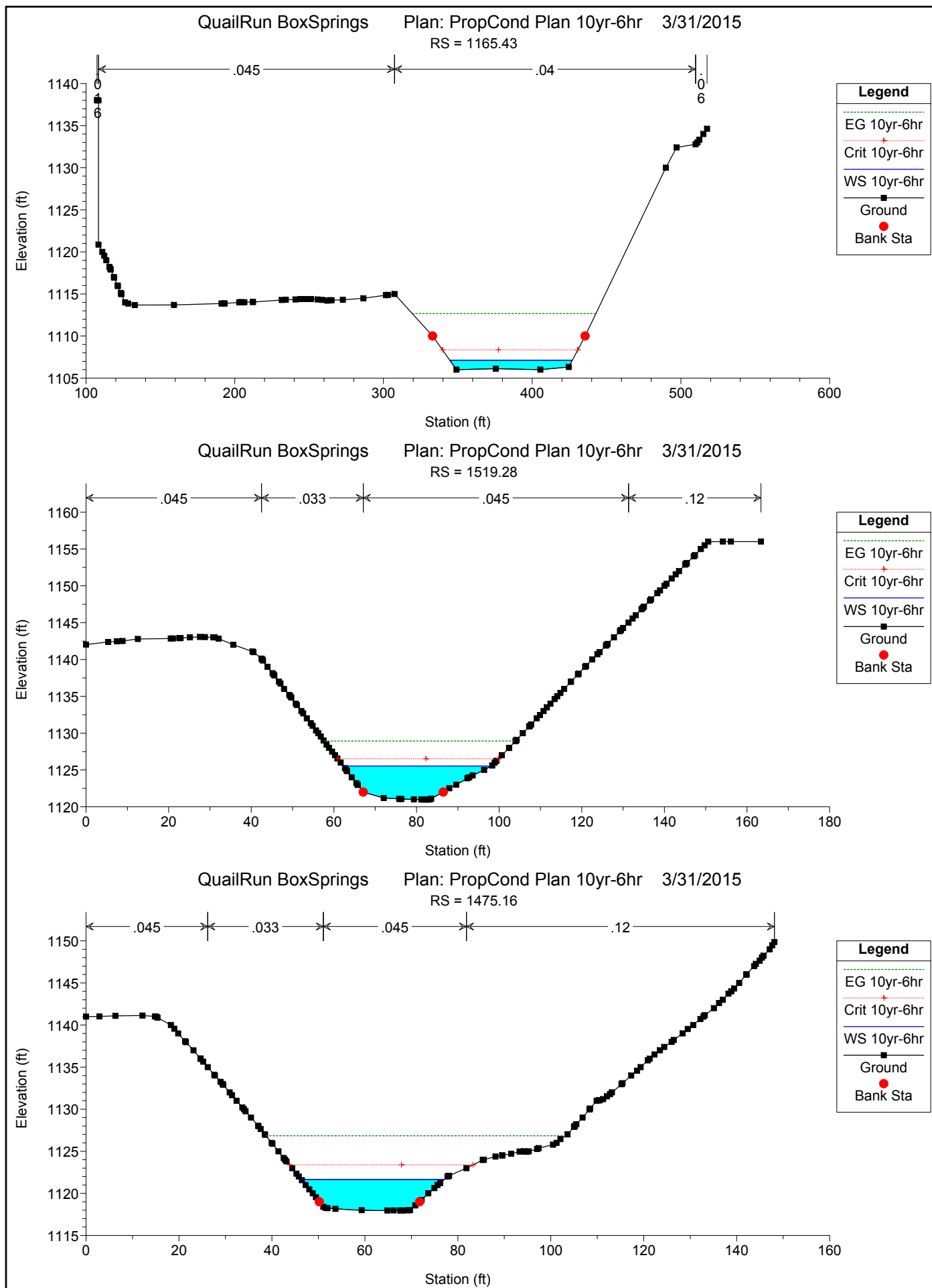
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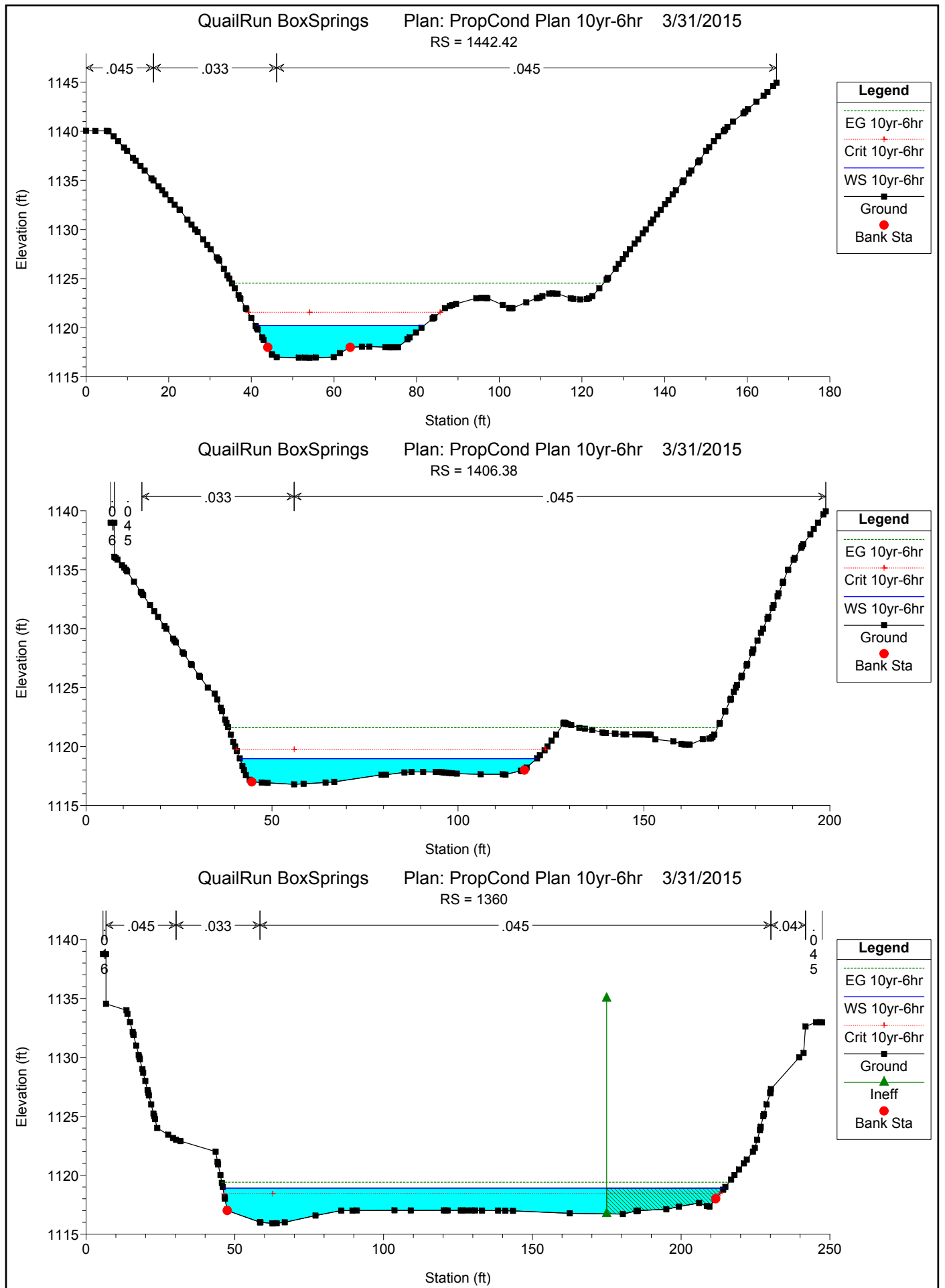
Quail Run Quail Run CL

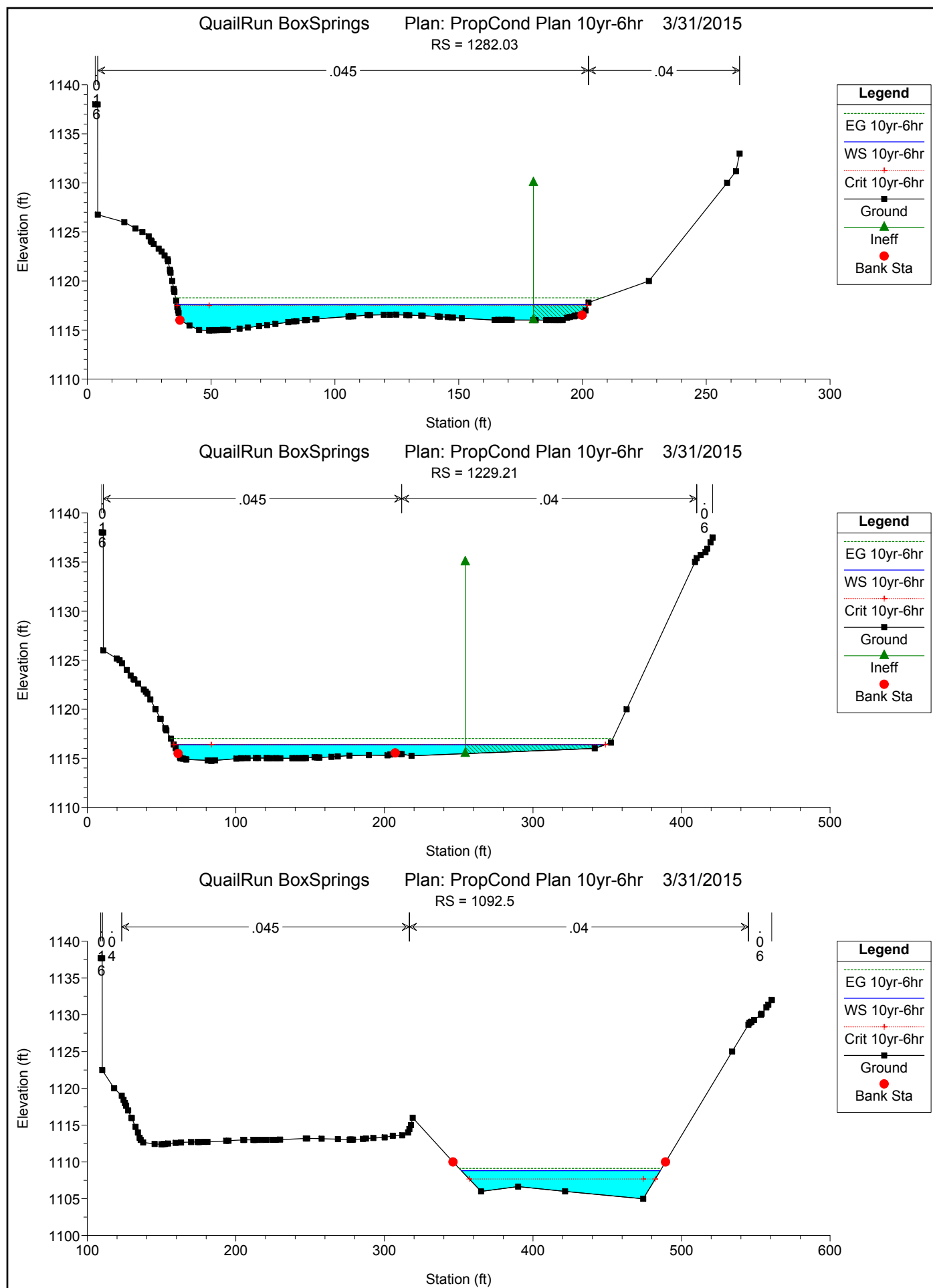


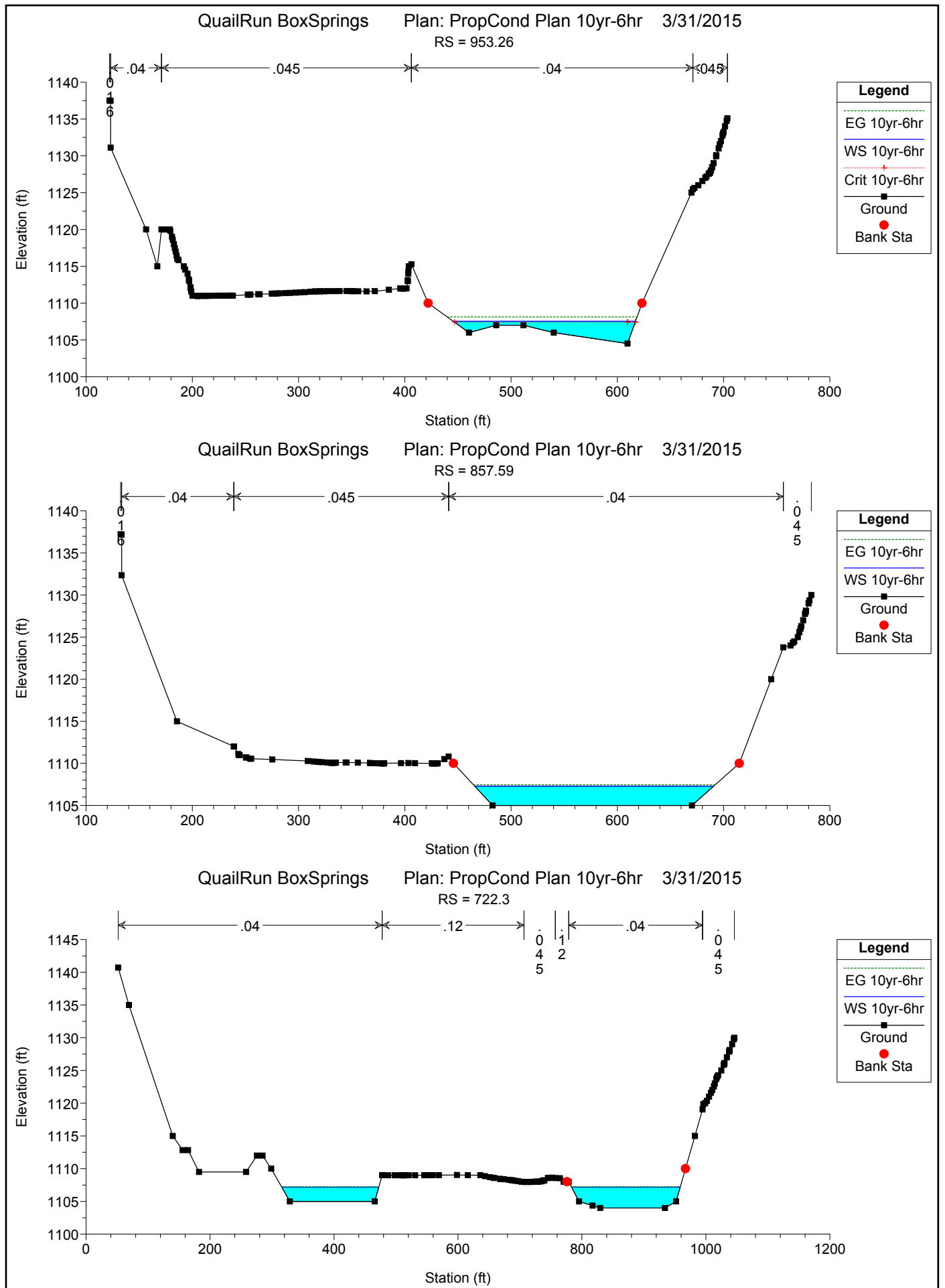
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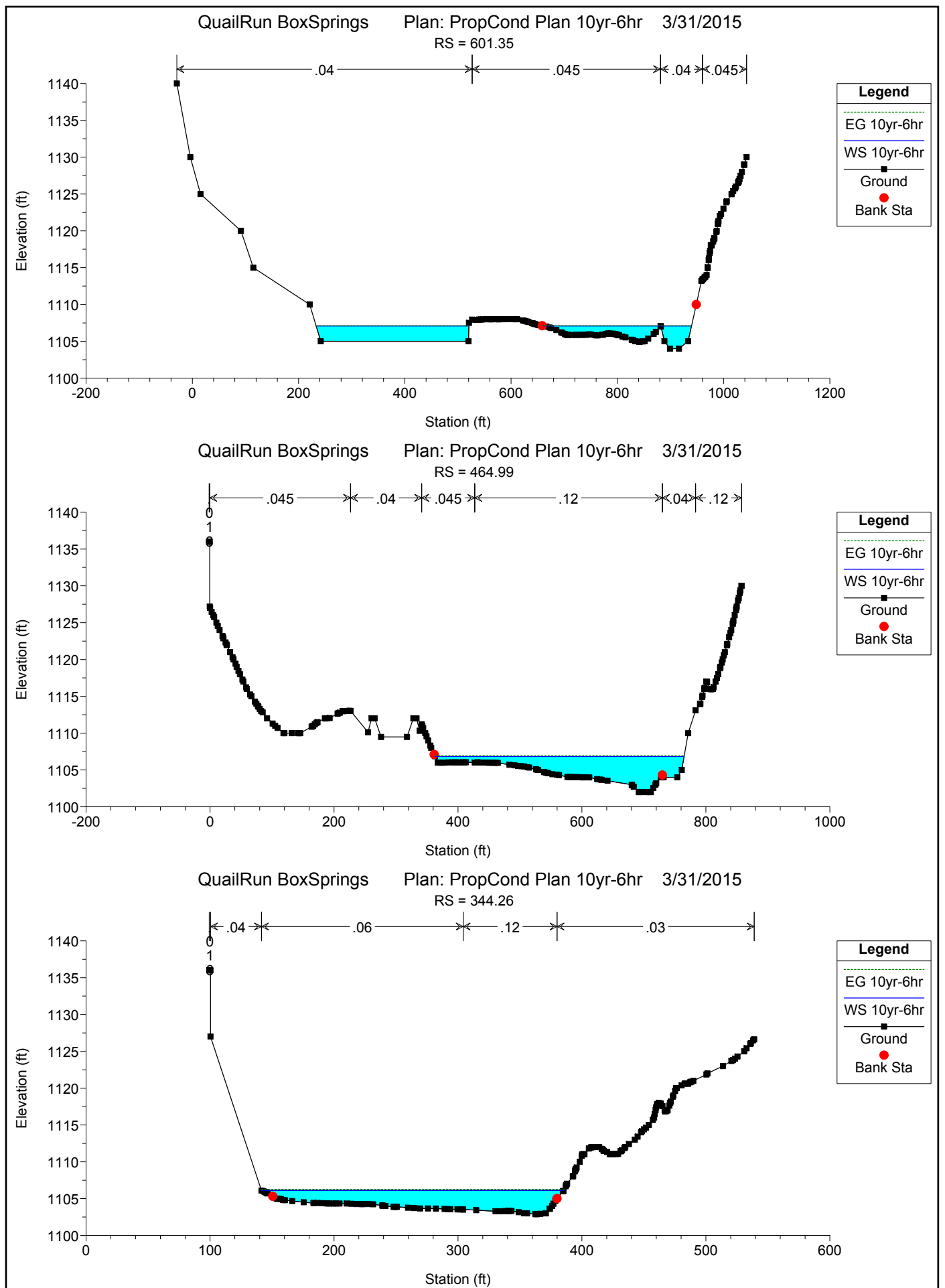
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Quail Run CL	1519.28	10yr-6hr	1553.60	1120.98	1125.54	1126.52	1128.93	0.033022	15.67	111.35	35.84	1.34
Quail Run CL	1475.16	10yr-6hr	1553.60	1117.96	1121.65	1123.39	1126.83	0.060282	18.78	88.24	30.76	1.76
Quail Run CL	1442.42	10yr-6hr	1553.60	1116.93	1120.23	1121.57	1124.53	0.063923	18.10	97.74	40.84	1.81
Quail Run CL	1406.38	10yr-6hr	1553.60	1116.79	1118.97	1119.76	1121.60	0.071063	13.06	120.16	79.85	1.84
Quail Run CL	1360	10yr-6hr	1553.60	1115.91	1118.90	1118.43	1119.41	0.010201	5.72	272.15	168.55	0.69
Quail Run CL	1282.03	10yr-6hr	1553.60	1114.96	1117.59	1117.50	1118.28	0.021172	6.66	233.95	165.98	0.92
Quail Run CL	1229.21	10yr-6hr	1553.60	1114.75	1116.39	1116.39	1117.02	0.026191	6.46	244.08	290.53	0.99
Quail Run CL	1165.43	10yr-6hr	1553.60	1106.00	1107.13	1108.36	1112.68	0.260990	18.90	82.18	82.39	3.34
Quail Run CL	1092.5	10yr-6hr	1553.60	1104.99	1108.83	1107.68	1109.12	0.003737	4.35	357.38	134.00	0.47
Quail Run CL	953.26	10yr-6hr	1553.60	1104.51	1107.55	1107.42	1108.13	0.016325	6.13	253.27	171.67	0.89
Quail Run CL	857.59	10yr-6hr	1553.60	1105.00	1107.29		1107.45	0.002939	3.30	471.38	224.85	0.40
Quail Run CL	722.3	10yr-6hr	1553.60	1104.00	1107.18		1107.24	0.000789	2.07	811.18	331.87	0.22
Quail Run CL	601.35	10yr-6hr	1553.60	1104.00	1107.09		1107.13	0.000819	1.34	993.64	566.75	0.20
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Quail Run CL	344.26	10yr-6hr	1553.60	1102.89	1106.13		1106.27	0.007934	2.96	526.45	243.91	0.35
Quail Run CL	187.19	10yr-6hr	1553.60	1101.00	1104.35		1104.99	0.007744	6.40	242.92	135.74	0.84
Quail Run CL	107.9	10yr-6hr	1553.60	1101.96	1103.52	1103.52	1104.20	0.013022	6.63	234.84	174.54	1.00
Quail Run CL	0	10yr-6hr	1553.60	1101.00	1102.68	1102.42	1103.13	0.006606	5.43	287.72	189.23	0.76

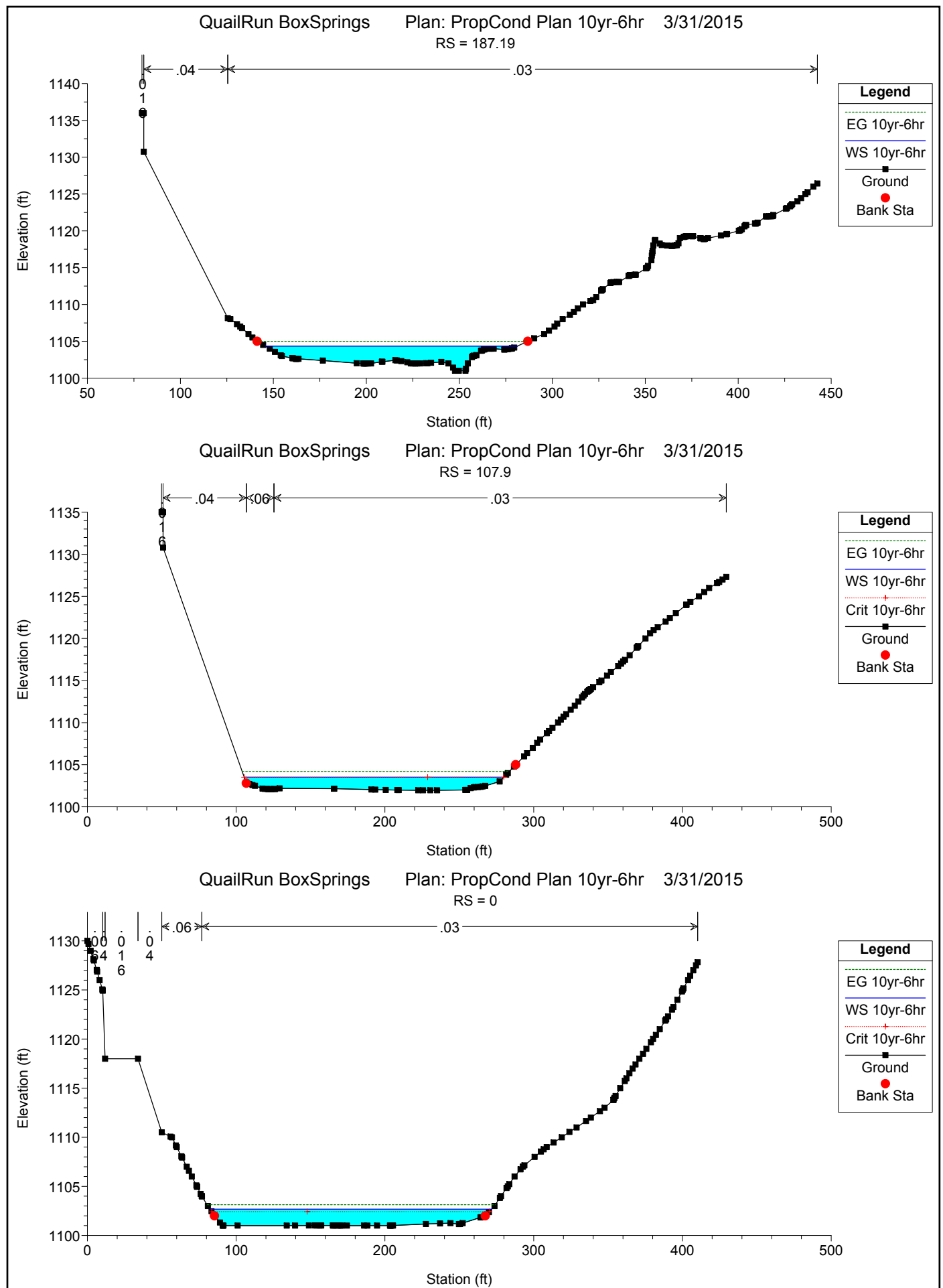




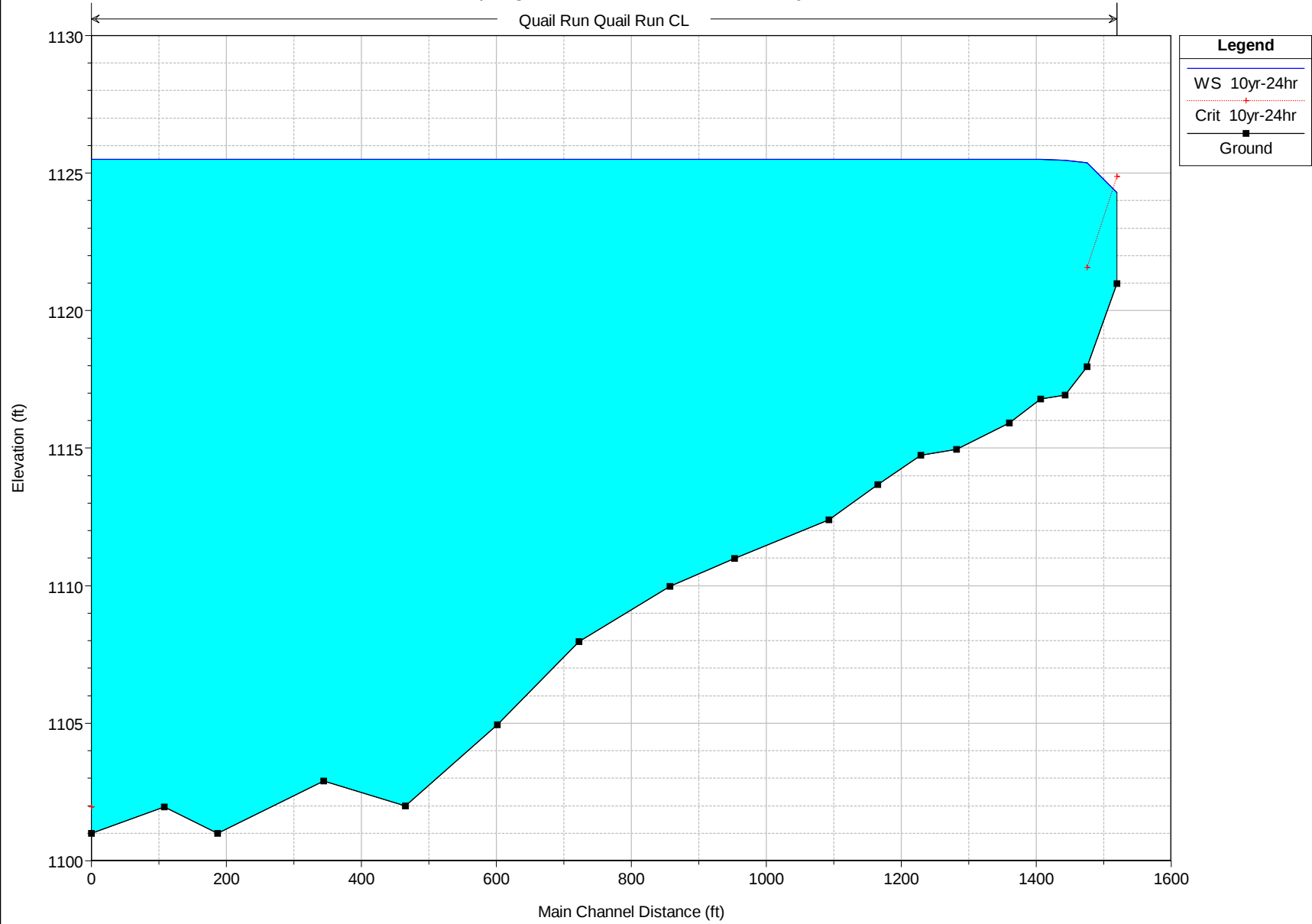








QuailRun BoxSprings Plan: ExistCond Plan 10yr-24hr 3/31/2015

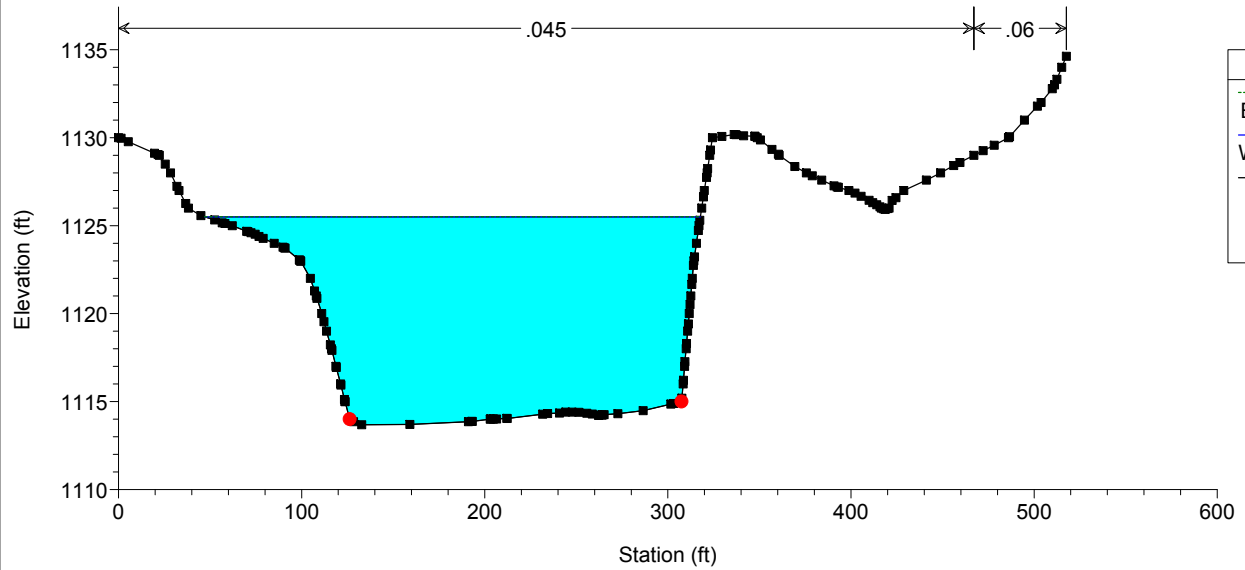


HEC-RAS Plan: ExCond 1024 River: Quail Run Reach: Quail Run CL Profile: 10yr-24hr

Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Quail Run CL	1519.28	10yr-24hr	805.90	1120.98	1124.29	1124.88	1126.52	0.033009	12.43	70.20	29.79	1.26
Quail Run CL	1475.16	10yr-24hr	805.90	1117.96	1125.38	1121.58	1125.59	0.001007	3.93	242.61	56.57	0.26
Quail Run CL	1442.42	10yr-24hr	805.90	1116.93	1125.47		1125.53	0.000298	2.39	452.62	93.06	0.15
Quail Run CL	1406.38	10yr-24hr	805.90	1116.79	1125.49		1125.51	0.000052	1.02	903.67	143.94	0.06
Quail Run CL	1360	10yr-24hr	805.90	1115.91	1125.50		1125.50	0.000014	0.53	1572.67	205.80	0.03
Quail Run CL	1282.03	10yr-24hr	805.90	1114.96	1125.50		1125.50	0.000012	0.51	1649.14	190.67	0.03
Quail Run CL	1229.21	10yr-24hr	805.90	1114.75	1125.50		1125.50	0.000009	0.46	1851.03	209.10	0.03
Quail Run CL	1165.43	10yr-24hr	805.90	1113.68	1125.50		1125.50	0.000005	0.37	2338.86	270.60	0.02
Quail Run CL	1092.5	10yr-24hr	805.90	1112.40	1125.50		1125.50	0.000003	0.32	2987.27	428.90	0.02
Quail Run CL	953.26	10yr-24hr	805.90	1110.99	1125.50		1125.50	0.000001	0.22	4500.21	601.07	0.01
Quail Run CL	857.59	10yr-24hr	805.90	1109.98	1125.50		1125.50	0.000001	0.17	6376.36	743.82	0.01
Quail Run CL	722.3	10yr-24hr	805.90	1107.97	1125.50		1125.50	0.000000	0.07	11418.85	1003.47	0.00
Quail Run CL	601.35	10yr-24hr	805.90	1104.94	1125.50		1125.50	0.000000	0.05	13880.95	1008.92	0.00
Quail Run CL	464.99	10yr-24hr	805.90	1102.00	1125.50		1125.50	0.000000	0.05	13283.67	837.39	0.00
Quail Run CL	344.26	10yr-24hr	805.90	1102.89	1125.50		1125.50	0.000000	0.10	7866.16	520.61	0.00
Quail Run CL	187.19	10yr-24hr	805.90	1101.00	1125.50		1125.50	0.000000	0.17	6078.02	426.07	0.01
Quail Run CL	107.9	10yr-24hr	805.90	1101.96	1125.50		1125.50	0.000000	0.15	6707.06	407.46	0.01
Quail Run CL	0	10yr-24hr	805.90	1101.00	1125.50	1101.95	1125.50	0.000000	0.13	7225.91	392.75	0.00

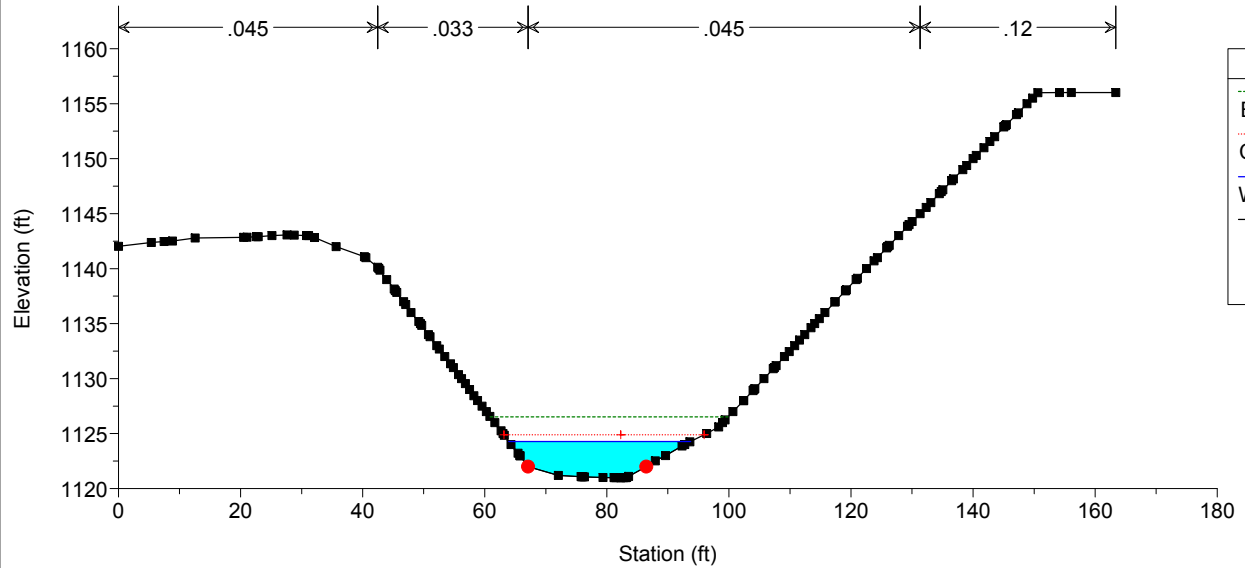
QuailRun BoxSprings Plan: ExistCond Plan 10yr-24hr 3/31/2015

RS = 1165.43



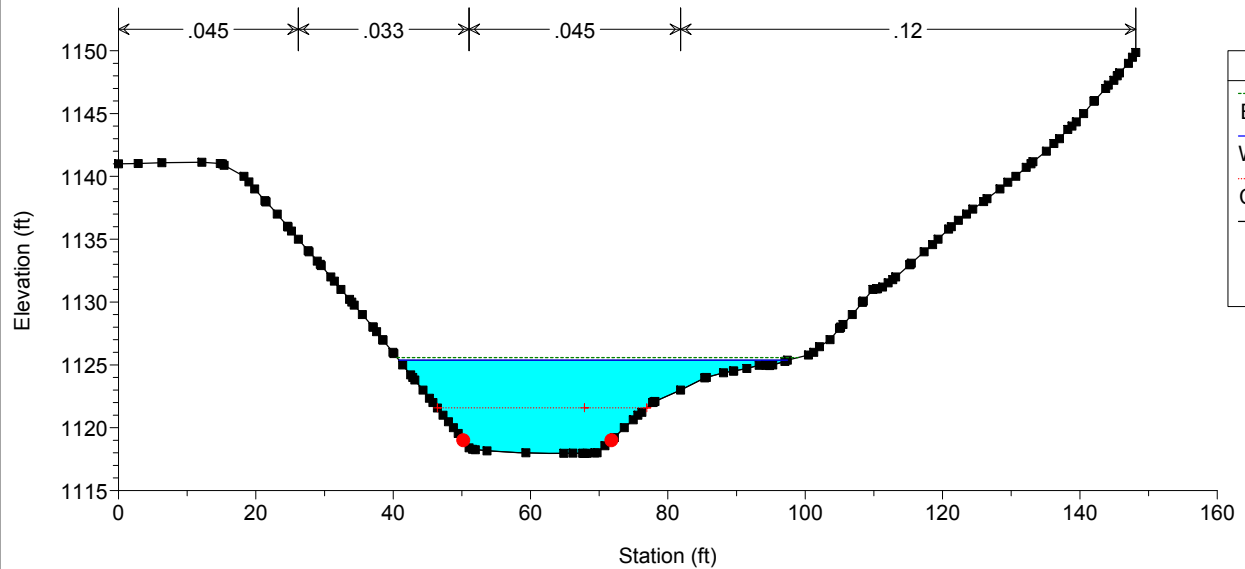
QuailRun BoxSprings Plan: ExistCond Plan 10yr-24hr 3/31/2015

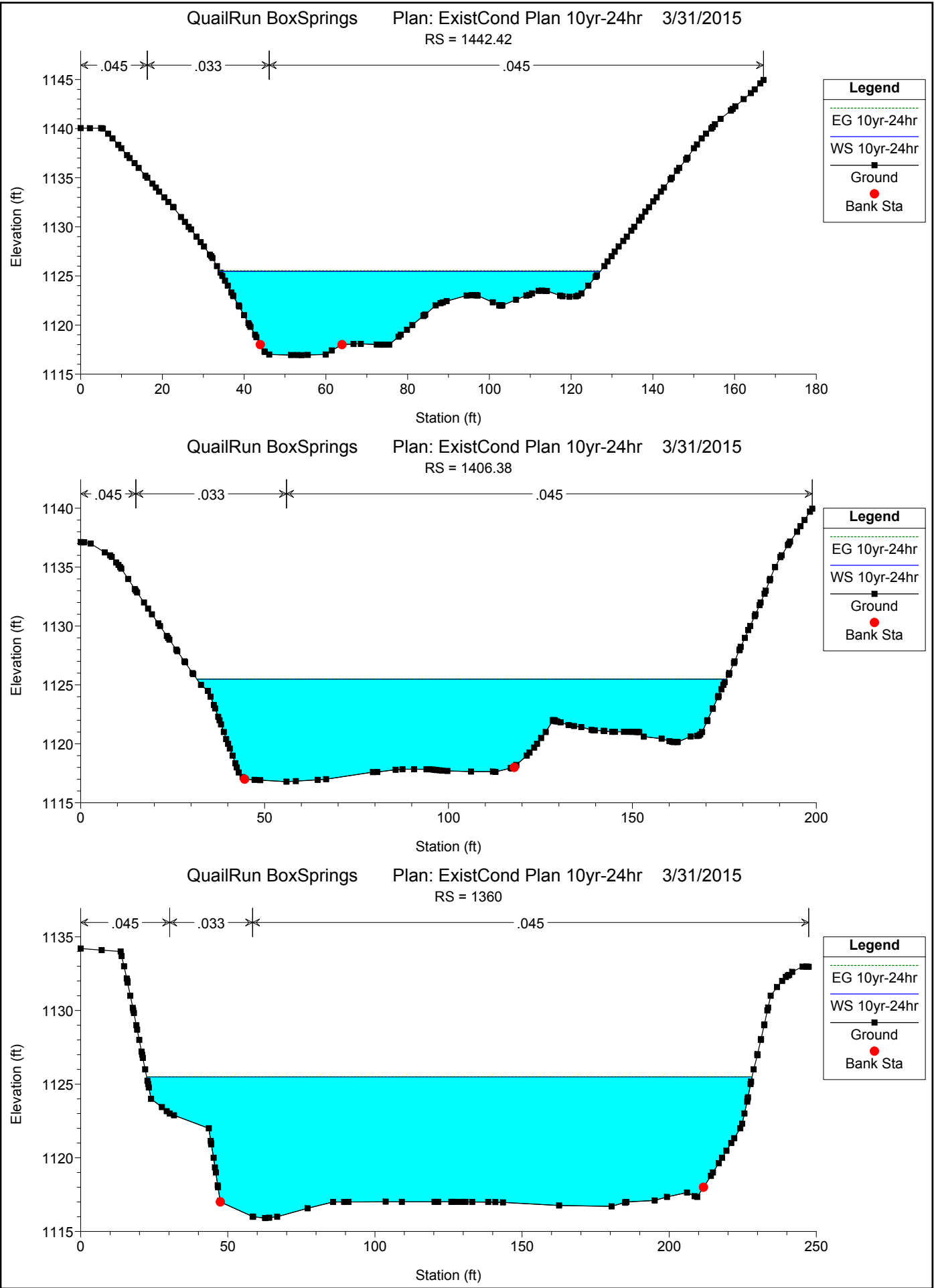
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QuailRun BoxSprings Plan: ExistCond Plan 10yr-24hr 3/31/2015

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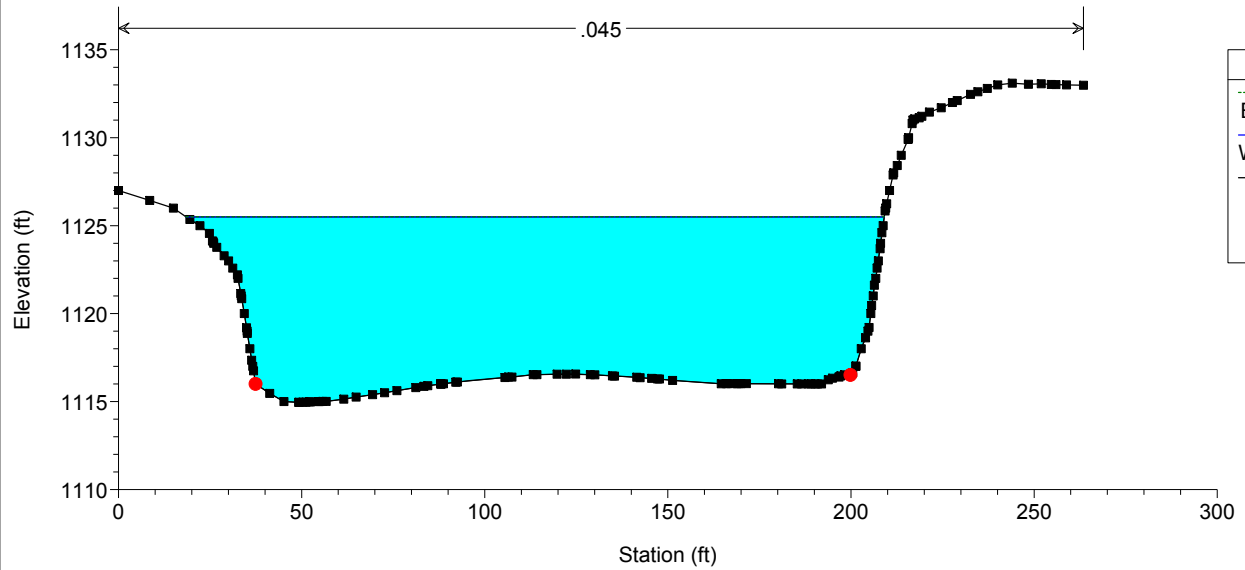




QuailRun BoxSprings

Plan: ExistCond Plan 10yr-24hr 3/31/2015

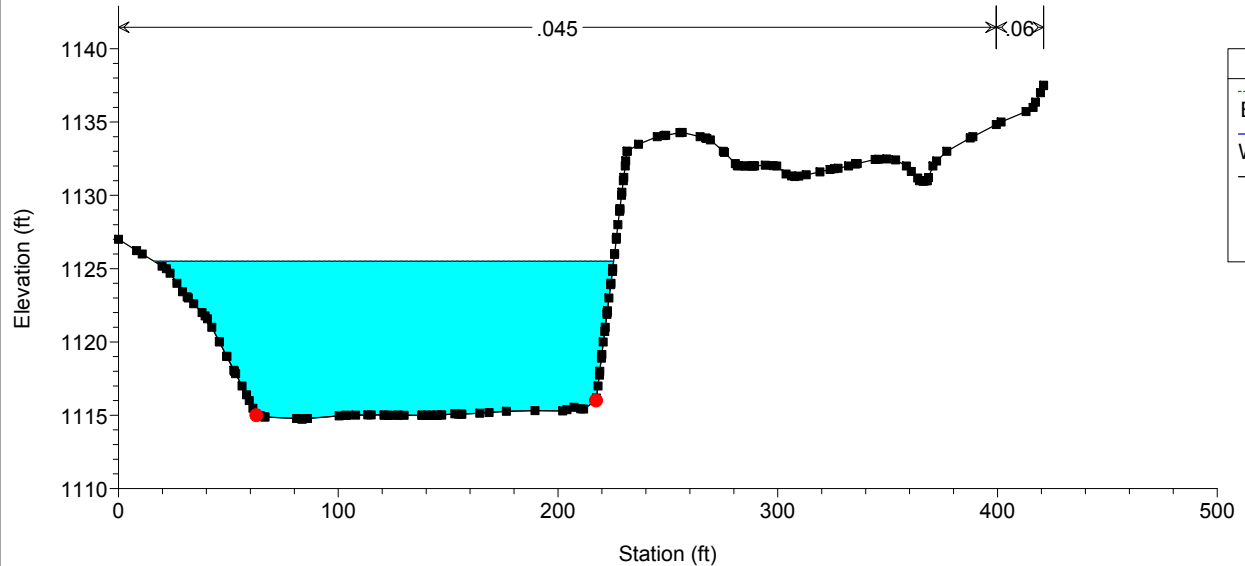
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QuailRun BoxSprings

Plan: ExistCond Plan 10yr-24hr 3/31/2015

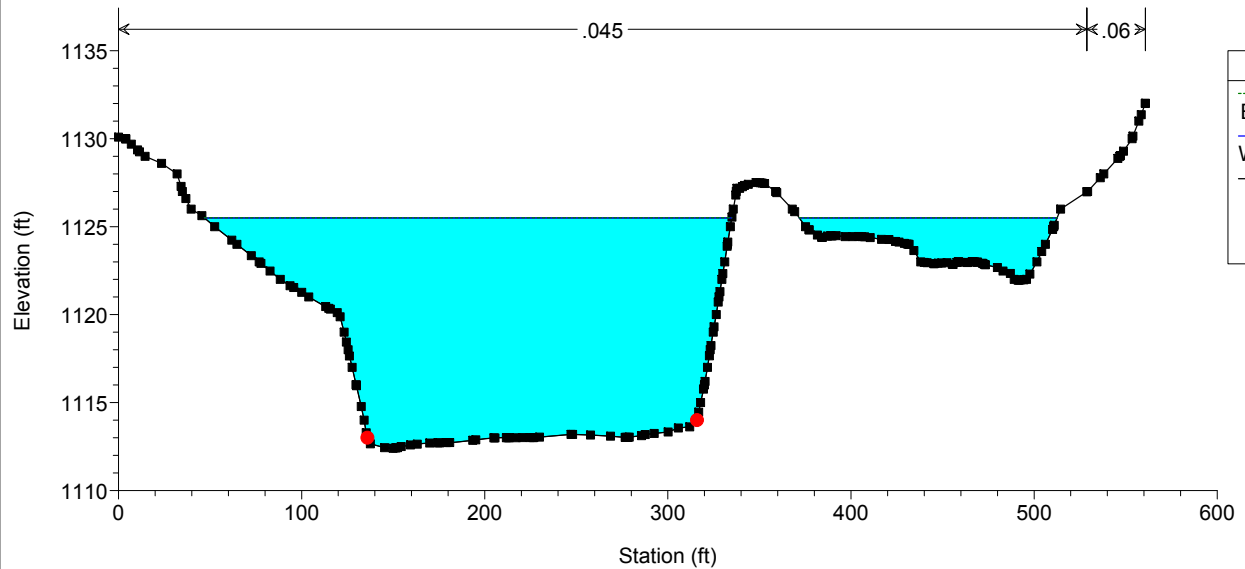
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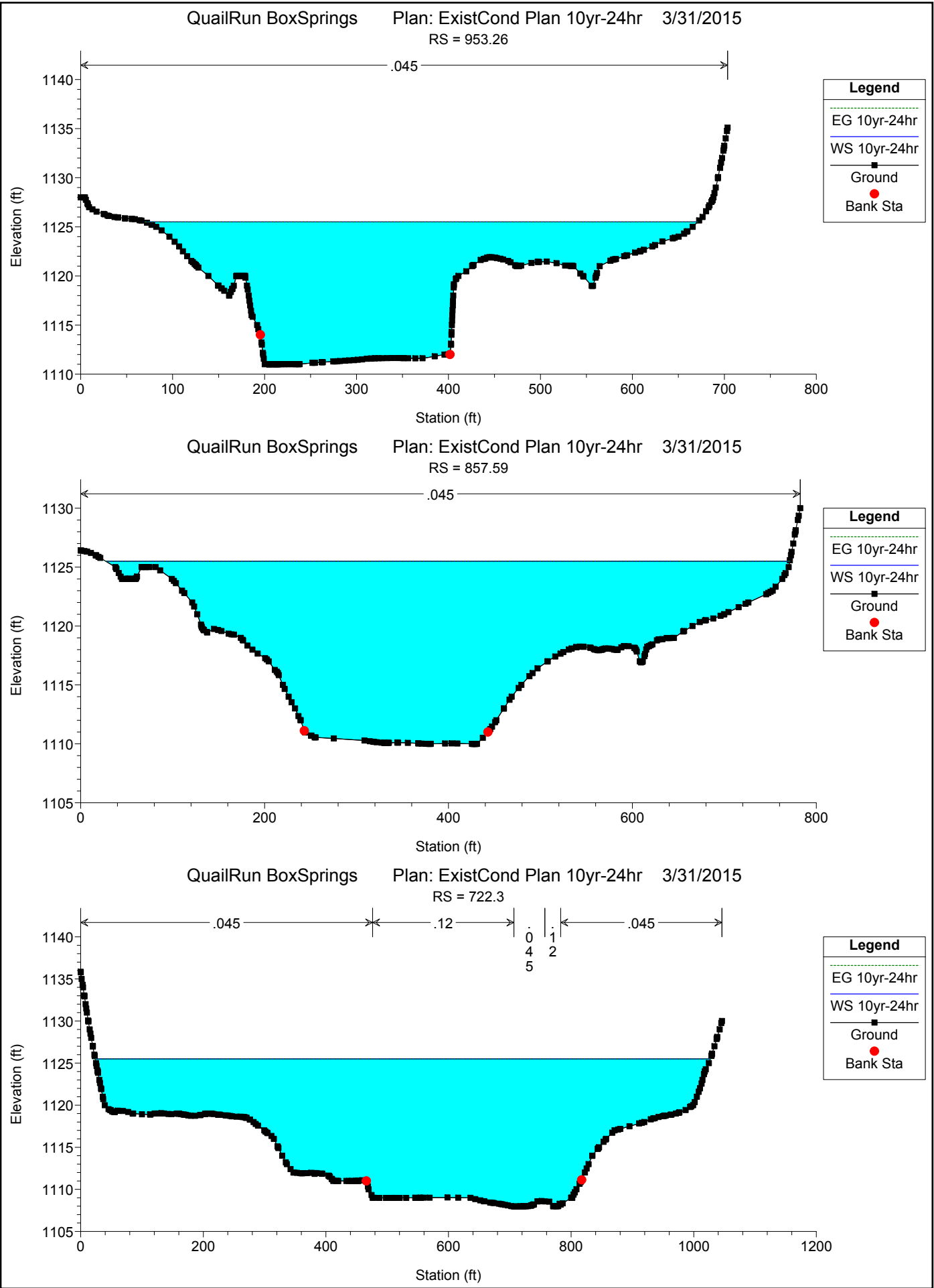


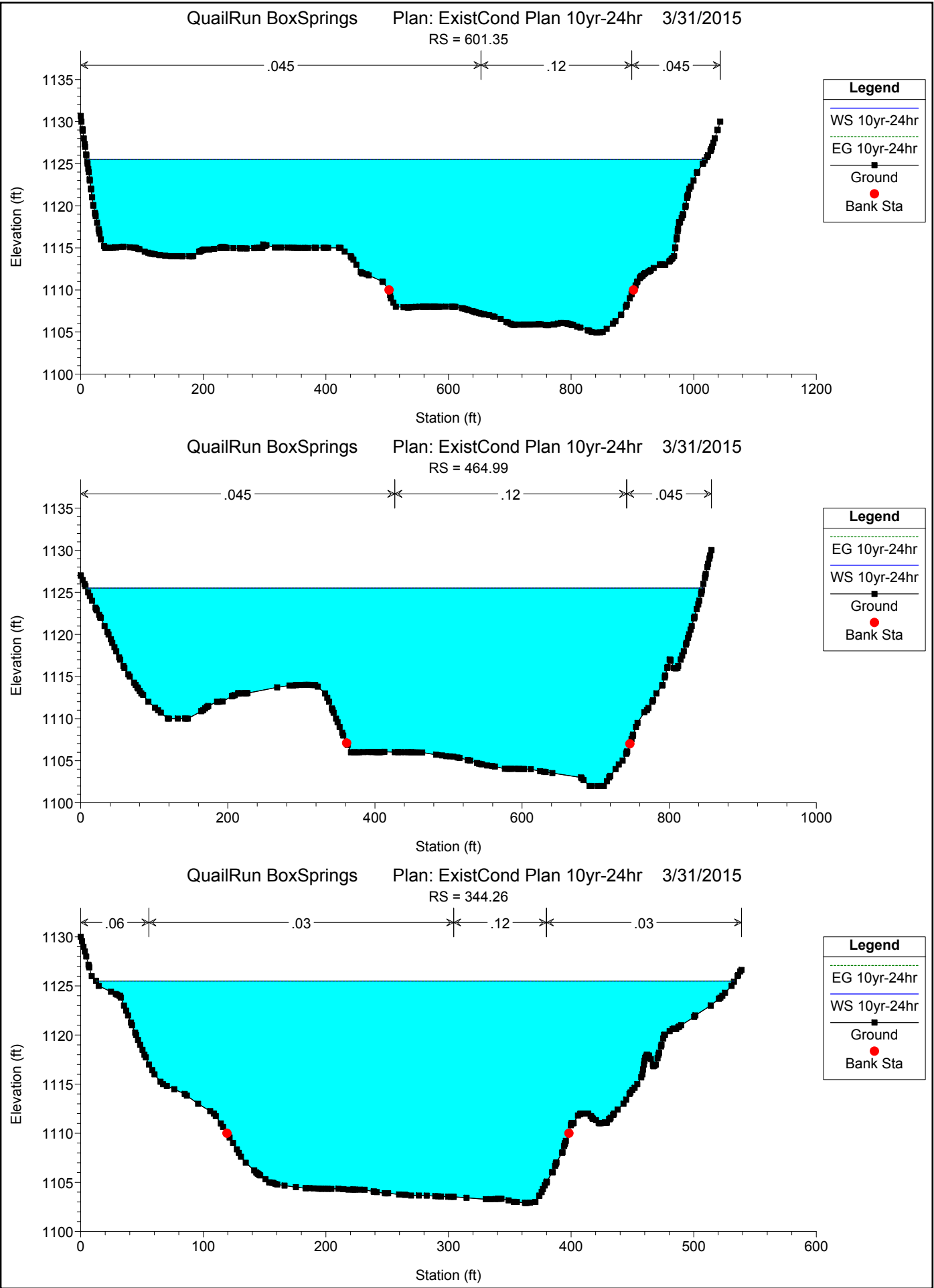
QuailRun BoxSprings

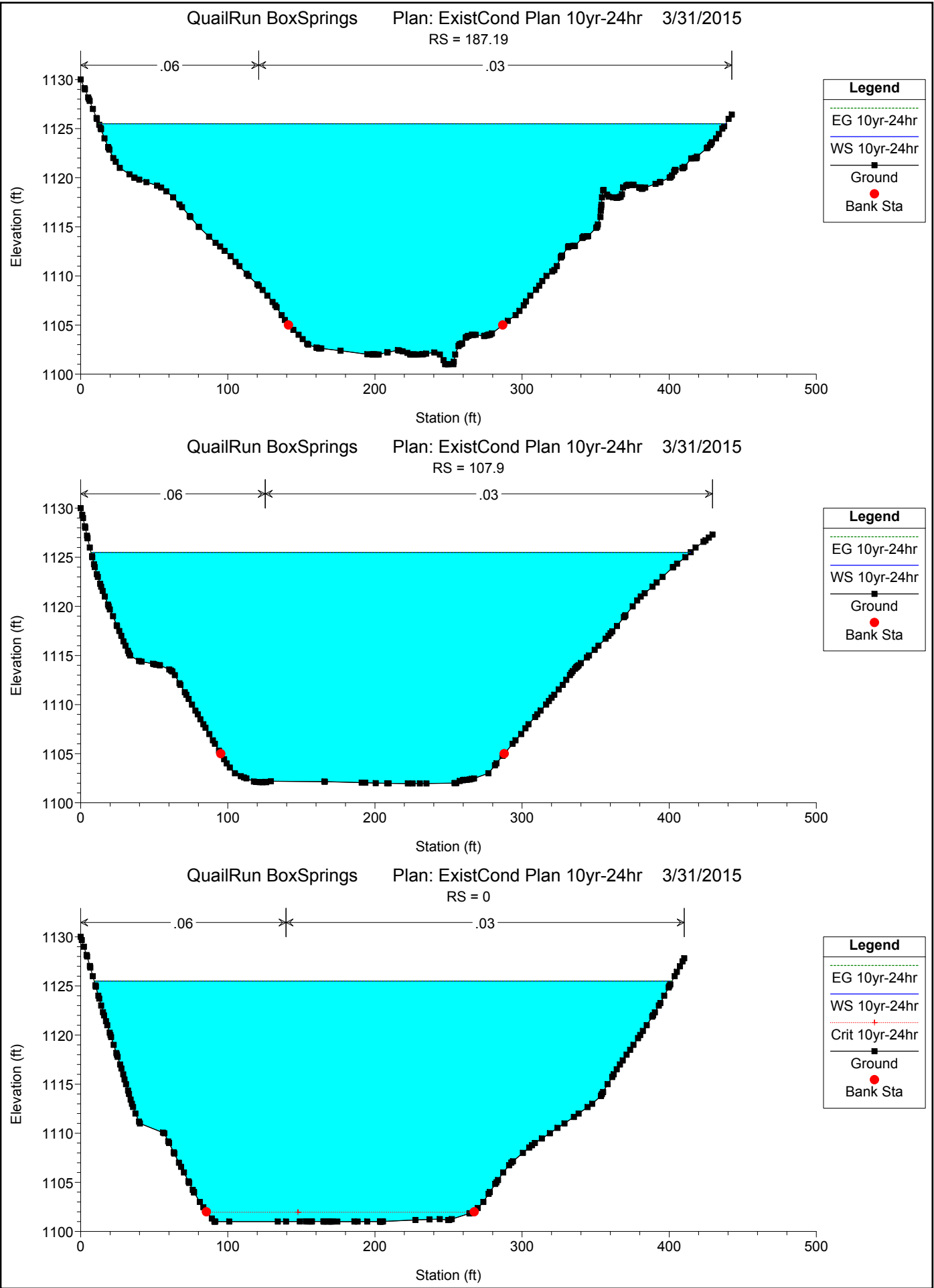
Plan: ExistCond Plan 10yr-24hr 3/31/2015

RS = 1092.5

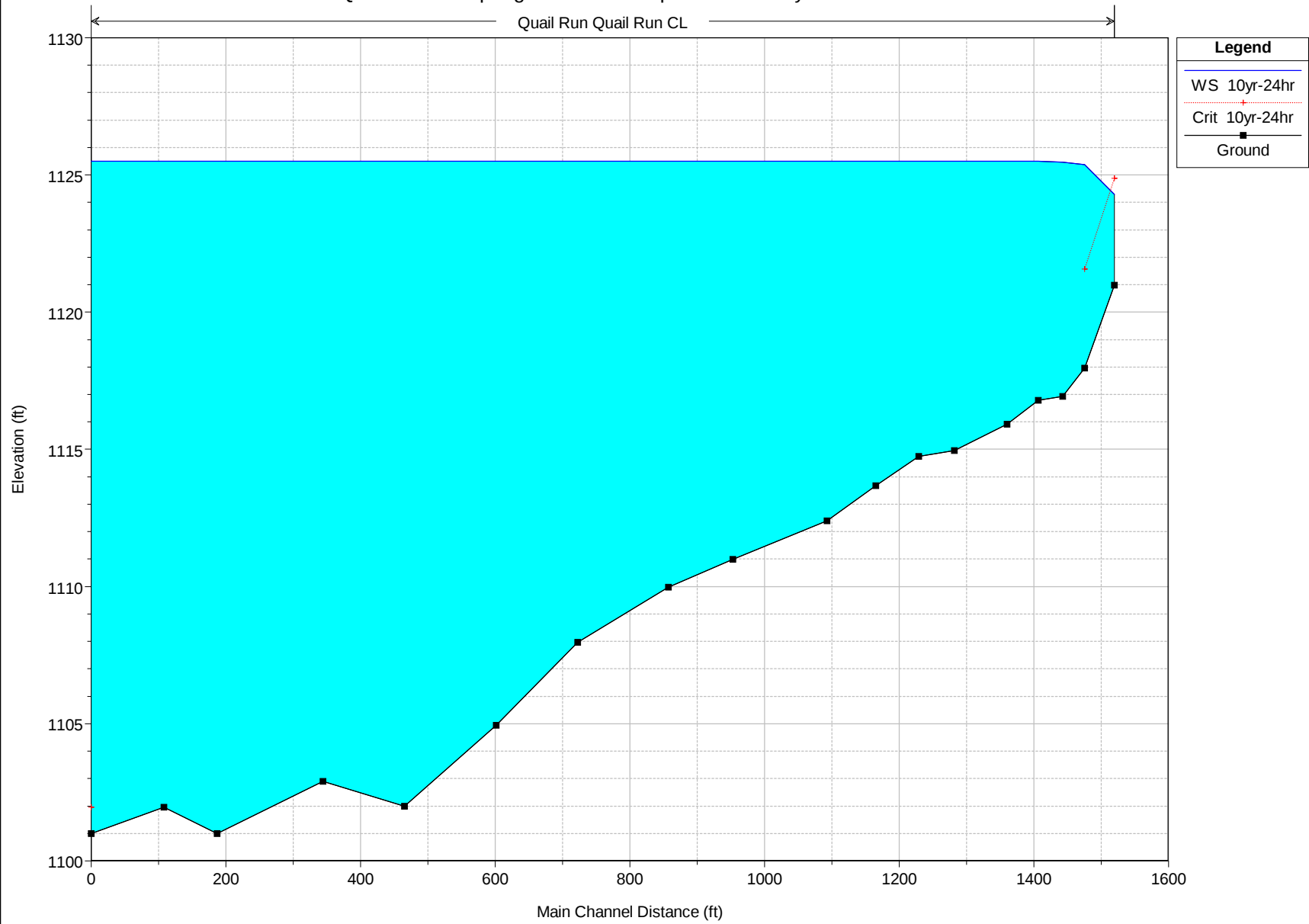






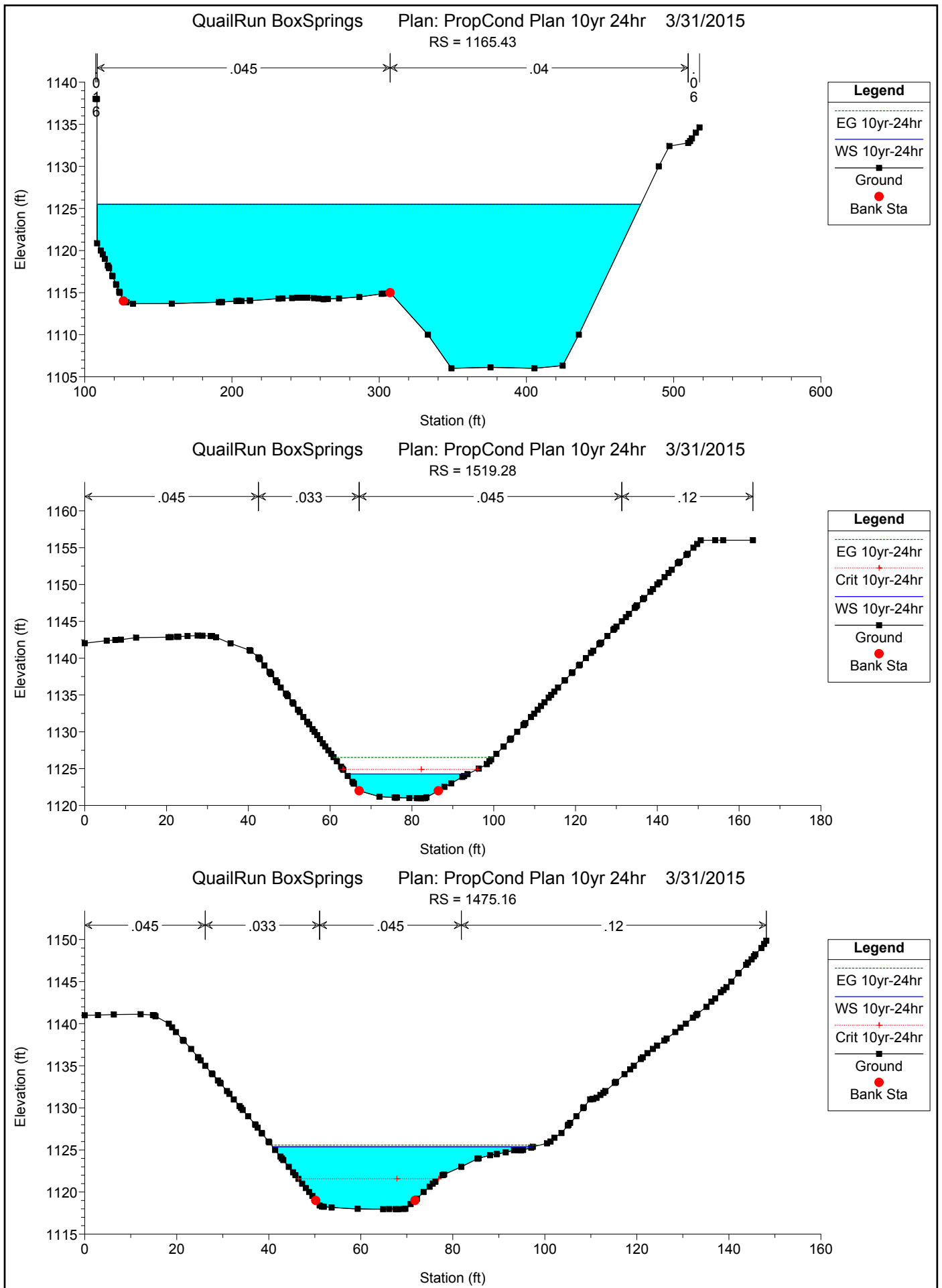


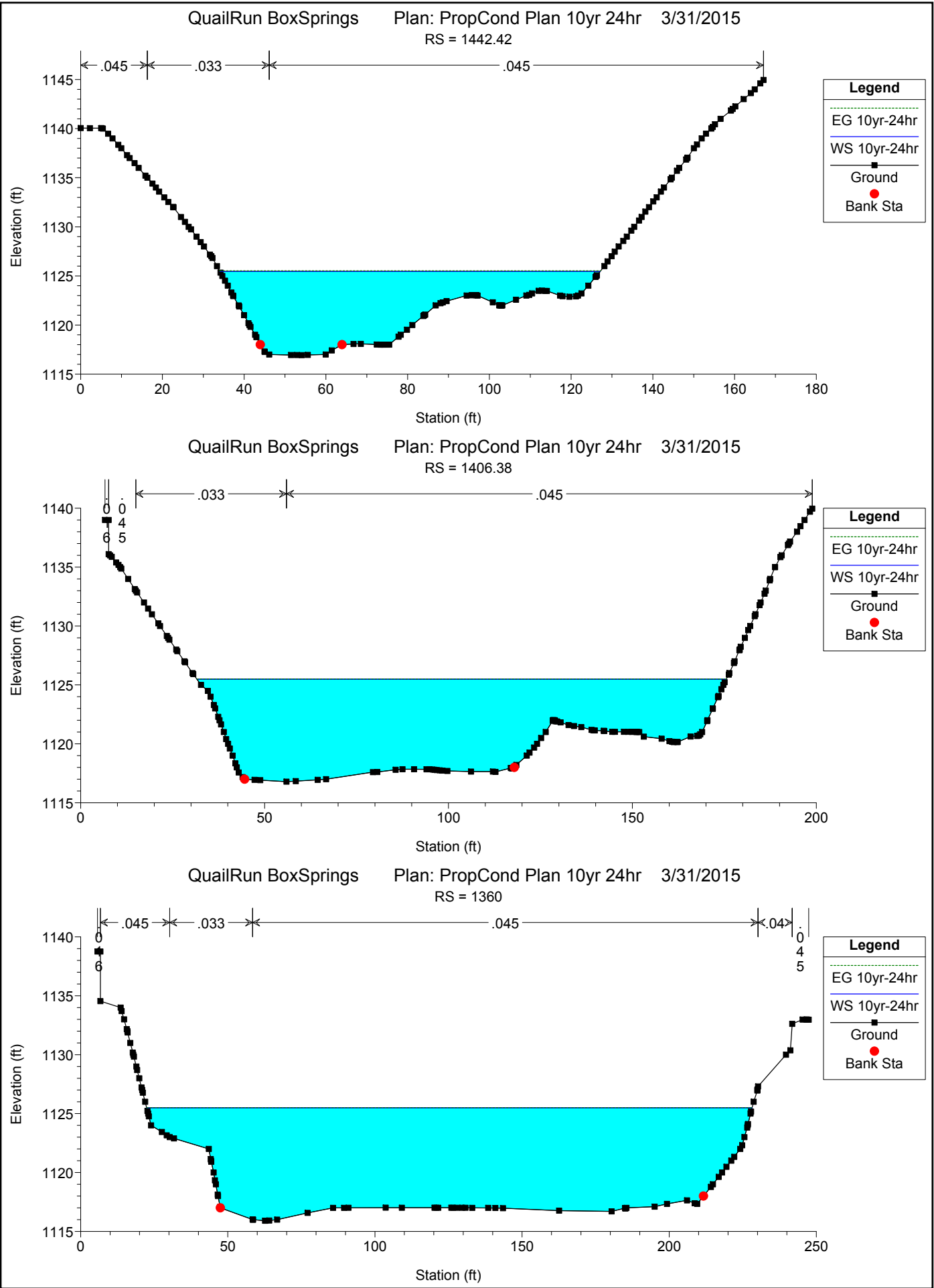
QuailRun BoxSprings Plan: PropCond Plan 10yr 24hr 3/31/2015



HEC-RAS Plan: PropCond 1024 River: Quail Run Reach: Quail Run CL Profile: 10yr-24hr

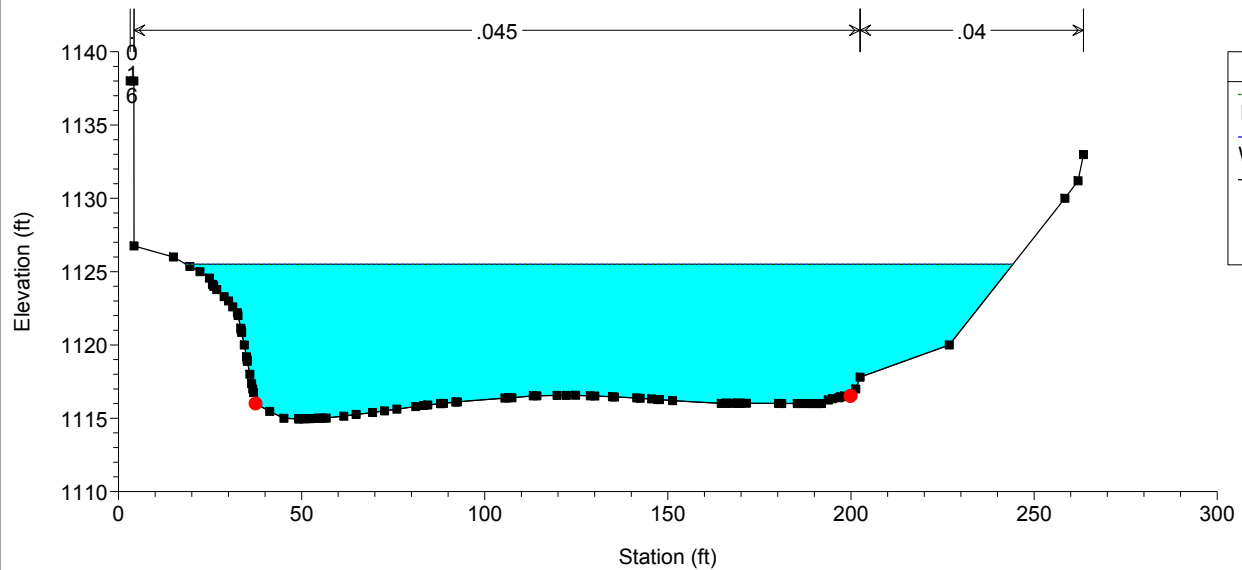
Reach	River Sta	Profile	Q Total	Min Ch El	W.S. Elev	Crit W.S.	E.G. Elev	E.G. Slope	Vel Chnl	Flow Area	Top Width	Froude # Chl
			(cfs)	(ft)	(ft)	(ft)	(ft)	(ft/ft)	(ft/s)	(sq ft)	(ft)	
Quail Run CL	1519.28	10yr-24hr	805.90	1120.98	1124.29	1124.88	1126.52	0.033009	12.43	70.20	29.79	1.26
Quail Run CL	1475.16	10yr-24hr	805.90	1117.96	1125.38	1121.58	1125.58	0.001008	3.93	242.54	56.56	0.26
Quail Run CL	1442.42	10yr-24hr	805.90	1116.93	1125.47		1125.53	0.000298	2.39	452.50	93.06	0.15
Quail Run CL	1406.38	10yr-24hr	805.90	1116.79	1125.49		1125.51	0.000052	1.02	903.49	143.94	0.06
Quail Run CL	1360	10yr-24hr	805.90	1115.91	1125.50		1125.50	0.000014	0.53	1572.42	205.79	0.03
Quail Run CL	1282.03	10yr-24hr	805.90	1114.96	1125.50		1125.50	0.000010	0.46	1827.33	225.76	0.03
Quail Run CL	1229.21	10yr-24hr	805.90	1114.75	1125.50		1125.50	0.000003	0.25	3265.78	363.73	0.01
Quail Run CL	1165.43	10yr-24hr	805.90	1113.68	1125.50		1125.50	0.000001	0.14	4802.04	369.34	0.01
Quail Run CL	1092.5	10yr-24hr	805.90	1112.40	1125.50		1125.50	0.000000	0.12	5906.20	425.49	0.01
Quail Run CL	953.26	10yr-24hr	805.90	1110.99	1125.50		1125.50	0.000000	0.09	7711.96	531.44	0.00
Quail Run CL	857.59	10yr-24hr	805.90	1109.98	1125.50		1125.50	0.000000	0.07	9646.64	617.04	0.00
Quail Run CL	722.3	10yr-24hr	805.90	1107.97	1125.50		1125.50	0.000000	0.03	15509.77	924.44	0.00
Quail Run CL	601.35	10yr-24hr	805.90	1104.94	1125.50		1125.50	0.000000	0.05	16518.99	1005.60	0.00
Quail Run CL	464.99	10yr-24hr	805.90	1102.00	1125.50		1125.50	0.000000	0.05	13745.41	837.39	0.00
Quail Run CL	344.26	10yr-24hr	805.90	1102.89	1125.50		1125.50	0.000000	0.11	6950.15	429.92	0.00
Quail Run CL	187.19	10yr-24hr	805.90	1101.00	1125.50		1125.50	0.000000	0.18	5348.03	347.57	0.01
Quail Run CL	107.9	10yr-24hr	805.90	1101.96	1125.50		1125.50	0.000000	0.16	5953.89	353.04	0.01
Quail Run CL	0	10yr-24hr	805.90	1101.00	1125.50	1101.95	1125.50	0.000000	0.13	7198.88	392.75	0.00





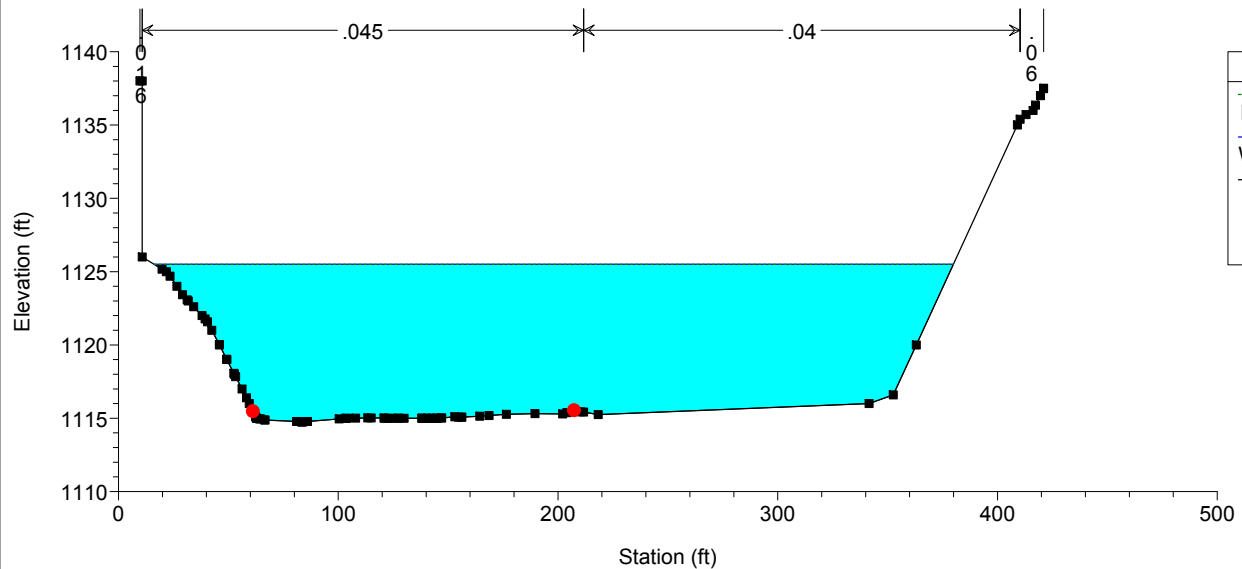
QuailRun BoxSprings Plan: PropCond Plan 10yr 24hr 3/31/2015

RS = 1282.03



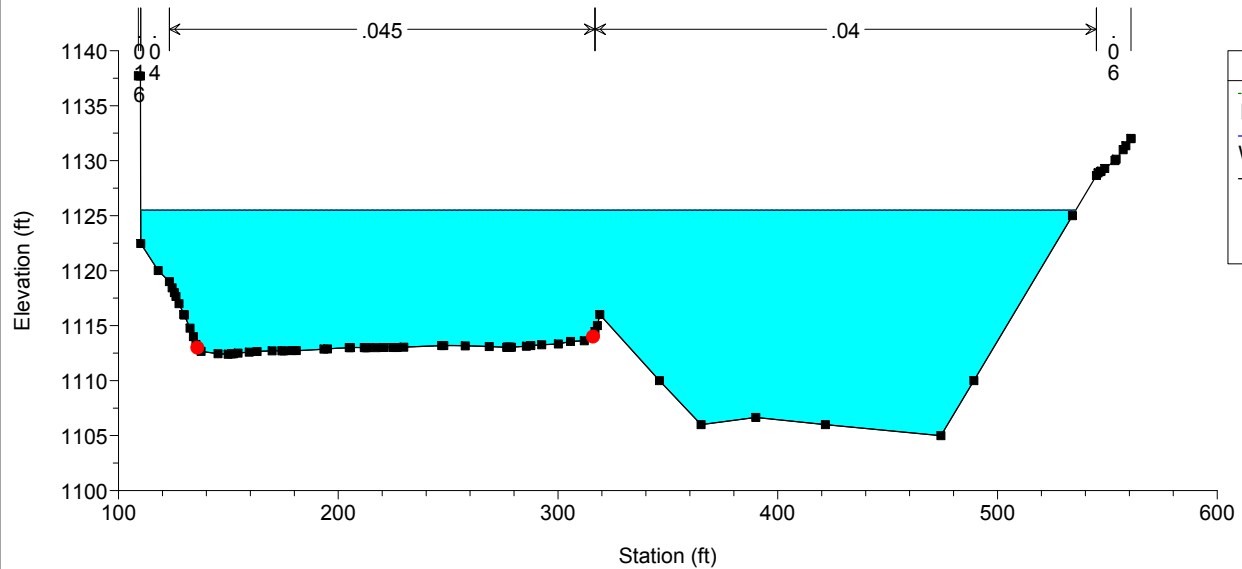
QuailRun BoxSprings Plan: PropCond Plan 10yr 24hr 3/31/2015

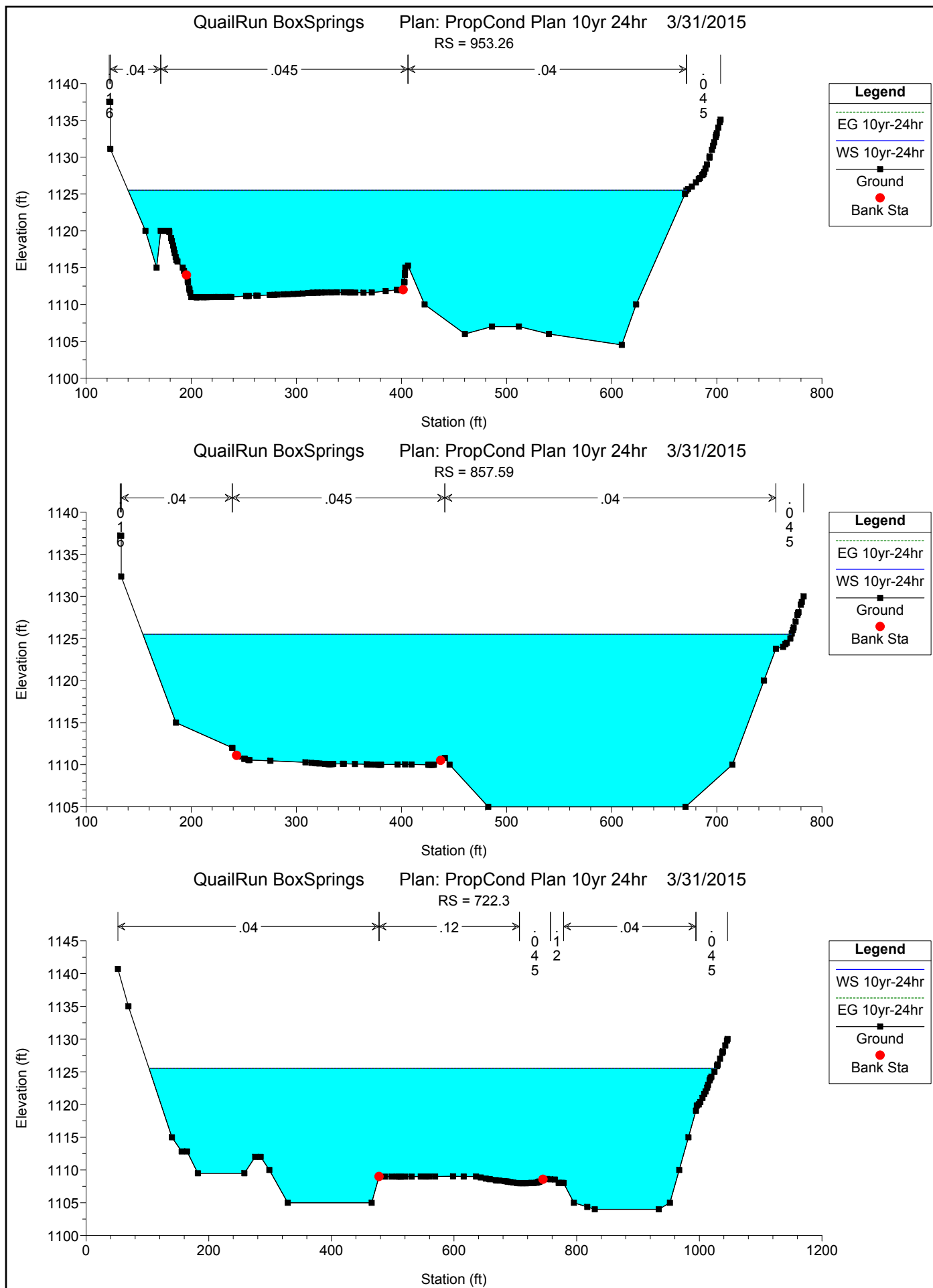
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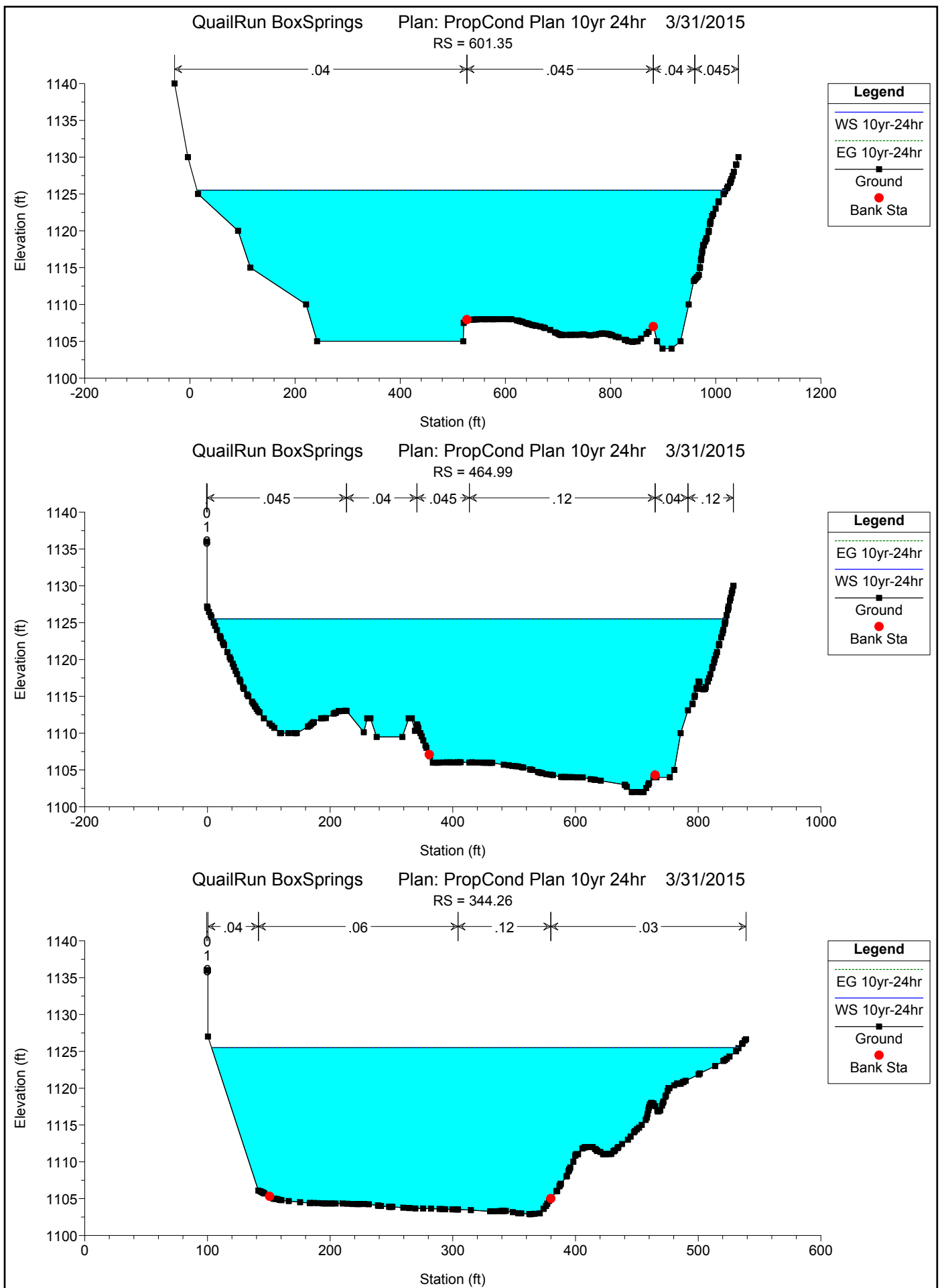


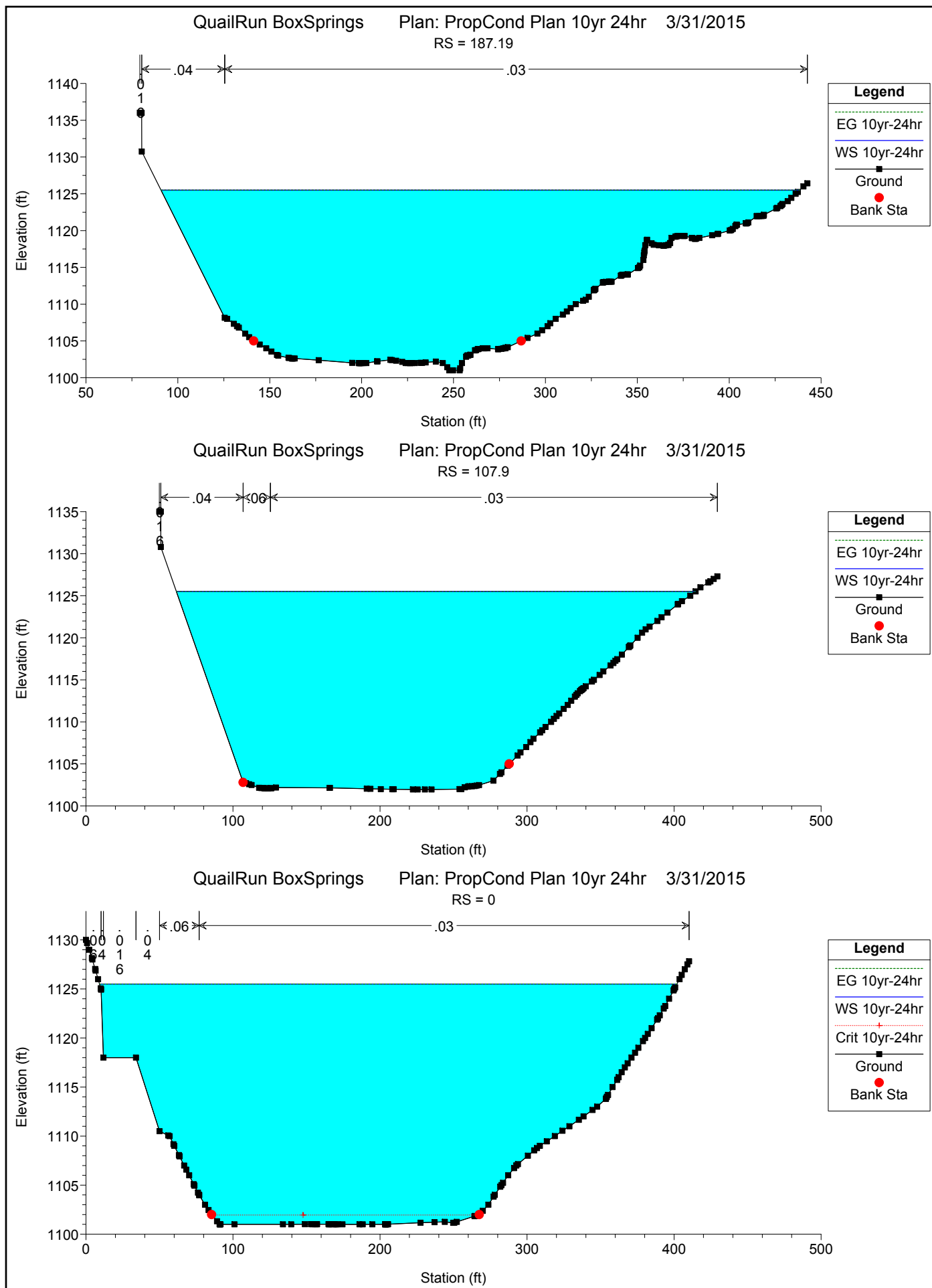
QuailRun BoxSprings Plan: PropCond Plan 10yr 24hr 3/31/2015

RS = 1092.5









APPENDIX 3

Box Springs Dam Drawings

CONSTRUCTION DRAWINGS

BOX SPRINGS DAM AND DIKE

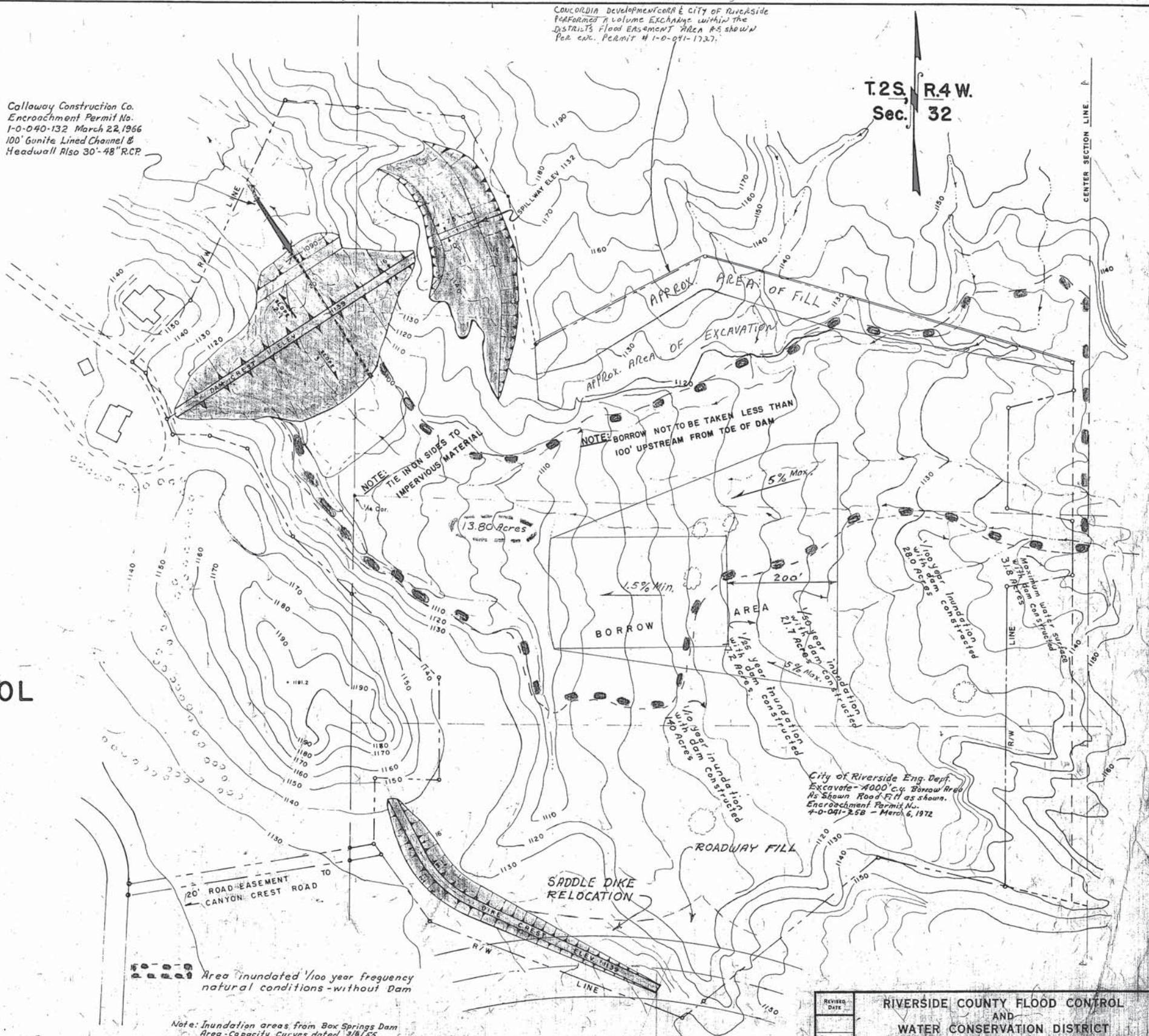
PREPARED BY

RIVERSIDE COUNTY FLOOD CONTROL
AND
WATER CONSERVATION DISTRICT

Calloway Construction Co.
Encroachment Permit No.
1-0-040-132 March 22, 1966
100' Gunite Lined Channel &
Headwall Also 30'-48" R.C.P.

CONCORDIA Development Co. & City of Riverside
Performed a Volume Exchange within the
District's Flood Easement Area as shown
Per exch. Permit # 1-0-041-1737.

T.2S. R.4W.
Sec. 32



Note: Inundation areas from Box Springs Dam
Area Capacity Curves dated 3/8/55

PLAN

100 50 0 50 100 FEET

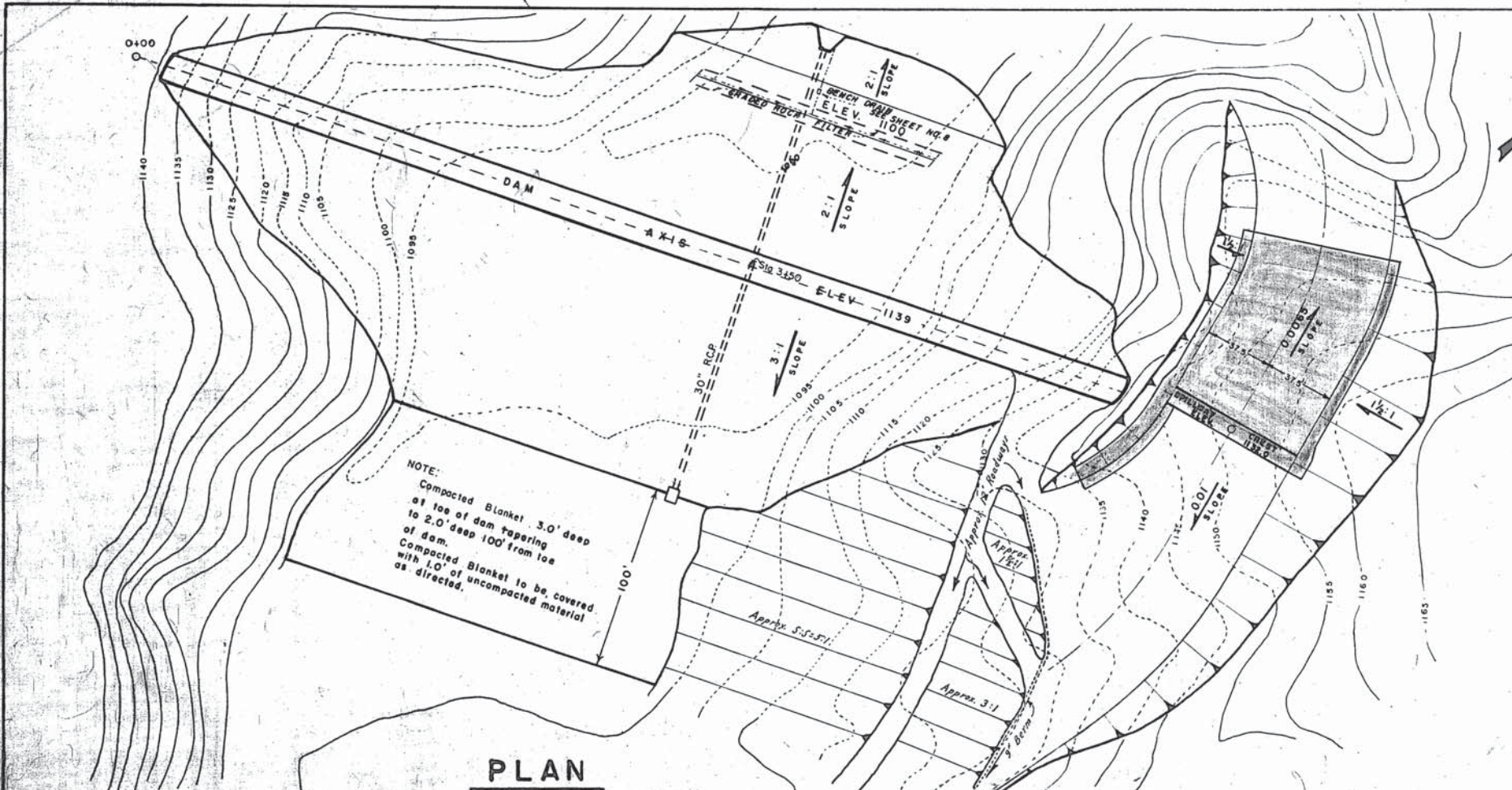
MARCH 1960

STATE OF CALIFORNIA DEPARTMENT OF WATER RESOURCES
APPLICATION NO. 1003-7
APPROVED AS TO SAFETY
9-10-59

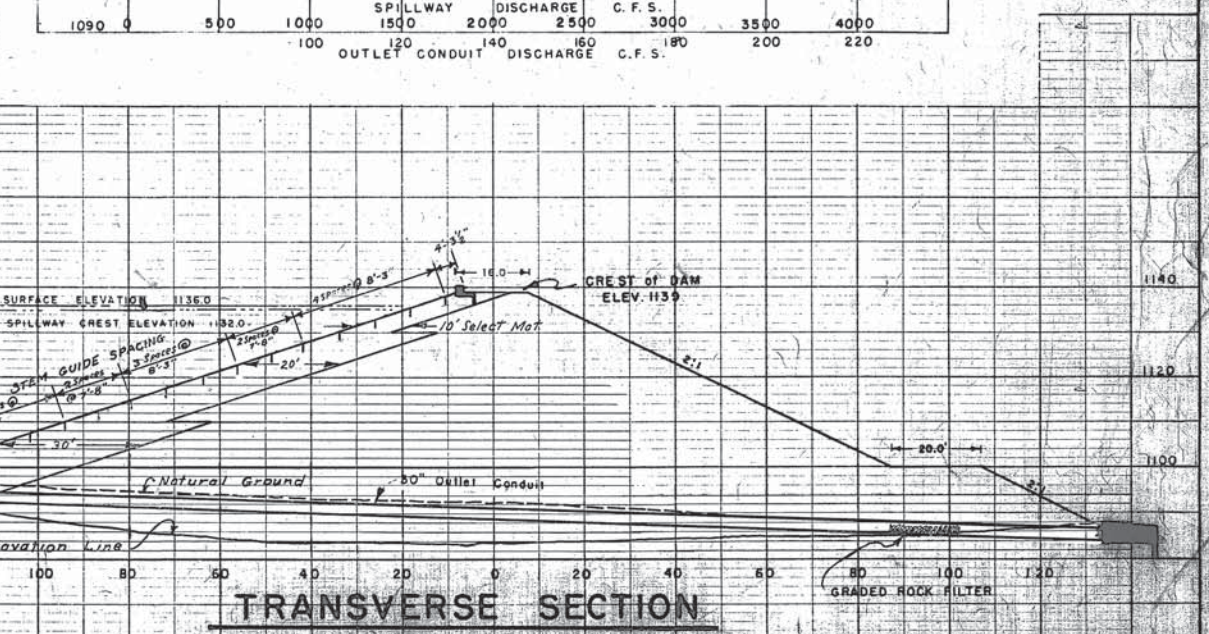
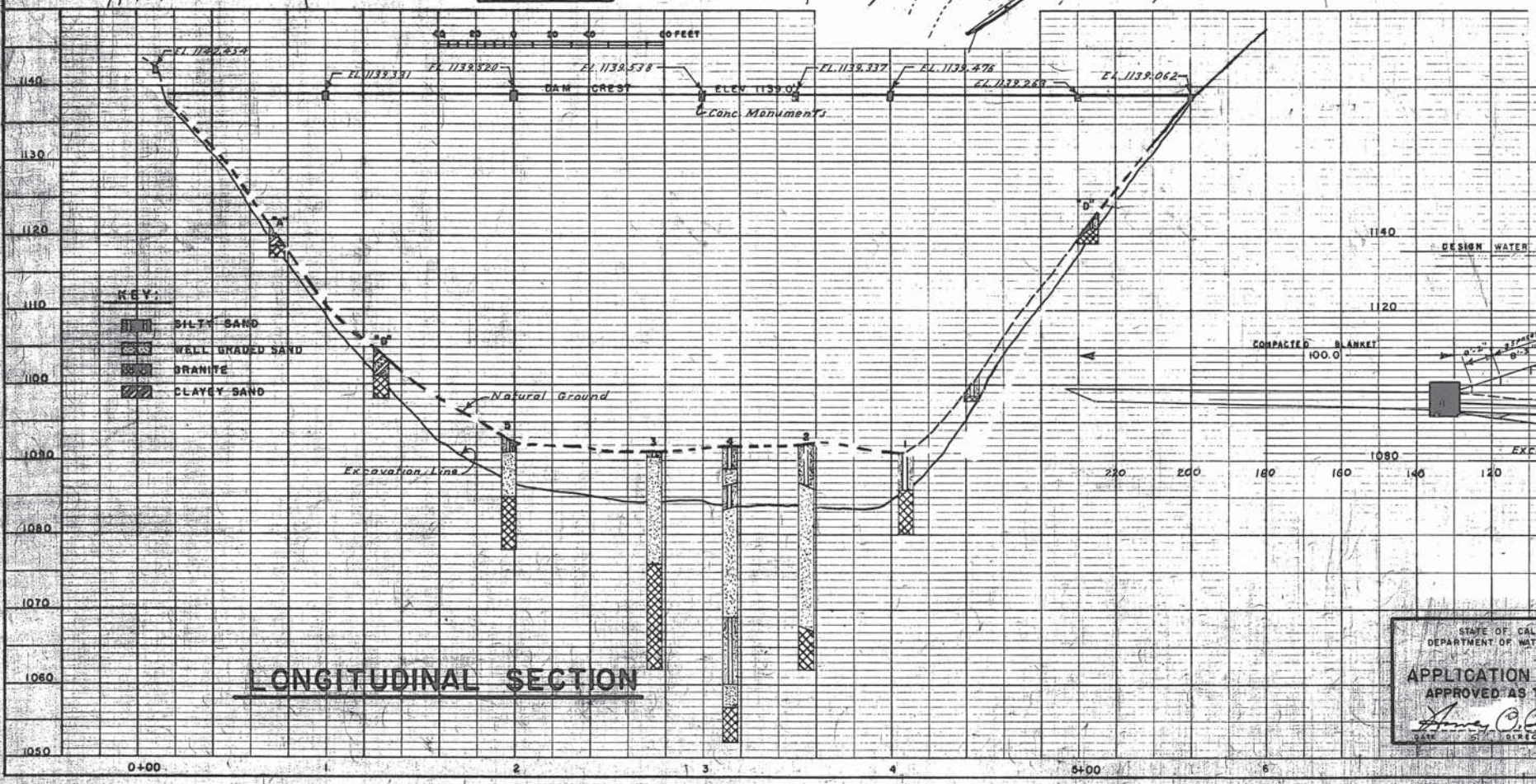
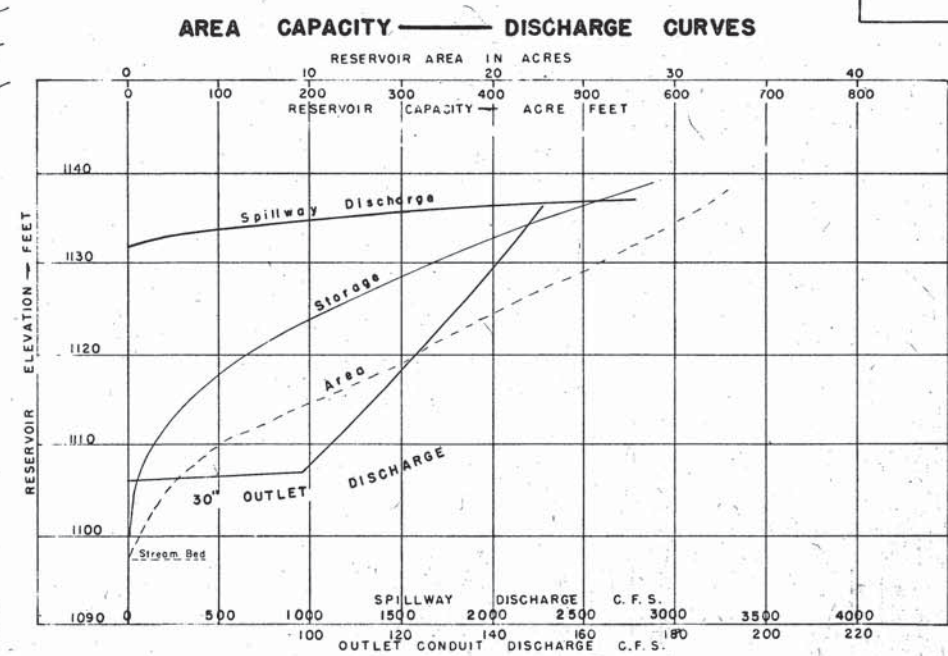
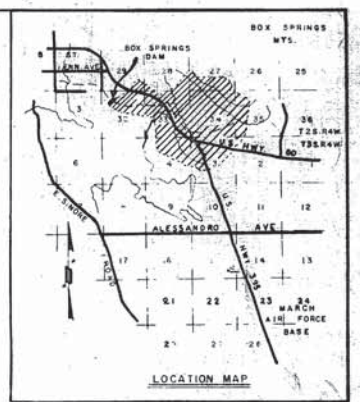
RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT	
BOX SPRINGS DAM	
APPROVED: <i>[Signature]</i> DATE: <i>[Date]</i>	CHECKED: D.M.M. DATE: Jan. 6, 1958
DRAWN: D.M.M. DATE: <i>[Date]</i>	SHEET NO. 10

DATE
BY
SURVEYED
PLOTTED
CHECKED
NOTE BOOK
BY OF MAP CHECKED
NO.

DATE
BY
SURVEYED
PLOTTED
CHECKED
NOTE BOOK
BY OF MAP CHECKED
NO.



T2S R4W
Sec. 32

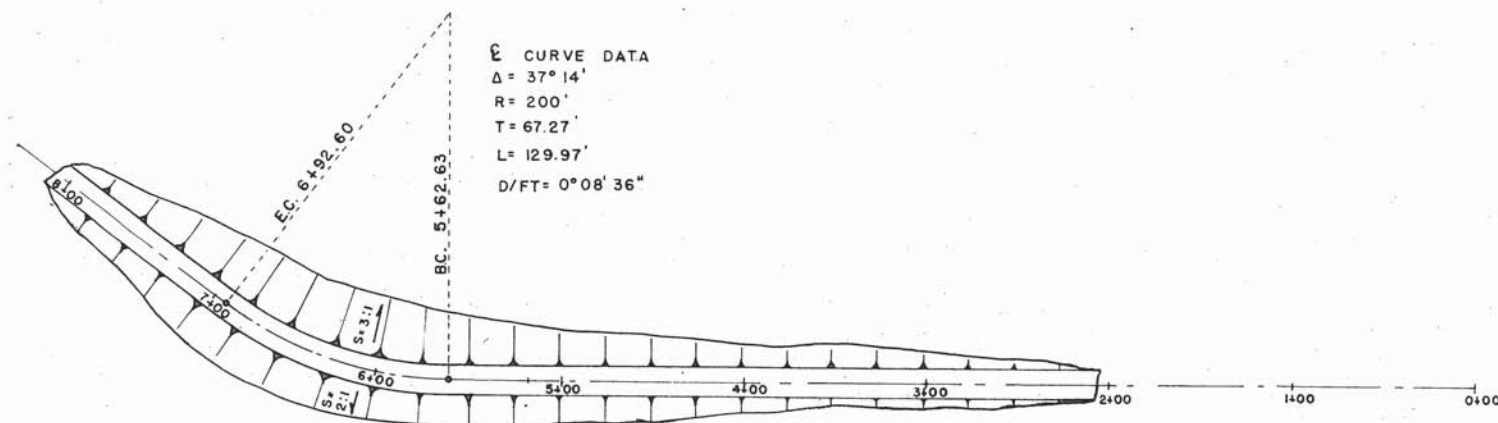


STATE OF CALIFORNIA
DEPARTMENT OF WATER RESOURCES
APPLICATION NO. 1003-7
APPROVED AS TO SAFETY
Amey C. B. B.
SAC, SAN JOSE DISTRICT

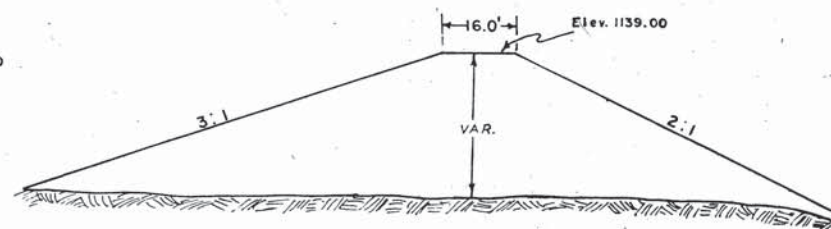
REVISOR
DATE
RIVERSIDE COUNTY FLOOD CONTROL
AND
WATER CONSERVATION DISTRICT
BOX SPRINGS DAM
PLAN & PROFILE
Drawn: C. Brown
Checked: J. D. D.
DATE: JAN 7, 1959
PROJECT NO. 1-0-14

PLAN	DATE	
	BY	
NOTE BOOK NO.	SURVEYED	
	PLOTTED	
	ALIGNED	
	CHECKED	
	RT. OF WAY	

PROFILE	DATE	
	BY	
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	STRUCTURE NOTATIONS	

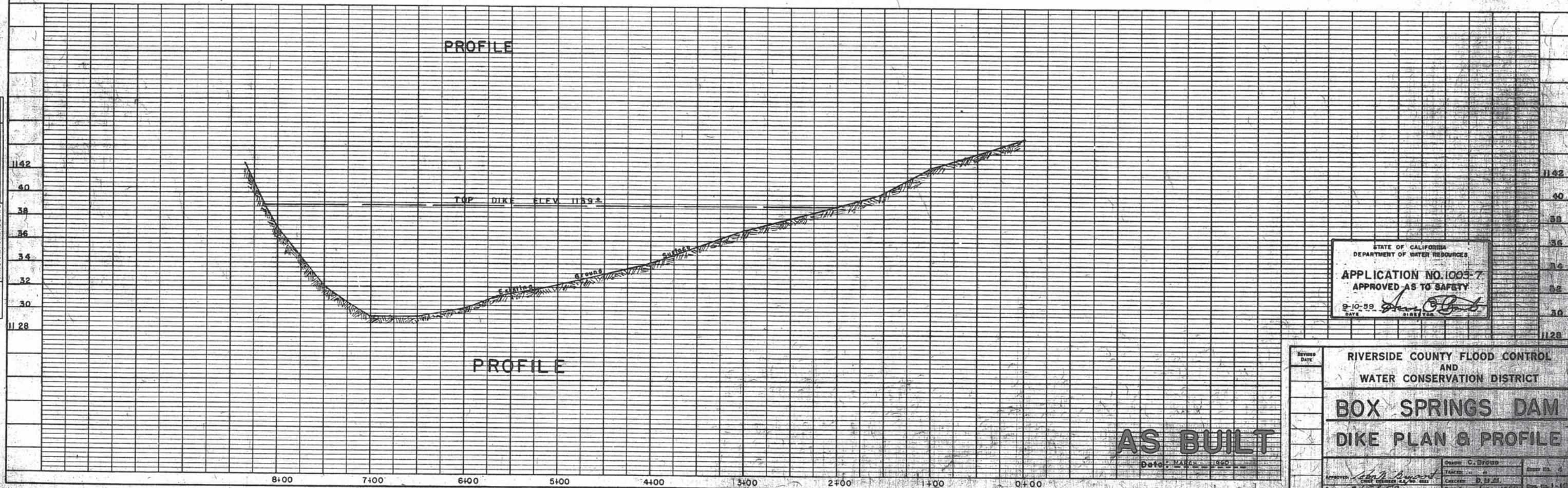


PLAN



TYPICAL SECTION

NO. SCALE



PROFILE

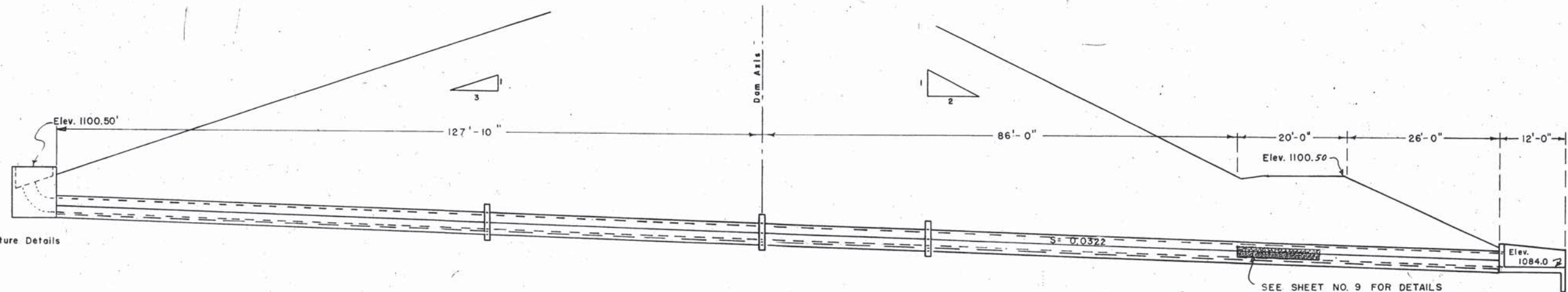
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AS BUILT

STATE OF CALIFORNIA DEPARTMENT OF WATER RESOURCES	
APPLICATION NO. 1003-7	
APPROVED AS TO SAFETY	
DATE: 9-10-59	SIGNATURE: [Signature]

RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT			
BOX SPRINGS DAM DIKE PLAN & PROFILE			
APPROVED: [Signature]	DESIGN: C. Briggs	CHECKED: D. H. H.	DATE: 10/1/59
PROJECT NO. 1-8-040			

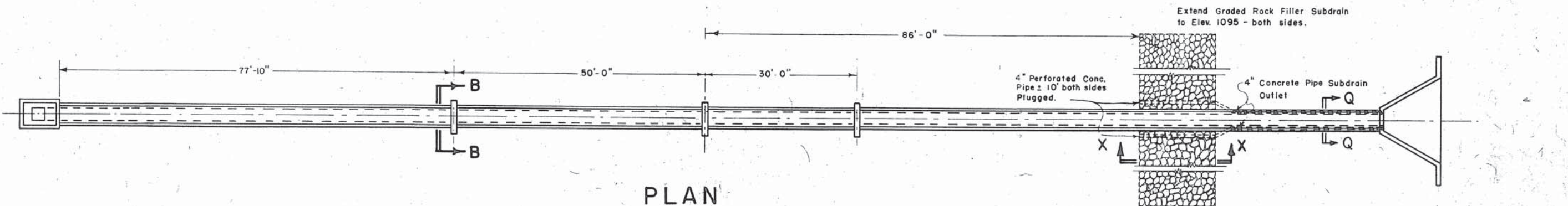
NOTE:
For Inlet Structure Details
See Sheet No. 7



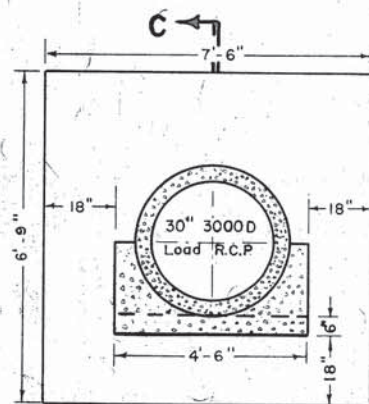
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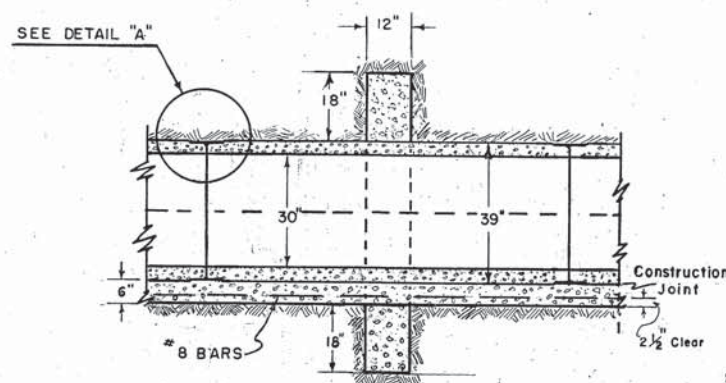
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For Outlet Structure Details
See Sheet No. 6



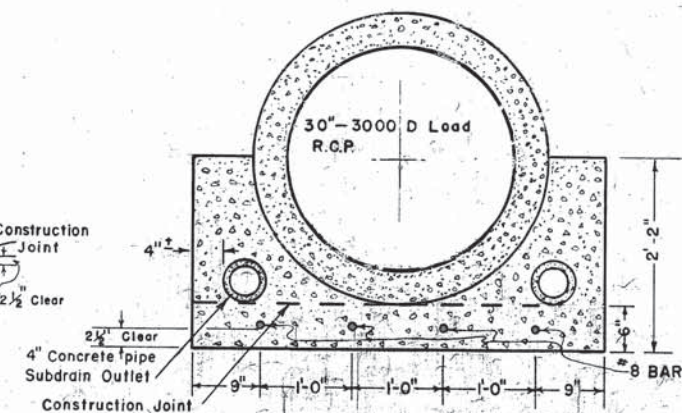
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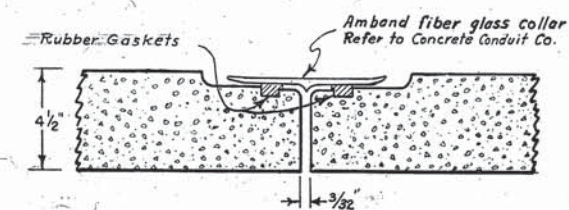
SECTION B-B



SECTION C-C



SECTION Q-Q



DETAIL - A

STATE OF CALIFORNIA
DEPARTMENT OF WATER RESOURCES
APPLICATION NO. 1003-7
APPROVED AS TO SAFETY
Anna O. Bond
S.E.T. 9-11-60

SECTION X-X

GRADED ROCK FILTER REQUIREMENTS

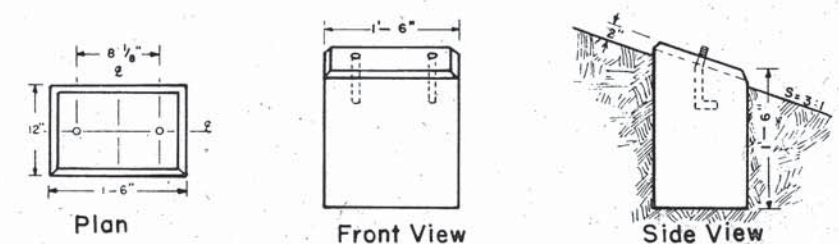
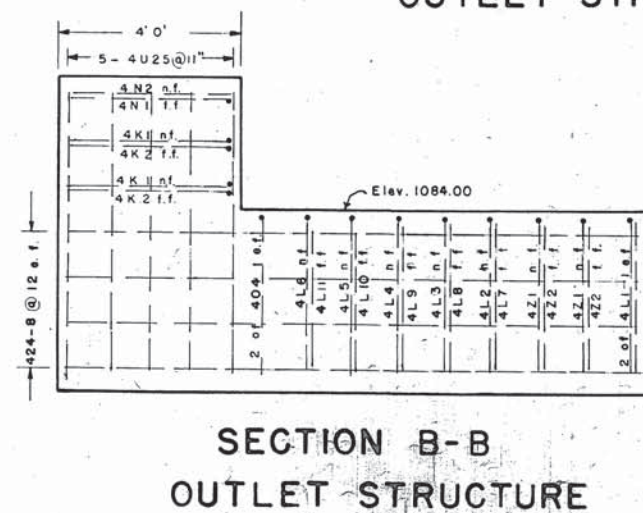
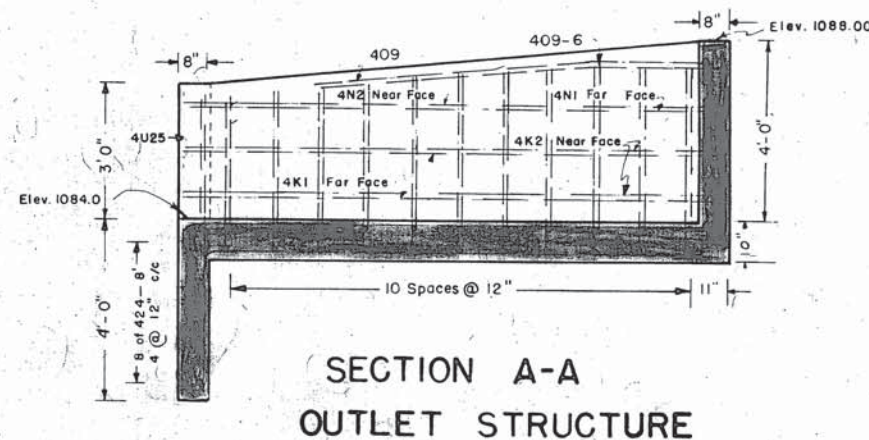
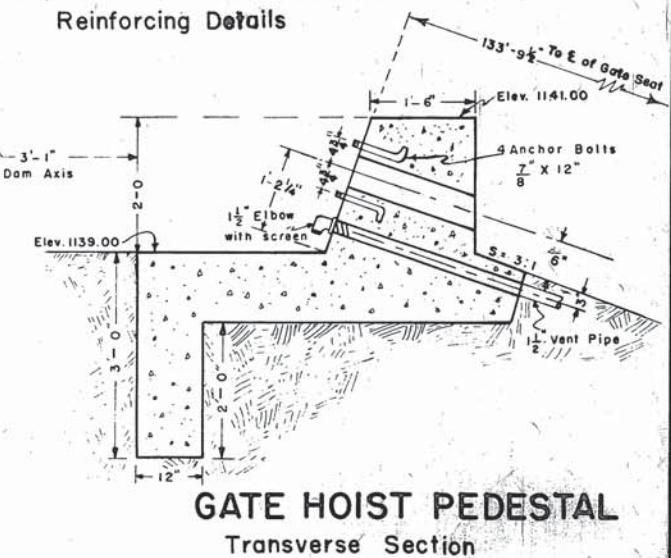
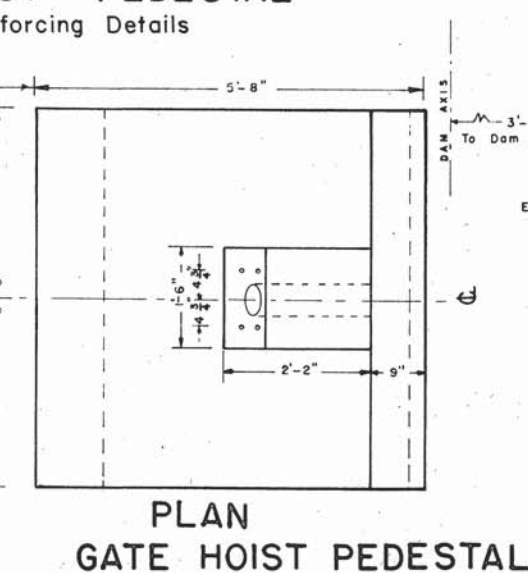
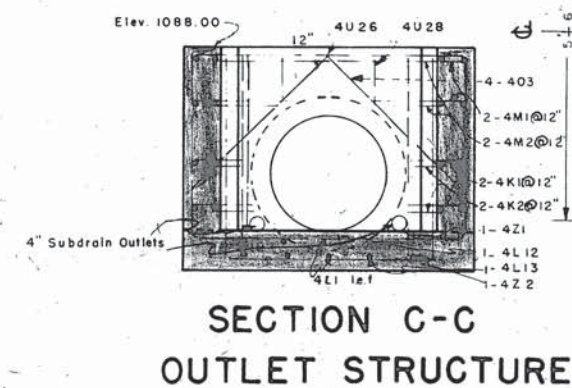
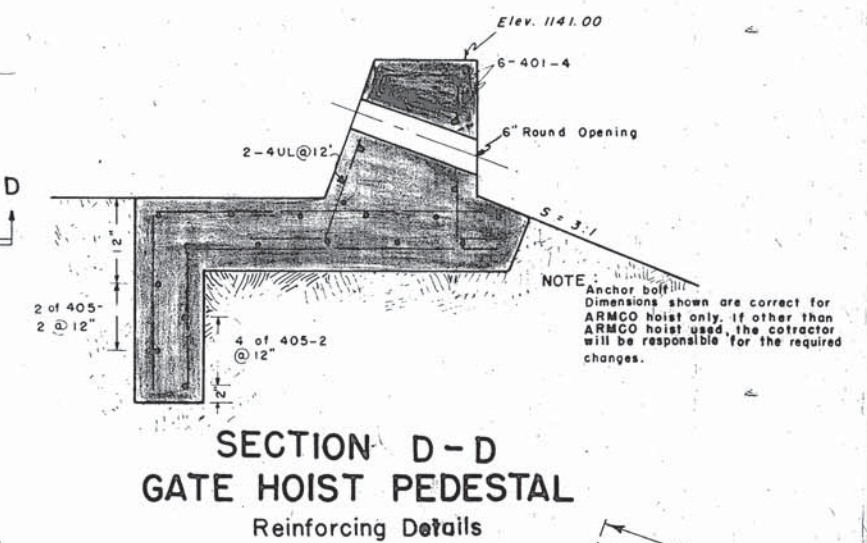
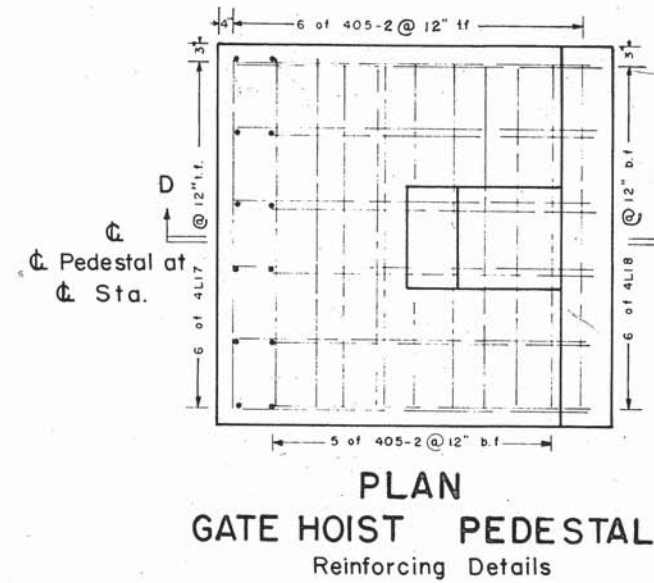
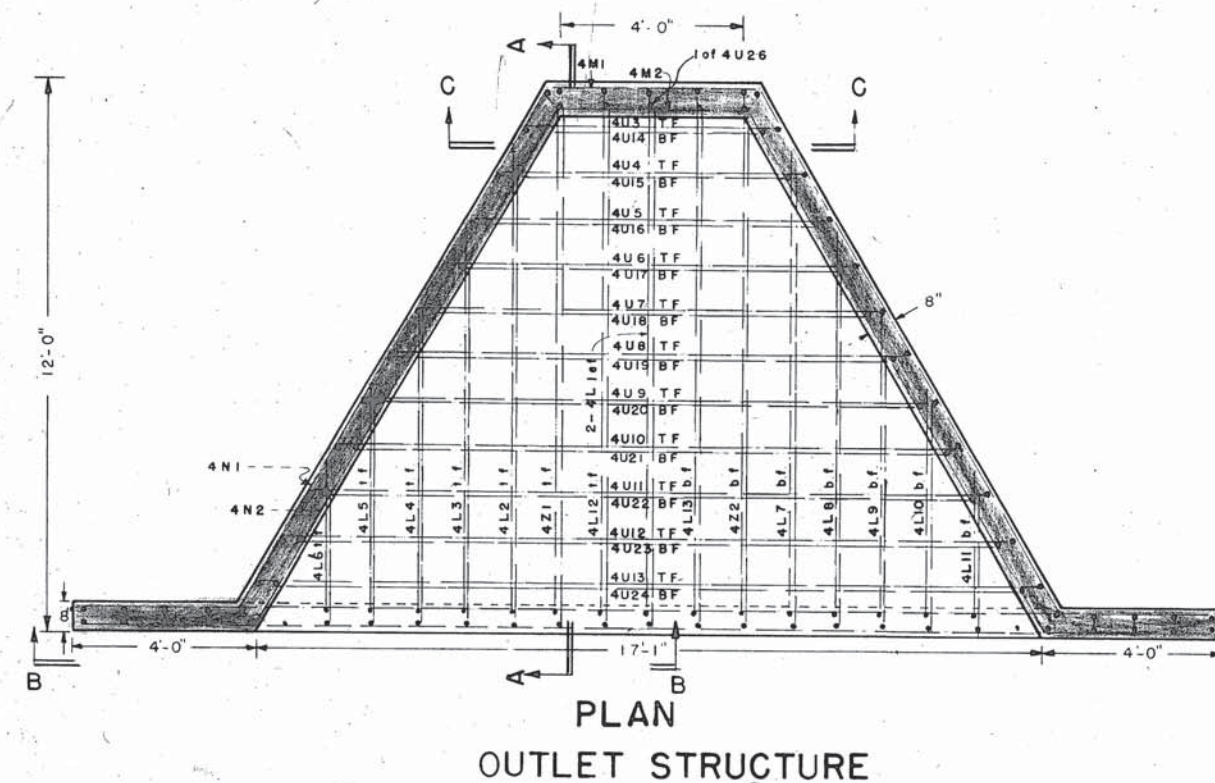
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1"	30-55
3/4"	20-40
3/8"	15-30
3/16"	0-10
1/8"	0

RIVERSIDE COUNTY FLOOD CONTROL
AND
WATER CONSERVATION DISTRICT

BOX SPRINGS DAM
--- DETAILS ---
30" OUTLET PIPE

APPROVED: *John A. Bond* DATE: 12/23/60
CHECKED BY: B.B. D.M. DATE: JULY 1965
PROJECT NO. 1-0-040

MARCH 1960



2" Gate Stem Guide Supports

TO BE INSTALLED AT 10 FOOT CENTERS OR AS DIRECTED
BY THE ENGINEER

NOTE: Dimensions of 2" Stem Guide Supports are typical for ARMCO fully adjustable stem guide Type I. The Contractor will be responsible for necessary adjustments. If an other type is used.

STATE OF CALIFORNIA
DEPARTMENT OF WATER RESOURCES

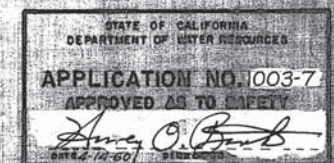
APPLICATION NO. 1003-
APPROVED AS TO SAFETY

9-10-59 *Anne O. Bond*
DATE _____ SIGNED BY _____

AS BUILT
Date: MARCH 1960

REVISED DATE	RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT		
	BOX SPRINGS DAM DETAILS OF		
	OUTLET STRUCTURE, GATE HOIST PEDESTAL & GATE STEM GUIDE SUPPORTS		
APPROVED	DATE	DRAWN	SHEET NO.
CIVIL ENGINEER R.E. ROY 4811	11-2-59	C. Brown	6
DATE	CHECKED	DATE DRAWN	DWG. NO.
	D.M.M.	11-2-59	1-101

PROJECT NO. I-0-040

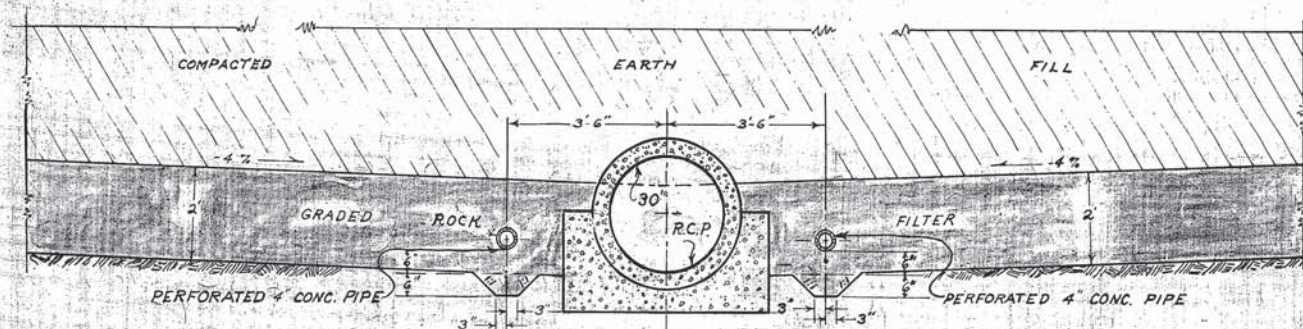
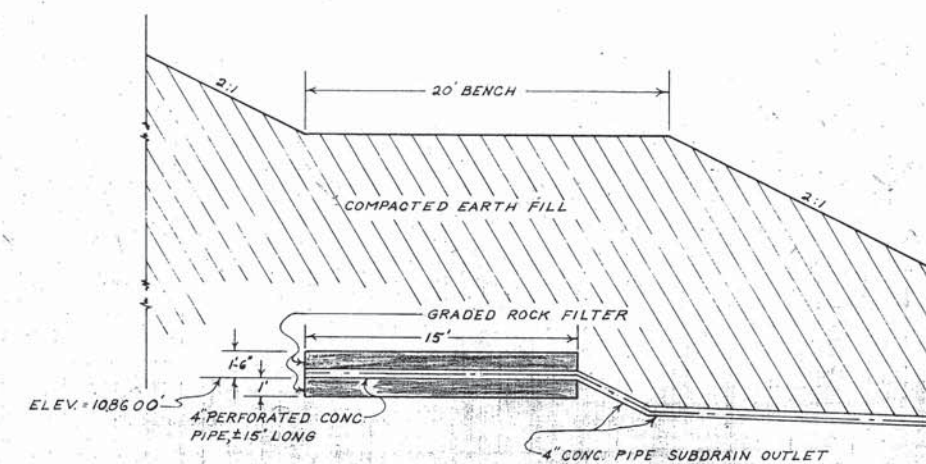
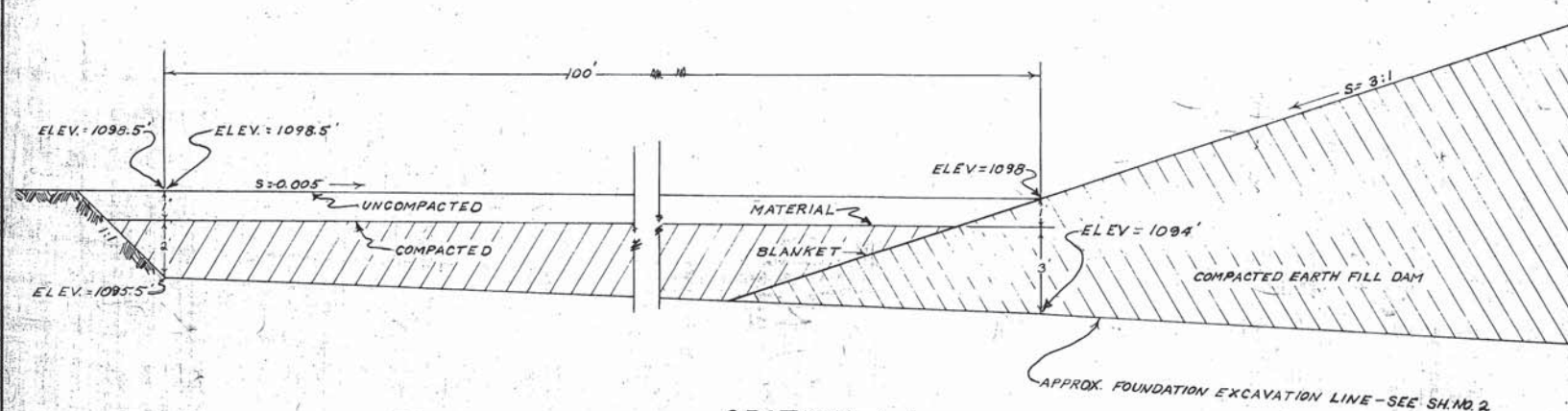
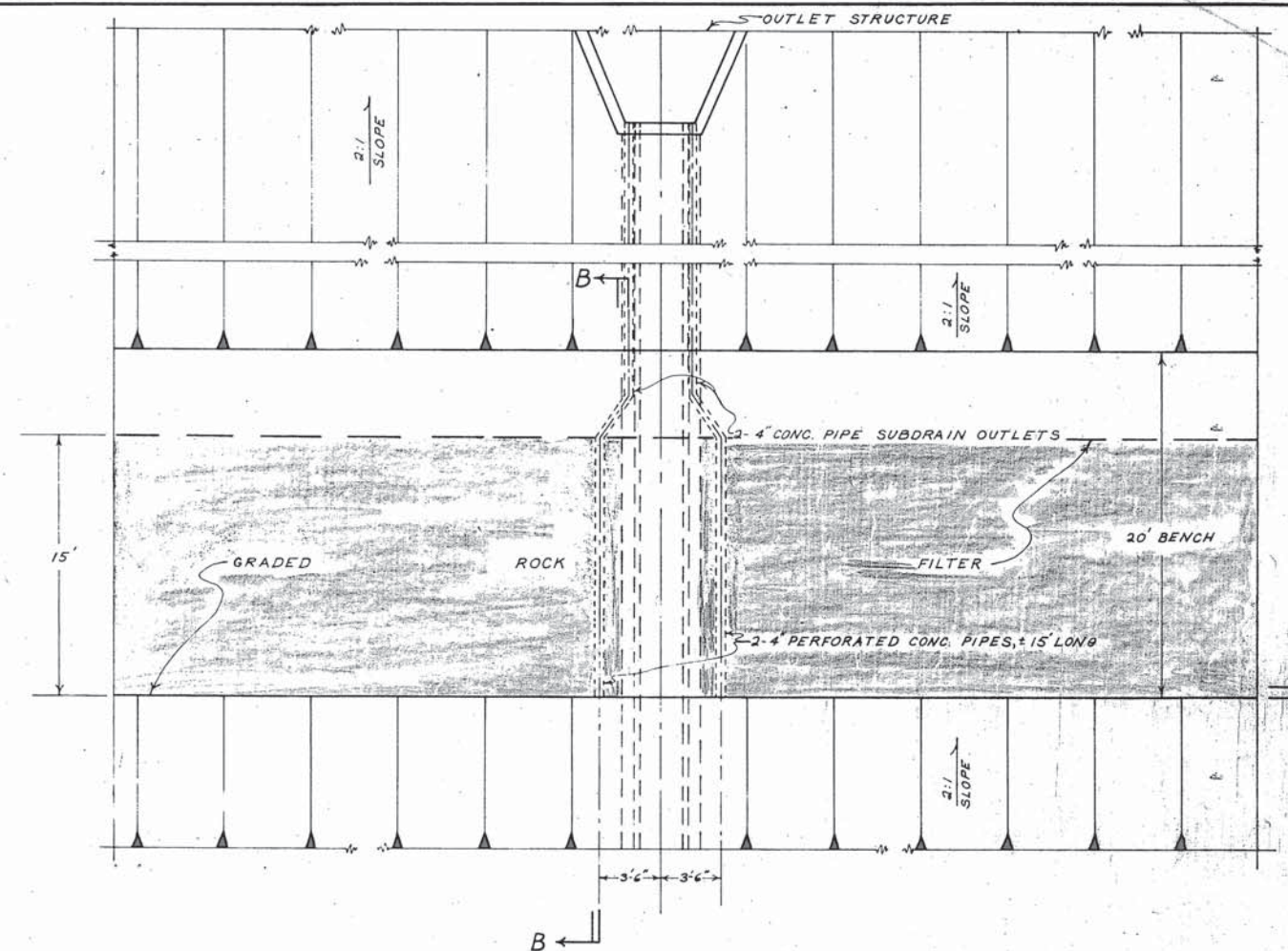
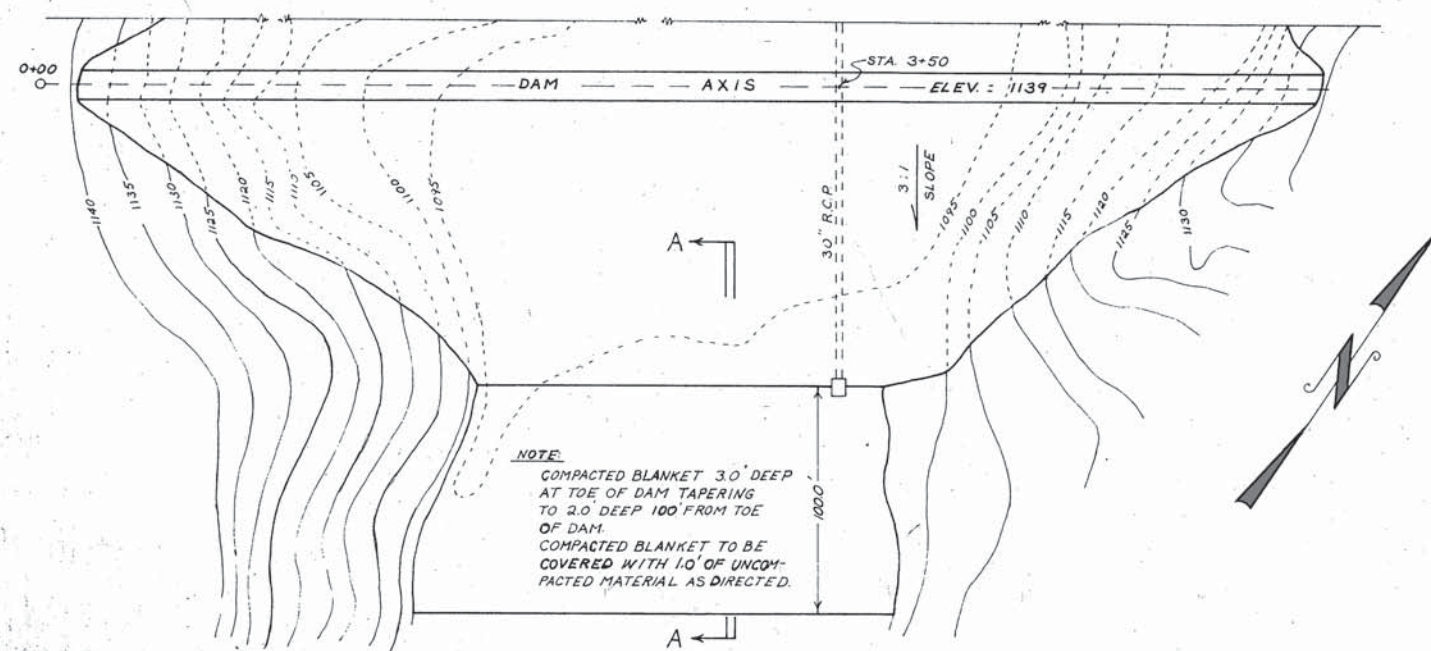


RETURN DATE 3/18/68	RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT BOX SPRINGS DAM BENCH DRAIN DETAIL		DRAWN: C. Brown CHECKED: D. M.M. DATE DRAWN: JUNE 1959	SHEET NO. 6 OF 6
APPROVED: <i>Robert Bryant</i> DATE: 12-3-60			DATE DRAWN: JUNE 1959	SHEET NO. 6 OF 6

AS BUILT

Date: MARCH 1960

PROJECT NO. 1-0-040



STATE OF CALIFORNIA
DEPARTMENT OF WATER RESOURCES

APPLICATION NO. 1003-7
APPROVED AS TO SAFETY

James O. Smith
DATE 4-4-60 ENGINEER

REVISED DATE	RIVERSIDE COUNTY FLOOD CONTROL AND WATER CONSERVATION DISTRICT		
3/11/60	<u>BOX SPRINGS DAM</u> <u>BLANKET & SUBDRAINAGE</u> <u>DETAILS</u>		
APPROVED	<i>[Signature]</i>	DRAWN R. DAVIS	SHEET NO. 9
CHECKED	<i>[Signature]</i>	TRACE R. DAVIS	
DATE	3/12/60	CHECKED DATE DRAWN 7/18/57	ONG. NO. 1-101
PROJECT NO. 1-0-040			

