





























1) The approximate lateral response of a single 24-inch diameter, 40-foot long pile is shown in the deflection, shear and moment diagrams provided above.

2) The analyses assume at least 10-foot embedment into dense Old Alluvium, with 4,000 psi concrete strength and (6) No. 10 transverse bars (Grade 60).



PROJECT NAME Riverside Community Hospital HCA Design and Construction

24-0011 FIGURE NUMBER

SD809

LATERAL PILE CAPACITY (GARAGE SITE - NORTH)



1) The approximate lateral response of a single 24-inch diameter, 40-foot long pile is shown in the deflection, shear and moment diagrams provided above.

2) The analyses assume at least 10-foot embedment into dense Old Alluvium, with 4,000 psi concrete strength and (6) No. 10 transverse bars (Grade 60).



LATERAL PILE C (GARAGE SITE -	SOUTH)
Riverside Community Hospital HCA Design and Construction	FIGURE NUMBER
ROJECT NAME	24-0011
SAN DIEGO, CA 92120 (050) 550-1000	04 0011

SD809



1) The approximate lateral response of a single 24-inch diameter, 40-foot long pile is shown in the deflection, shear and moment diagrams provided above.

2) The analyses assume at least 10-foot embedment into dense Old Alluvium, with 4,000 psi concrete strength and (6) No. 10 transverse bars (Grade 60).



24-0011 PROJECT NAME Riverside Community Hospital HCA Design and Construction

FIGURE NUMBER

SD809

LATERAL PILE CAPACITY (TOWER SITE - WEST)



1) The approximate lateral response of a single 24-inch diameter, 40-foot long pile is shown in the deflection, shear and moment diagrams provided above.

2) These analyses assume the eastern Tower piles are completely embedded within dense Old Alluvium, with a pile cut-off below Elevation 810 feet (MSL).



PROJECT NAME Riverside Community Hospital HCA Design and Construction LATERAL PILE CAPACITY (TOWER SITE - EAST)

24-0011 FIGURE NUMBER

SD809

MENT NUMBE



2) The analyses assume at least 10-foot embedment into dense Old Alluvium, with 4,000 psi concrete strength and (6) No. 14 transverse bars (Grade 60).

LATERAL PILE CAPACITY (GARAGE SITE - NORTH)

DELTA



1) The approximate lateral response of a single **30-inch diameter**, 40-foot long pile is shown in the deflection, shear and moment diagrams provided above.

2) The analyses assume at least 10-foot embedment into dense Old Alluvium, with 4,000 psi concrete strength and (6) No. 14 transverse bars (Grade 60).

PROJECT NAME Riverside Community Hospital HCA Design and Construction FIGURE NUMBER LATERAL PILE CAPACITY (GARAGE SITE - SOUTH)

DELTA

24-0011



1) The approximate lateral response of a single **30-inch diameter**, 40-foot long pile is shown in the deflection, shear and moment diagrams provided above.

2) The analyses assume at least 10-foot embedment into dense Old Alluvium, with 4,000 psi concrete strength and (6) No. 14 transverse bars (Grade 60).





1) The approximate lateral response of a single **30-inch diameter**, 40-foot long pile is shown in the deflection, shear and moment diagrams provided above.

2) These analyses assume the eastern Tower piles are completely embedded within dense Old Alluvium, with a pile cut-off below Elevation 810 feet (MSL).

SD809 IMENT NUMBE 24-0011 PROJECT NAME Riverside Community Hospital HCA Design and Construction FIGURE NUMBE



LATERAL PILE CAPACITY (TOWER SITE - EAST)







LAIERAL EARIN PRESSURES						
L EARTH RE TYPE	EQUIVALENT FLUID PRESSURE (PCF)					
VE, P _A	LEVEL BACKFILL	2:1 SLOPING BACKFILL				
CTED FILL	35	55				
SMIC ENT, ΔΡ _ε *	23					
VE, P _P **	300					
ARGE, P _s	0.3q					
	-					



LATERAL	EARTH	PRESSURES

L EARTH	EQUIVALENT FLUID PRESSURE (PCF)				
EST, P _o	LEVEL BACKFILL	2:1 SLOPING BACKFILL			
CTED FILL	60	90			
SMIC ENT, ΔΡ _ε *	(SEE FIGURE 9C)				
VE, P _P **	300				
ARGE, P _s	0.5q				



- 1) Perforated pipe should outlet through a solid pipe to a free gravity outfall. Perforated pipe and outlet pipe should have a fall of at least 1%.
- 2) As an alternative to the perforated pipe and outlet, weep-holes may be constructed. Weep-holes should be at least 2 inches in diameter, spaced no greater than 8 feet, and be located just above grade at the bottom of wall.
- 3) Filter fabric should consist of Mirafi 140N, Supac 5NP, Amoco 4599, or similar approved fabric. Filter fabric should be overlapped at least 6-inches.
- 4) Geocomposite panel drain should consist of Miradrain 6000, J-DRain 400, Supac DS-15, or approved similar product.



APPENDIX A FIELD EXPLORATION

APPENDIX A

FIELD EXPLORATION

The field exploration program included a geologic reconnaissance of the site, the advancement of 5 cone penetrometer test (CPT) soundings, the drilling of 10 exploratory borings, and borehole percolation testing at 12 locations. The subsurface explorations and field infiltration tests were completed between March 22nd and April 5th, 2024. The CPT soundings were advanced by Kehoe Testing and Engineering. The exploratory borings were advanced by Yellow Jacket Drilling. The infiltration tests were completed by Group Delta personnel using a hand auger. The exploration locations are shown on the Exploration Plans, Figures 3A to 3C. The CPT data and interpreted soil profiles are presented in Figures A-1 to A-5. Boring Records are provided in Figures A-6 to A-15. The field infiltration test results are discussed in the text of this report and presented in detail in the figures of Appendix D. The subsurface explorations are summarized in the table below.

Exploration	Exploration	Latitude	Longitude	Ground Surface	Exploration	Figure
ID	Date			Elevation [FT]	Depth [FT]	No.
CPT-1	03/22/24	33.977986°	-117.382803°	800	34.0	A-1
CPT-2	03/22/24	33.977770°	-117.383393°	796	37.5	A-2
CPT-3	03/22/24	33.977144°	-117.383833°	793	20.7	A-3
CPT-4	03/22/24	33.976485°	-117.383034°	794	25.0	A-4
CPT-5	03/22/24	33.976042°	-117.381919°	839	54.7	A-5
	1	1	1	ſ		
B-01	04/04/24	33.978130°	-117.383124°	800	36½	A-6
B-02	04/01/24	33.977907°	-117.383271°	798	51½	A-7
B-03	04/02/24	33.977754°	-117.382979°	798	36½	A-8
B-04	04/04/24	33.977536°	-117.383915°	794	31	A-9
B-05	04/04/24	33.977440°	-117.383294°	796	31½	A-10
B-06	04/03/24	33.977308°	-117.384145°	794	31½	A-11
B-07	04/04/24	33.976992°	-117.383266°	794	28	A-12
B-08	04/01/24	33.976503°	-117.383073°	794	30	A-13
B-09	04/05/24	33.976063°	-117.382178°	832	31½	A-14
B-10	04/02/24	33.976190°	-117.381820°	838	51½	A-15

The 10 CPT soundings were advanced by Kehoe Testing and Engineering in general accordance with ASTM D5778 using a 30-ton truck mounted rig with a 15 cm² cone. Integrated electronic circuitry was used to measure the tip resistance (Qc) and skin friction (Fs) at one-inch intervals while the CPT was advanced into the soil using hydraulic down pressure. Note that each of the CPT soundings was pushed to practical refusal, as indicated by a CPT tip resistance in excess of 700 tons per square foot (TSF). A piezometer located behind the cone tip also measured transient pore pressure (u). The CPT data was used to characterize the soil profile are based on normalized cone resistance and friction ratio interpretations (Robertson, 2010). The first figure for each CPT sounding presents both the raw CPT data and the interpreted soil profile (Figures A-1a to A-5a). The raw CPT data is also shown in more detail in the following Figures A-1b to A-5b for each CPT sounding.



APPENDIX A

FIELD EXPLORATION (Continued)

At the location of soundings CPT-1, CPT-2 and CPT-5, shear wave velocity measurements were collected at 5-foot depth intervals using an air actuated hammer located inside the front jack of the rig. The interval shear wave data measured in each of these three CPT soundings is attached immediately after the interpreted soil parameters. The average shear wave velocity measured within the upper 34 to 35 feet (Vs_d) at the location of CPT-1 and CPT-2 was 876 to 899 ft/s. If we assume a uniform shear wave velocity of 1,940 ft/s for the Old Alluvium below that depth based on the lower bound of the shear wave velocity measurements conducted by others, the average shear wave velocity for the upper 100 feet of the soil profile (Vs₃₀) for the Garage site would be about 1,390 ft/s (CHJ, 2007). This correlates to a 2022 CBC Site Class C for the proposed Garage site.

The average shear wave velocity measured within the upper 55 feet (Vs_d) of the compacted fill and Old Alluvium along the east end of Tower Site in CPT-5 was 1,390 ft/s. If we again assume a shear wave velocity of 1,940 ft/s for the Old Alluvium below that depth, the average shear wave velocity for the upper 100 feet of the soil profile (Vs₃₀) at the east end of the Tower site is estimated at 1,600 ft/s (CHJ, 2007). This also corresponds to a 2022 CBC Site Class C for the proposed Tower site.

The exploratory borings were advanced by Yellow Jacket Drilling using their CME 75 limited access track mounted rig, and their CME 85 truck mounted rig. Disturbed soil samples were collected from the exploratory borings using a 2-inch outside diameter Standard Penetration Test (SPT) sampler. Less disturbed samples were collected using a 3-inch outside diameter ring lined sampler (a modified California sampler). Automatic hammers with calibrated Energy Transfer Ratios (ETR) ranging from approximately 74 to 80 percent were used to collect all of the drive samples. For each sample, the number of blows needed to drive the sampler 12 inches was recorded on the logs. The field blow counts (N) were normalized to approximate a standard 60 percent ETR as shown on the logs (N_{60}). Bulk samples were also collected from the explorations at selected intervals.

The field exploration locations were determined by visually estimating, pacing and taping distances from landmarks shown on the Exploration Plans, Figures 3A to 3C. The locations shown should not be considered more accurate than is implied by the method of measurement used and the scale of the map. The lines designating the interface between differing soil materials on the logs may be abrupt or gradational. Further, soil conditions at locations between the excavations may be substantially different from those at the specific locations we explored. It should be noted that the passage of time may also result in changes in the soil conditions reported in the logs.

Logs for several previous field explorations completed by others are also included in Appendix A1 for reference. These logs include Borings B-1, B-2, B-6, B-7, B-8 and B-9 as well as CPT-1 to CPT-6 at the proposed Tower site (CHJ, 2008). Borings B-1 and B-2 are also included from a previous investigation for the MOB located along the southern edge of the Garage site (CHJ, 2012). The approximate locations of the relevant previous explorations are shown in Figures 3B and 3C.

	DESCRIPTION SEQUENCE				
ence		Refe Sec	er to tion	ired	onal
Sequ	Identification Components	Field	Lab	Requ	Optic
1	Group Name	2.5.2	3.2.2	•	0
2	Group Symbol	2.5.2	3.2.2	•	
	Description Components				
3	Consistency of Cohesive Soil	2.5.3	3.2.3	•	
4	Apparent Density of Cohesionless Soil	2.5.4		•	
5	Color	2.5.5		•	
6	Moisture	2.5.6		٠	
	Percent of Proportion of Soil	2.5.7	3.2.4	٠	•
7	Particle Size	2.5.8	2.5.8	•	•
	Particle Angularity	2.5.9			0
	Particle Shape	2.5.10			0
8	Plasticity (for fine-grained soil)	2.5.11	3.2.5		0
9	Dry Strength (for fine-grained soil)	2.5.12			0
10	Dilatency (for fine-grained soil)	2.5.13			0
11	fine-grained soil)	2.5.14			0
12	Structure	2.5.15			0
13	Cementation	2.5.16		•	
14	Percent of Cobbles and Boulders	2.5.17		•	
14	Description of Cobbles and Boulders	2.5.18		•	
15	Consistency Field Test Result	2.5.3		•	
16	Additional	2.5.19			0

SOIL IDENTIFICATION AND

EXPLORATION IDENTIFICATION

Explorations are identified using the following convention

H-YY-NNN

Where:

H: Exploration type code

YY: 2-digit year (where utilized)

NNN: Exploration number

Hole Type Code and Description

Hole Type Code	Description
А	Auger Boring (Hollow or solid stem bucket)
BA	Bucket Auger
СРТ	Cone Penetration Test
D	Driven (dynamic cone penetrometer)
HA	Hand Auger
HD	Hand driven (1-inch soil tube)
0	Other (note on LOTB)
Р	Rotary Percussion Boring (AIr)
R	Rotary drilled boring (Conventional)
RC	Rotary core (self-cased wire-line, continuosly sampled)
RW	Rotary cored (self cased wire-line, not continuosly sampled)
TP	Test Pit

Description Sequence Examples:

SANDY lean CLAY (CL); very stiff; yellowish brown; moist; mostly fines; some SAND, from fine to medium; few gravels; medium plasticity; PP=2,75.

Well-graded SAND with SILT and GRAVEL and COBBLES (SW-SM); dense; brown; moist; mostly SAND, from fine to coarse; some fine GRAVEL; few fines; weak cementation; 10% GRANITE COBBLES; 3 to 6 inches; hard, subrounded.

Clayey SAND (SC) medium dense, light brown; wet; mostly fine sand; little fines; low plasticity.



REFERENCE: Caltrans Soil and Rock Logging, Classification, and Presentation Manual (2010).

Minimum Required Sequence:

Particle Size; Plasticity (optional).

Cementation; % cobbles & boulders; Description of cobbles & boulders; Consistency field test result

Where applicable:

• =

USCS Group Name (Group Symbol); Consistency or

optional for non-Caltrans projects

Density; Color; Moisture; Percent of Proportion of Soil;

	10	GROUP SYMBO	DLS AI	ND NA	MES		FIEL	D AND LABORATORY TESTING
raphic	/Symbol	Group Names	Graphi	c/Symbol	Group Names	с	Cons	olidation (ASTM D 2435)
5	GW	Well-graded GRAVEL Well-graded GRAVEL with SAND			Lean CLAY Lean CLAY with SAND	CL	Colla	pse Potential (ASTM D 4546)
रत व				CL	SANDY lean CLAY	СР	Com	paction Curve (CTM 216)
500	GP	Poorly graded GRAVEL	\langle / \rangle		GRAVELLY lean CLAY with GRAVEL	CR	Corro	osion, Sulfates, Chlorides (CTM 643; CTM 417;
020					GRAVELLY lean CLAY with SAND		CTM	422) olidated Undrained Triavial (ASTM D 4767)
	GW-GM	Well-graded GRAVEL with SILT and CAND			SILTY CLAY with SAND		Direc	t Shear (ASTM D 3080)
Цļ		Well-graded GRAVEL with CLAY (or SILTY		CL-ML	SANDY SILTY CLAY		Expe	nsion Index (ASTM D 4829)
	GW-GC	CLAY) Well-graded GRAVEL with CLAY and SAND			GRAVELLY SILTY CLAY	M	Moie	ture Content (ASTM D 2216)
2019		(or SILTY CLAY and SAND)	/		SILT		0rg2	nic Content (ASTM D 2974)
japp.	GP-GM	Poorly graded GRAVEL with SILT Poorly graded GRAVEL with SILT and SAND			SILT with SAND SILT with GRAVEI	p	Porm	reability (CTM 220)
804		Poorly graded GRAVEL with CLAY (or SILTY		ML	SANDY SILT with GRAVE		Parti	rle Size Analysis (ASTM D 6013
32	GP-GC	CLAY) Poorly graded GRAVEL with CLAY and SAND			GRAVELLY SILT		ASTN	1 D 7928)
<u>k</u>		(or SILLY CLAY and SAND)			ORGANIC lean CLAY	PI	Liqui (AAS	a Limit, Plastic Limit, Plasticity Index HTO T 89, AASHTO T 90)
	GM	SILTY GRAVEL with SAND	P		ORGANIC lean CLAY with SAND ORGANIC lean CLAY with GRAVEL	PL	Point	Load Index (ASTM D 5731)
5		CLAYEY GRAVEL	\mathcal{D}	OL	SANDY ORGANIC lean CLAY SANDY ORGANIC lean CLAY	PM	Press	ure Meter
3%)	GC	CLAYEY GRAVEL with SAND	\mathbb{R}		GRAVELLY ORGANIC lean CLAY	R	R-Va	ue (CTM 301)
			655		ORGANIC SILT	SE	Sand	Equivalent (CTM 217)
122	GC-GM	SILTY, CLAYEY GRAVEL with SAND	$ \langle \langle \langle $		ORGANIC SILT with SAND	SG	Speci	fic Gravity (AASHTO T 100)
٢			$\langle \langle $	OL	SANDY ORGANIC SILT	SL	Shrin	kage Limit (ASTM D 427)
	SW	Well-graded SAND with GPAVE	{}}		GRAVELLY ORGANIC SILT	sw	Swel	Potential (ASTM D 4546)
· • -					Fat CLAY	UC	Unco	nfined Compression - Soil (ASTM D 2166)
	SP				Fat CLAY with SAND	00	Unco Unco	ntined Compression - Rock (ASTM D 2938) nsolidated Undrained Triaxial (ASTM D 2850)
		Woll graded SAND with SUT		СН	SANDY fat CLAY with CDAVIC	UW	Unit	Weight (ASTM D 2937)
	SW-SM	Well-graded SAND with SILT and CRAVE				WA	Perce	ent passing the No. 200 Sieve (ASTM D 1140)
		Well-graded SAND with CLAY (or SILTY			GRAVELLY fat CLAY with SAND			
	SW-SC	CLAY) Well-graded SAND with CLAY and GRAVEL			Elastic SILT with SAND		SA	MPLER GRAPHIC SYMBOLS
		(or SIETY CLAY and GRAVEL)		MH	SANDY elastic SILT			
	SP-SM	Poorly graded SAND with SILT			GRAVELLY elastic SILT			
-		Poorly graded SAND with SILI and GRAVEL	Ц <u>Ц</u>		GRAVELLY elastic SILT with SAND	$\mid M$	Stand	dard Penetration Test (SPT)
	SP-SC	CLAY) Poorly graded SAND with CLAY and GRAVE	DP.		ORGANIC fat CLAY with SAND			
-[4]		(or SILTY CLAY and GRAVEL)	151	ОН	SANDY ORGANIC fat CLAY			
	SM		PP		SANDY ORGANIC fat CLAY with GRAVEL GRAVELLY ORGANIC fat CLAY		Modi	itied California Sampler (2.4" ID, 3" OD)
IJIJ			R		GRAVELLY ORGANIC fat CLAY with SAND			
	SC	CLAYEY SAND	$ \langle \langle \langle $		ORGANIC elastic SILT with SAND		Shelb	y Tube
44			$ \langle \rangle \rangle$	ОН	SANDY elastic SILI with GRAVEL			
	SC-SM		$ \rangle\rangle\rangle $		SANDY ORGANIC elastic SILT with GRAVEL GRAVELLY ORGANIC elastic SILT		NX R	ack Core HO Bock Core
μĶ		SILIT, CLATET SAND WITH GRAVEL			GRAVELLY ORGANIC elastic SILT with SAND			
21-21-	PT	PEAT			ORGANIC SOIL with SAND	889		
				OL/OH	SANDY ORGANIC SOIL		Bulk	Sampler Other (see remarks)
54		COBBLES AND BOULDERS	PA		GRAVELLY ORGANIC SOIL with GRAVEL			
XX		BOOLDERS			GRAVELLY ORGANIC SOIL with SAND			
				CVRAD				
		DRILLING MET	HUD	STIVIB	ULS			WATER LEVEL SYIVIBOLS
וכדן		P						
- KI	Auger D	Drilling Rotary Drilling	Dy Py	namic Co	ne or Diamond Core	▼ :	Static V	Vater Level Reading
ЦЦ			∟∠_ Ha	and Drive				
						L		
erm	Defi	inition Sur	nbol					
					1			
Unit	Cha	ange in geoilogic unit						
Chang	e							
					1			
Materi	ial Chạ	ange of soil classification within geologic	·					
Within U	,∽ uni Jnit	τ						
								GROUP DELT
FREN		altrans Soil and Rock Logging	laccif	ication	and Presentation Manual (201	0)		
LIVEL	UL. U	and and Jon and NOCK LUBBILIS, I	-102211	icaciól	, and i resentation Manual (201	<i></i>		

CONSISTENCY OF COHESIVE SOILS				
Description	Shear Strength (tsf)	Pocket Penetrometer , PP Measurement (tsf)	Torvane, TV, Measurement (tsf)	Vane Shear, VS, Measurement (tsf)
Very Soft	Less than 0.12	Less than 0.25	Less than 0.12	Less than 0.12
Soft	0.12 - 0.25	0.25 - 0.5	0.12 - 0.25	0.12 - 0.25
Medium Stiff	0.25 - 0.5	0.5 - 1	0.25 - 0.5	0.25 - 0.5
Stiff	0.5 - 1	1 - 2	0.5 - 1	0.5 - 1
Very Stiff	1 - 2	2 - 4	1 - 2	1 - 2
Hard	Greater than 2	Greater than 4	Greater than 2	Greater than 2

APPARENT DENSITY OF COHESIONLESS SOILS		
Description	SPT N ₆₀ (blows / 12 inches)	
Very Loose	0 - 4	
Loose	5 - 9	
Medium Dense	10 - 29	
Dense	30 -50	
Very Dense	Greater than 50	

PERCENT OR PROPORTION OF SOILS

Description	Criteria
Trace	Particles are present but estimated to be less than 5%
Few	5 - 10%
Little	15 - 25%
Some	30 - 45%
Mostly	50 - 100%

CEMENTATION			
Description	Criteria		
Weak	Crumbles or breaks with handling or little finger pressure		
Moderate	Crumbles or breaks with considerable finger pressure		
Strong	Will not crumble or break with finger pressure		

MOISTURE		
Description Criteria		
Dry	No discernable moisture	
Moist	Moisture present but no free water	
Wet	Visible free water	

CONSISTENCY OF COHESIVE SOILS*

Description	SPT N ₆₀ (blows / 12 inches)
Very Soft	0 - 1
Soft	2 - 3
Medium Stiff	4 - 7
Stiff	8 - 14
Very Stiff	15 - 30
Hard	Greater than 30

* Ref: Peck, Hansen, and Thornburn, 1974, "Foundation Engineering," Second Edition.

Note: Only to be used (with caution) when pocket penetrometer or other data on undrained shear strength are unavailable. Not allowed by Caltrans Soil and Rock Logging and Classification Manual, 2010.

PARTICLE SIZE		
Description		Size (in)
Boulder		Greater than 12
Cobble		3 - 12
Gravel	Coarse	3/4 - 3
	Fine	1/5 - 3/4
	Coarse	1/16 -1/5
Sand	Medium	1/64 - 1/16
	Fine	1/300 - 1/64
Silt and Clay		Less than 1/300

PLASTICITY		
Description	Criteria	
Nonplastic	A 1/8-in. thread cannot be rolled at any water content	
Low	The thread can barely be rolled and the lump cannot be formed when drier than the plastic limit	
Medium	The thread is easy to roll and not much time is required t reach the plastic limit. The thread cannot be rerolled afte reaching the plastic limit. The lump crumbles when drier the plastic limit	
High	It takes considerable time rolling and kneading to reach the plastic limit. The thread can be rerolled several times after reaching the plastic limit. The lump can be formed without crumbling when drier than the plastic limit.	

REFERENCE: Caltrans Soil and Rock Logging, Classification, and Presentation Manual (2010), with the exception of consistency of cohesive soils vs. N₆₀.



LEGEND OF ROCK MATERIALS	BEDDING SPACING	
	Description	Thickness / Spacing
Igneous Rock	Massive	Greater than 10ft
	Very Thickly Bedded	3 ft - 10 ft
Sedimentary Rock	Thickly Bedded	1 ft - 3 ft
	Moderately Bedded	4 in - 1 ft
	Thinly Bedded	1 in - 4 in
Metamorphic Rock	Very Thinly Bedded	1/4 in - 1 in
	Laminated	Less than 1/4 in
WEATHERING DESCRIPTORS FOR INTACT ROCK		

Description	Chemical Weathering - Discoloration - Oxidation		Mechanical Weathering	Texture and Leaching		
Description	Body of Rock	Fracture Surfaces	Conditions	Texture	Leaching	General Characteristics
Fresh	No discoloration, no oxidized	No discoloration or oxidation	No separation, intact (tight)	No change	No leaching	Hammer rings when crystalline rocks are struck
Slightly Weathered	Discoloration or oxidation is limited to surface of, or short distance from, fractures; some feldspar crystals are dull	Minor to complete discoloration or oxidation of most surfaces	No visible separation, intact (tight)	Preserved	Minor leaching of some soluble minerals	Hammer does not ring when rock is struck. Body of rock not weakened
Moderately Weathered	Discoloration or oxidation extends from fractures usually throughout; Fe-Mg minerals are "rusty"; feldspar crystals are "cloudy"	All fracture surfaces are discolored or oxidized	Partial separation of boundaries visible	Generally Preserved	Soluble minerals may be mostly leached	Hammer does not ring when rock is struck. Body of rock is slightly weakened
Intensely Weathered	Discoloration or oxidation throughout; all feldspars and Fe-Mg minerals are altered to clay to some extent; or chemical alteration produces in situ disaggregation, grain boundary conditions	All fracture surfaces are discolored or oxidized; surfaces friable	Partial separation, rock is friablr; in semi-arid conditions, granitics are disaggregated	Texture altered by chemical disintegration (hydration, argillation)	Leaching of soluble minerals may be complete	Dull sound when struck with hammer; usually can be broken with moderate to heavy manual pressure or by light hammer blow without reference to planes of weakness such as incipient or hairline fractures of veinlets. Rock is significantly weakened.
Decomposed	Discolored of oxidized throughout, but resistant minerals such as quartz may be unaltered; all feldspars and Fe-Mg minerals are completely altered to clay		Complete separation of grain boundaries (disaggregated)	Resembles a soil remnant rock sti preserved; leach minerals usually	; partial or complete ructure may be ing of soluble complete	Can be granulated by hand. Resistant minerals such as quartz may be present as "stringers" or "dikes"

PERCENT CORE RECOVERY (REC)
Σ LENGTH OF THE RECOVERED CORE PIECES (IN.) x 100 TOTAL LENGTH OF CORE RUN (IN.)
PERCENT CORE RECOVERY (REC)

 $\frac{\sum \text{ Length of the intact core pieces } \geq 4 \text{ in.}}{\text{ total length of core run (in.)}} \times 100$

ROCK HARDNESS				
Description	Criteria			
Extremely Hard	Cannot be scratched with a pocketknife or sharp pick. Can only be chipped with repeated heavy hammer blows,			
Very Hard	Cannot be scratched with a pocketknife or sharp pick. Breaks with repeated heavy hammer blows.			
Hard	Can be scratched with a pocketknife or sharp pick with difficulty (heavy pressure). Breaks with heavy hammer blows.			
Moderately Hard	Can be scratched with a pocketknife or sharp pick with light or moderate pressure. Breaks with moderate hammer blows.			
Moderately Soft	Can be grooved 1/16 in. deep with a pocketknife or sharp pick with moderate or heavy pressure. Breaks with light hammer blow or heavy manual pressure.			
Soft	Can be grooved or gouged easily with a pocketknife or sharp pick with light pressure, can be scratched with fingernail. Breaks with light moderate manual pressure.			
Very Soft	Can be readily indented, grooved or gouged with fingernail, or carved with a pocketknife. Breaks with light manual pressure.			

FRACTURE DENSITY		
Description	Observed Fracture Density	
Unfractured	No fractures	
Very Slightly Fractured	Core lengths greater than 3 ft	
Slightly Fractured	Core lengths mostly from 1 to 3 ft	
Moderately Fractured	Core lengths mostly 4 in. to 1 ft.	
Intensely Fractured	Core lengths mostly from 1 to 4 in.	
Very Intensely Fractured	Mostly chips and fragments.	

PR-2024-001701 (GPA, SPA, RZ, DR) Exhibit 13 - EIR Addendum and appedices

Group Delta Consultants 9245 Activity Road, Suite 103 San Diego, California 92126 www.GroupDelta.com

Project: Riverside Community Hospital

GROUP DELTA

Location: 4468 Brockton Avenue, Riverside, California 92501



CPeT-IT v.3.5.3.3 - CPTU data presentation & interpretation software - Report created on: 3/26/2024, 4:24:54 PM PR-2021e0 থা বেঠা (এরিমিik স্টির্মান রিক্রার্মির্টেরি বির্বাধির বির্বাধির স্রার্ধ রের্টার্মার্টের বির্বাধির স্রার্ধ রের্টার্মার্টার বির্বাধির স্বার্ধার্মার্মার্টার্মার্মার্টার্মার্টার্মার্টার

CPT-1 Total depth: 33.99 ft, Date: 3/22/2024 Surface Elevation: 800.00 ft



Group Delta Consultants 9245 Activity Road, Suite 103

San Diego, California 92126 www.GroupDelta.com

Project: Riverside Community Hospital

GROUP DELTA

Location: 4468 Brockton Avenue, Riverside, California 92501



CPT-2 Total depth: 37.54 ft, Date: 3/22/2024 Surface Elevation: 796.00 ft



Group Delta Consultants 9245 Activity Road, Suite 103 San Diego, California 92126 www.GroupDelta.com

Project: Riverside Community Hospital

Location: 4468 Brockton Avenue, Riverside, California 92501



Total depth: 20.67 ft, Date: 3/22/2024 Surface Elevation: 793.00 ft

GROUP DELTA



Group Delta Consultants 9245 Activity Road, Suite 103 San Diego, California 92126 www.GroupDelta.com

Project: Riverside Community Hospital

Location: 4468 Brockton Avenue, Riverside, California 92501



CPeT-IT v.3.5.3.3 - CPTU data presentation & interpretation software - Report created on: 3/26/2024, 4:30:47 PM ମନ୍ତା ସେଥିଲିକାରେ ସେଥିଲେ ସେଥି CPT-4 Total depth: 25.01 ft, Date: 3/22/2024 Surface Elevation: 794.00 ft

GROUP DELTA

РТ-4


GROUP DELTA GROUP DELTA GROUP DELTA San Diego, California 92126 www.GroupDelta.com

Project: Riverside Community Hospital

Location: 4468 Brockton Avenue, Riverside, California 92501



CPeT-IT v.3.5.3.3 - CPTU data presentation & interpretation software - Report created on: 3/26/2024, 4:31:20 PM የጭታሪወረብፈውርባ(ርዕሬክሬ) ይመስመራ የአመር የመስመራ የመስ

CPT-5 Total depth: 54.73 ft, Date: 3/22/2024 Surface Elevation: 839.00 ft



Group Delta Consultants Riverside Community Hospital Riverside, CA

CPT Shear Wave Measurements

					S-Wave	Interval
	Tip	Geophone	Travel	S-Wave	Velocity	S-Wave
	Depth	Depth	Distance	Arrival	from Surface	Velocity
Location	(ft)	(ft)	(ft)	(msec)	(ft/sec)	(ft/sec)
CPT-1	5.02	4.02	4.49	5.86	766	· · · ·
	10.04	9.04	9.26	13.64	679	613
	15.03	14.03	14.17	20.74	683	692
	20.01	19.01	19.11	27.28	701	756
	25.03	24.03	24.11	31.36	769	1225
	30.02	29.02	29.09	35.44	821	1220
	33.92	32.92	32.98	37.44	881	1946
CPT-2	5.02	4.02	4.49	4.24	1059	
	10.01	9.01	9.23	13.50	684	512
	15.03	14.03	14.17	19.80	716	785
	20.01	19.01	19.11	26.12	732	782
	25.03	24.03	24.11	32.00	754	850
	30.02	29.02	29.09	35.22	826	1545
	35.01	34.01	34.07	37.78	902	1945
CPT-5	5.02	4.02	4.49	2.46	1825	
	10.01	9.01	9.23	5.94	1554	1362
	15.06	14.06	14.20	8.96	1585	1646
	20.05	19.05	19.15	13.04	1469	1214
	25.03	24.03	24.11	16.26	1483	1540
	30.02	29.02	29.09	20.44	1423	1190
	35.01	34.01	34.07	24.28	1403	1297
	39.99	38.99	39.04	26.98	1447	1842
	45.01	44.01	44.06	31.46	1400	1119
	50.03	49.03	49.07	35.20	1394	1341
	54.69	53.69	53.73	38.74	1387	1315

Shear Wave Source Offset -

2 ft

S-Wave Velocity from Surface = Travel Distance/S-Wave Arrival Interval S-Wave Velocity = (Travel Dist2-Travel Dist1)/(Time2-Time1)









PR-2024-001701 (GPA, SPA, RZ, DR) Exhibit 13 - EIR Addendum and appedices

E	BOR		GF	RECC	ORD		PROJEC Rivers	<mark>ст nai</mark> ide C	NE ommu	unity Ho	ospital			PROJECT SD809	NUMBER		BORING B-02
SITE LO	OCATION	1	_		_					-	•	STAR	RT	FIN	SH		SHEET NO.
Prop	osed P	arking	g Gara	age and	Tower	Sites				FTUOD		4/1	/2024	4/	1/2024		3 of 3
	w lack	PANY ot Dri	llina						ING M IOW St	ETHOD tom Διι	der				Вт	MA	
DRILLI		PMEN	шıу Г					BORI		. (in)		DEPTH (ft)	GROUN	D ELEV (ft)	DEPTH	ELEV. G	ROUNDWATER (ft)
CME	85 Tru	ick Mo	ounte	d Rig #1	20			8			51.5	(,	798	(,	▼ 36.	0 / 762	2.0
SAMPL	ING MET	HOD		0			NOTES	6							-		
Ham	mer: 14	0 lbs	, Dro	p: 30 in.	(Autom	atic)	ETR	~ 80	%, N ₆	₀ ~ 80/	60 * N ~	1.33 * N					
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	°° N	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DES	CRIPTION /	AND CLAS	SIFICAT	ΓΙΟΝ
_		\square	S-11									OLD A dense; mostly	LLUVIU light yel fine to n	I <u>M</u> : WELL- llowish bro nedium SA	-GRADEI wn (10YF ND; few 1	D SANE R 6/4); s fines; n	D (SW); saturated; onplastic.
_	745									-		Total D Ground)epth: 51 dwater D	1½ Feet Depth: 36 F	eet		
- 55	_									- 55 —		Ground when t	dwater ir he botto	nitially obse m of the bo	erved at 4 prehole h	l6 feet eaved.	
										_							
	740									_							
-	740									_							
-										-							
60										60 —							
-										-							
-										-	-						
-	735									-	-						
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-65	_									65 —	-						
4/19/2										-	-						
GDT										_							
CLOG	720																
- 60	730									_							
- c es:c										-							
0]—70 60										70 —							
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Σ MX L	725									-							
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		DE	LT/		ISUL	.TAN	NTS,	INC	TH OF SU	IS SUMI THIS BO BSURFA	MARY API ORING AN ACE CON	PLIES ONLY ND AT THE DITIONS M	Y AT THE TIME OF AY DIFFE	E LOCATIO		F	IGURE
0	Sa	n Di	ego	, Calif	fornia	1 921	126		WI PR CO	TH THE ESENTE	PASSAGE ED IS A SI NS ENCO	E OF TIME. IMPLIFICAT UNTERED.	THE DA	TA THE ACTUA			A-7 c





PR-2024-001701 (GPA, SPA, RZ, DR) Exhibit 13 - EIR Addendum and appedices



PR-2024-001701 (GPA, SPA, RZ, DR) Exhibit 13 - EIR Addendum and appedices

E	BOR		GF	RECO	ORD)	PROJE Rivers	CT NAI	ME ommu	unity Ho	ospital				PROJE	ст и 09	UMBER		BORING B-04
SITE LO	OCATION	1										S	TAR	т	F	INIS	н		SHEET NO.
Prop	osed P	arking	g Gar	age and	Tower	Sites							4/4/	/2024		4/4	/2024		2 of 2
DRILLI		PANY	lling						LING M	ETHOD					LOGG	ED B	Ϋ́	CHE	
DRILLI	W Jack		ning T							(in)	UTOTAI	DEPTH	(ft)	GROUN) (ft)			
CME	75 Lin	nited A	Acces	s Ria #1	42			8		()	31		(11)	794	0	,	▼ / na	•. •	
SAMPL	ING MET	HOD		<u> </u>			NOTES	3										-	
Ham	mer: 14	0 lbs.	, Dro	p: 30 in.	(Auton	natic)	ETR	~ 74	%, N ₆	₀ ~ 74/6	50 * N ~	· 1.23 *	Ν						
feet)	NOI	гүрЕ	ON	ATION ANCE / 6 IN)	"N" T		RE	SITY	S	feet)	lic								
DEPTH	ELEVAT (feet	SAMPLE -	SAMPLE	PENETR RESIST (BLOWS	BLOW/F	z° Z	MOISTL (%)	DRY DEN (pcf)	OTHE TEST	DEPTH (GRAPH LOG			DES	CRIPTIC	N AN	ND CLAS	SIFICA	TION
_			R-6	40 50 (4")	100+	100+				-		OLI GR/ moi	D Al AVE ist; n	LUVIU L (SW) nostly fin	<u>M</u> : WE ; very de ne SAN aceous	LL-G ense D; lit	GRADED ; strong ttle GRA) SANI brown VEL; f	D WITH (7.5YR 5/8); ew fines;
-	 765									-									
30 _		\ge	S-7	16 50	76	93				30 — -									
-										-		Tota Gro	al De ound	epth: 31 water N	Feet lot Enco	unte	ered		
	760									- 35									
_										-									
-										_									
- 40	755 									- 40 —									
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า45 45										45									
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	745									_									
	OUP 924	DE 5 A	LT/ ctivi	A CON	ISUL ad, S	TAI Suite	NTS , 103	INC	TH OF SU LO	IS SUMN THIS BO BSURFA CATION	MARY AP DRING AI CE CON S AND M	PLIES O ND AT TI DITIONS AY CHAI	NLY HE T S MA	AT THE IME OF Y DIFFE	E LOCAT DRILLIN ER AT O S LOCA	ion Ng. Thei Fion	۲ ۱	F	FIGURE
	Sa	n Di	ego	, Calif	fornia	a 92 ⁻	126		WI PR CO	TH THE ESENTE NDITION	PASSAG D IS A S NS ENCO	e of tin Implific Nuntere	ME. CATI ED.	THE DA ON OF 1	TA THE AC	UAL			A-9 b



E	BOR		G F	RECO	DRD)	PROJEC Rivers	<mark>ст nai</mark> ide C	NE ommu	unity Ho	ospital			PROJECT	NUMBER	२	BORING B-05
SITE LO		۱	~		-	0.1						STAF	RT	FIN	ISH		SHEET NO.
Prop	DSed P	arking	g Gar	age and	lower	Sites				ETUOD		4/4	/2024		4/2024	CHE	
Yello	w Jack	et Dri	llina					Hol	low S	tem Au	aer			JWJ	ы		AF
DRILLI	IG EQUI	PMEN	T					BORI	NG DIA	. (in)		DEPTH (ft)	GROUN	ID ELEV (ft)	DEPTH	ELEV. G	ROUNDWATER (ft)
CME	75 Lin	nited A	Acces	s Rig #1	42			8			31.5		796		▼ / r	าล	
SAMPL	ING MET	HOD	-	00 ·	/A 1	<i>.</i>	NOTES	3	0/ 11	7.4.4	00 * NI	4 00 * N					
Hami	ner: 14	U IDS.	., Dro	p: 30 in.	(Autor	natic)	EIR	ζ~ /4	%, N ₆	₀ ~ 74/	50 ^ N ~	1.23 ^ N					
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	°°Z	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DES	CRIPTION /	AND CLA	SSIFICA	TION
-	770 		R-6	9 17 22	39	32	13.5	109		-		OLD A GRAVI yellowi mediur micace	LLUVIU EL (SW) sh brow n SAND cous.	<u>IM</u> : WELL ; dense; ve n (10YR 5/ ; some GR	-GRADE ery pale '4); mois AVEL; fi	D SANI brown (t; mostly ew fines	D WITH 10YR 8/2) to y fine to s; nonplastic;
30 -	765	\square	S-7	10 27 50	77	95				30 — -		Very pa (10YR	ale brow 6/4); vei	/n (10YR 8, ry dense.	/3) to ligl	ht yellov	vish brown
-										-		Total D Ground	0epth: 31 dwater N	1½ Feet Not Encoun	tered		
- 35										- 35 —							
-	760 									_							
-										-							
40										40 —							
06.GDT 4	755 									-							
										-							
										45 —							
	750									-							
										-							
	OUP 924	DE 5 A	LT/	A CON	ISUL ad, S	.TAN uite	NTS , 103	INC	TH OF SU LO	IS SUMI THIS BO BSURFA CATION	MARY API DRING AN ACE CON S AND M	PLIES ONL' ND AT THE DITIONS MAY CHANGI	Y AT THE TIME OF AY DIFFE	E LOCATIO DRILLING ER AT OTH	N ER DN	F	FIGURE
	Sa	n Di	ego	, Calif	ornia	a 92′	126		WI PR CO	TH THE ESENTE NDITIO	PASSAGI D IS A SI NS ENCO	E OF TIME. MPLIFICAT UNTERED.	THE DA	TA THE ACTUA			A-10 b



PR-2024-001701 (GPA, SPA, RZ, DR) Exhibit 13 - EIR Addendum and appedices

E	BOR		GF	RECO	DRD		PROJEC Rivers	CT NA	ME ommu	unitv He	ospital				PROJ SD	<mark>јест</mark>)809	NUMBEF	R	BORING B-06
SITE LO	CATION	1										ST	TAR	г		FINI	SH		SHEET NO.
Prop	osed P	arking	g Gara	age and	Tower	Sites						4	4/3/	2024		4/4	4/2024		2 of 2
DRILLI		PANY	llina						LING M	ETHOD	aor				LOG	GED	BY	CHE	
DRILLI	W Jack		ning r					BORI		(in)	UTOTAI	DEPTH	(ft) (GROUN		VJ V (ft)	DEPTH/		
CME	75 Lin	nited A	Acces	s Ria #1	42			8		()	31.5			794			▼ / r	na	
SAMPL	ING MET	THOD		<u> </u>			NOTES	3						-				-	
Ham	mer: 14	0 lbs.	, Dro	p: 30 in.	(Autom	natic)	ETR	~ 74	%, N ₆	₀ ~ 74/	60 * N ~	1.23 *	Ν						
H (feet)	ATION eet)	е түре	LE NO.	RATION STANCE S / 6 IN)	//FT "N"	2 0	TURE 6)	ENSITY of)	HER STS	H (feet)	PHIC 0G			DES	CDIDT				
DEPT	(fé	SAMPL	SAMP	PENET RESIS (BLOW	BLOW	2	SIOM	DRY DE (po	UTH TE	DEPTI	GRA			DES	UNF 1			551107	(IION
- - - 	 765		S-7 B-8	7 25 25	50	62				- - - 30 —		OLD very fine nonp	D AL der to m plast	LUVIU nse; yel nedium tic.	I <mark>M</mark> : SI Iowish SANE	ILTY n brov D; sor	SAND V vn (10Y ne fines	VITH G R 5/6); ; little (RAVEL (SM); moist; mostly GRAVEL;
_										-									
-										-		Tota Grou	al De undv	epth: 31 water N	1½ Fee lot Ene	et count	ered		
- 35	760									- 35 —									
-										-									
-										-									
	755 									- 40 —									
1 4/19/24										_									
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- 65	750									-									
2907 – 45	_									45 —									
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	745									_									
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و	Sa	n Di	ego	o, Calif	fornia	a 92'	126		WI PR CO	TH THE ESENTE	PASSAG ED IS A S NS ENCC	E OF TIM IMPLIFIC UNTERE	ATIC	THE DA	TA THE A	CTUA	L		A-11 b



E	BOR		GF	RECO	ORD		PROJEC Rivers	<mark>ст nai</mark> ide C	ME ommi	unity Ho	ospital			PROJECT SD809	NUMBEF	R E	BORING B-07
SITE LO	OCATION	I	_		_					-	•	STAR	RT	FINI	SH	5	SHEET NO.
Prop	osed P	arking	g Gara	age and	Tower	Sites				FTUOD		4/4	/2024	4/	4/2024		2 of 2
Vello	w Jack	PANI ≏t Dri	llina						ING M	tem Διι	aer				ВТ	MAF	
DRILLI	NG EQUI	PMEN	Г					BORI		. (in)		. DEPTH (ft)	GROUN	ID ELEV (ft)	DEPTH/	ELEV. GRO	OUNDWATER (ft)
CME	75 Lin	nited A	Acces	s Rig #1	42			8			28		794		⊈ / r	าล	
SAMPL	ING MET	HOD	_				NOTES	3									
Ham	mer: 14	0 lbs.	, Dro	p: 30 in.	(Autom	natic)	EIR	~ 74	%, N ₆	₀ ~ 74/6	30 * N ∼	· 1.23 * N					
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	09 29	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DES	CRIPTION A	AND CLA	SSIFICATIO	ON
-		X	S-6	21 29 20	49	60						<u>OLD A</u> dense; (10YR GRAVI	LLUVIU very pa 6/8); mc EL; trace	M: WELL- le brown (1 bist; mostly e fines; non	GRADEI 0YR 8/3 fine to c plastic;	D SAND (3) to brown coarse SA micaceou	SW); very hish yellow ND; few s.
- - 30	765									- 30		Total D Ground	0epth: 28 dwater N	3 Feet lot Encoun	tered		
_										_							
-										_							
- 35	760 									- 35 —							
-										-							
-	 755									-							
40 40										40							
										_							
GS.GPJ GL	750									_							
-45 45										45							
X SOIL SL										-							
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	OUP 924	DE 5 A	LT# ctivi	A CON ty Roa	ISUL ad, S	. TAN uite	NTS , 103	INC	TH OF SU LO	IS SUMA THIS BO BSURFA CATION	MARY AP DRING AI ACE CON S AND M	PLIES ONLY ND AT THE DITIONS M AY CHANGI	Y AT THE TIME OF AY DIFFE E AT THI	E LOCATION DRILLING. ER AT OTHI S LOCATIO	N ER VN	FI	GURE
	Sa	n Di	ego	, Calif	fornia	921	126		PR	ESENTE	ED IS A S	E OF TIME. IMPLIFICAT UNTERED.	ION OF	THE ACTUA		Ą	A-1∠ D



E	BOR		GF	RECO	DRD)	PROJE Rivers	CT NA	ME ommi	unitv He	ospital			PROJEC SD80	<mark>t numbe</mark> 9	R	BORING B-08
SITE LO	OCATION	1										STA	RT	Fil	NISH		SHEET NO.
Prop	osed P	arking	g Gar	age and	Tower	Sites						4/	1/2024	4	/1/2024		2 of 2
DRILLI								DRILL	ING M	ETHOD				LOGGEI	D BY	CHE	
Yello	W Jack		iling r					HOI		tem Au	Iger		GROUN				
CMF	75 l in	nited A	Acces	s Ria #1	42			8		. (11)	30		794		.,	na	
SAMPL	ING MET	HOD					NOTES	3			00				<u>+</u> /·		
Ham	mer: 14	0 lbs.	, Dro	p: 30 in.	(Auton	natic)	ETR	R ~ 80	%, N ₆	₀ ~ 80/	60 * N ~	· 1.33 * N					
				7													
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN	BLOW/FT "N"	Z ⁰⁰	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DES	SCRIPTION	AND CLA	.SSIFICA	TION
			R-6	24	74	66	3.2	112									
-	-		11-0	50	14		0.2	112		-		SAND	(GW); v	<u>JM</u> : WELL rery dense	-GRADE ; white (1	D GRA	VEL WITH 8) to very pale
-										-		brown coarse	(10YR / SAND:	(/4); moist trace fine	; mostly (s: nonpla	JRAVEI stic.	L; some fine to
													,		<i>,</i> ,		
F										-	p/g/						
-	765	\bigvee	6.7	22	0.0	1001				-	PRK						
30			5-1	(5")	02	100+				30]					
			1							30		T . 4 . 1 1					
-	-									-	-	Groun	Jeptn: 30 dwater N	o Feet Not Encou	ntered		
										_		_					
Γ																	
+	-									-	-						
	760									_							
35	-									35 —	-						
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7 T										-	1						
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			с I / 				1 00	INC	SU	BSURF	ACE CON	DITIONS N	IAY DIFF	ER AT OTH	HER	I	IGUIL
Ū	924	S A	CUVI		ad, S	ulte	103		LO	CATION	S AND M PASSAG	AY CHANG E OF TIMF	E AT TH	IS LOCATI ATA	ON		A-13 h
	Sa	n Di	ego	, Cali	fornia	a 92′	126		PR CO	ESENTE	ED IS A S		TION OF	THE ACTU	JAL		



E	BOR		G F	RECC	DRD)	PROJEC Rivers	CT NA	ME ommi	unitv H	ospital				PRO	D809	NUMBE	R	BORING B-09
SITE LO	OCATION	1										ST	TAR	г	-	FIN	ISH		SHEET NO.
Prop	osed P	arking	g Gar	age and	Tower	Sites						4	4/5/	2024		4	5/2024	4	2 of 2
DRILLI		PANY	lling						LING M	ETHOD							BY	CI	
			ning r							(in)	TOTAL	DEPTH ((ft) (GROUN		FV (ft)	DEPT		
CME	75 Lin	nited A	Acces	s Ria #1	42			8		()	31.5			832			v /	na	
SAMPL	ING MET	HOD		<u> </u>			NOTES	3			1 - 1						<u> </u>		
Ham	mer: 14	0 lbs.	, Dro	p: 30 in.	(Auton	natic)	ETR	. ~ 74	%, N ₆	₀ ~ 74/	60 * N ~	1.23 *	N						
PTH (feet)	EVATION (feet)	PLE TYPE	MPLE NO.	IETRATION SISTANCE DWS / 6 IN)	DW/FT "N"	N ⁰⁰	JISTURE (%)	DENSITY (pcf)	DTHER TESTS	PTH (feet)	RAPHIC LOG			DES	CRIF	TION	AND CL	ASSIFIC	CATION
DE	Ш	SAM	SA	(BLE	BL		Ŵ	DRY		DE	0								
-	 805		R-6	8 14 22	36	30	2.5	114		-		OLD SILT mois fines	<mark>) AL</mark> [(Sl st; m s; nc	D-SM); ostly fi onplasti	I <u>M</u> : den ne to ic.	POOF se; bro o med	LY-GR ownish um SA	ADED yellow ND; litt	SAND WITH (10YR 6/8); le GRAVEL; few
- - 30	 		0.7	7						- - 30 —		Grac	des i	from ve	ery p	ale br	own (10)YR 7/4	4) to yellowish
	 800	$\mid \land \mid$	S-7	12 16	28	34				-			vii (Jus, iia		
-										-	-	Grou	undv	epth: 31 water N	l 1/2 F lot E	eet ncour	tered		
35										- 35									
-	-									-	-								
-	795									-	-								
-										-	-								
40										40 —									
06.GDT	790									-	-								
										-									
										45 —	-								
										-	-								
										-	-								
										-	-								
	OUP	DE	LT/	۱ CON ity Ro:	ISUL ad S	.TAN	NTS, 103	INC	TH OF SU	IS SUMI THIS B BSURF CATION	MARY AP ORING AI ACE CON IS AND M	PLIES ON ND AT TH DITIONS AY CHAN	NLY IE T MA` NGF	AT THE IME OF Y DIFFE AT THI	E LO DRI ER A S L C	CATIO LLING T OTH CATIC	N ER N		FIGURE
	Sa	n Di	ego	o, Calif	fornia	a 92′	126		WI PR CO	TH THE ESENTE	PASSAG ED IS A S NS ENCC	E OF TIM IMPLIFIC UNTERE	IE. T ATIC	THE DA	TA	ACTU	AL.		A-14 b

F			G F	RECC			PROJE		ME						PROJE		MBER		BORING
SITELO		<u>, 11 M A</u>					Rivers	ide C	ommı	unity Ho	spital			т	SD8	U9 INISH			B-1U SHEET NO
Prop	osed P	• arkino	d Gar	ade and	Tower	Sites							4/2	/2024	'	4/2/2	024		1 of 3
DRILLI	NG COM	PANY	9 0 0					DRILL	ING M	ETHOD					LOGGE	D BY		CHEC	CKED BY
Yello	w Jack	et Dri	illing					Hol	low S	tem Au	ger				JWJ			MA	ν F
DRILLI	NG EQUI	PMEN	T					BORI	NG DIA	. (in)		L DEPTH	l (ft)	GROUN	DELEV	ft) DE	PTH/EL	EV. G	ROUNDWATER (ft)
CME	85 Tru	ick M	ounte	d Rig #1	20			8			51.	5		838		Ţ	l / na		
SAMPL	ING MET	THOD					NOTES	6											
Ham	mer: 14	0 lbs	., Dro	p: 30 in.	(Auton	natic)	ETR	<u>~ 80</u>	<u>%</u> , Ν _ε	_{io} ~ 80/6	60 * N	<u>~ 1.33 '</u>	* N						
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eet)	R	Υ E	Ŏ		z		Ш	È	~	eet)	O								
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-	-	×××									\sim	ove	er 6-i	nches c	of Aggree	gate B	ase.		
		\otimes										EU	1.0). mo	diuma	longo.	vollowich
		\otimes										bro	wn (10YR 5	/6); mois	t; mos	stly fine	ense, es: sor	ne fine to
-	835	\boxtimes	B-1						PA	-		me	diun	SAND	; lów pla	sticity	micad	eous.	
									CR			(0%	6 Gr	avel: 33	% Sand	67%	Fines)		
Γ												(07	0 01	avei, 55	/o Gana	01 /0	i incoj		
5										5									
			S-2	9	18	24													
-		$\angle $		9															
-										_									
F	830									-									
-										_		. <u> </u>							
												SIL	TY	SAND (S	SM); der	ise; ve	ellowisl	h brow	n (10YR 5/6);
10				8						10		mo	ist; r	nostly Ìi	ne to me	dium	SAND	; some	e fines; low
			R-3	14	34	30	5.8	99	DS			pla	sticit	y; mica	ceous.				
				20															
-																			
	825																		
-										-		÷							
15										15		SA	NDY	SILT (I	ML); me	dium c	lense t	o dens	se; brownish
9/24		\backslash		8		07						yel	low (diun	10YR 6 SAND	/8); moi: · low pla	st; mo: sticitv	stly fine	es; little eous	e fine to
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Ĭ	527				aa, U forni-		100		W	TH THE	PASSA	GE OF TI	ME.	THE DA	TA				A-15 a
	29	וט וו	eyo	, Call		a 92	120		CC	NDITION	IS A S	OUNTER	ED.	ON OF	I HE ACT	UAL			

E	BOR		GF	RECC	DRD)	PROJE Rivers	CT NAI	νε ommι	unity H	ospit	al			PROJECT	numbe i 9	R	BORING B-10
SITE LC	CATION	I	_										STAF	RT	FIN	IISH		SHEET NO.
Prop	osed Pa	arking	g Gara	age and	Tower	Sites							4/2	/2024	4	/2/2024		2 of 3
DRILLI			11:1-1-1					DRILL	ING M) BY	CHE	
Yello	W Jack		ning r					HOI		(in)	iger			CROUN				
	85 Tru		ı Sunta	d Ria #1	20			8		. (11)	5	1 AL DE	. F I П (I I)	838			ne ne	IROUNDWATER (II)
SAMPL		HOD	Junic		20		NOTES	3				1.5		000		<u> </u>	na –	
Ham	mer: 14	0 lbs.	. Dro	p: 30 in.	(Auton	natic)	ETR	- . ~ 80	%. Na	~ 80/	60 *	N ~ 1.3	33 * N					
		-	,			, 			, 0	0								
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ⁰⁰	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC	POG		DES	CRIPTION	AND CLA	SSIFICA	TION
-		X	S-6	5 9 12	21	28				-	-		<u>FILL:</u> yellowis fine SA	SANDY sh browi ND; nor	SILT (ML n (10YR 4 nplastic.); mediui /4); mois	m dense st; mostly	e; light y fines; little
- - 30 -	810 		R-7	7 30 50	80	71	7.5	113		- - 30 — -			OLD A very de SAND; micace	LLUVIU ense; stra some fil ous.	<u>M</u> : SILTY ong browr nes; little (γ SAND \ n (7.5YR GRAVEL	WITH G 5/8); mo ; nonpla	RAVEL (SM); bist; mostly fine astic;
- 35 -	 800	\mathbf{X}	S-8	8 17 18	35	47				- 35 — -			SILTY 5/8); m GRAVE	SAND (S oist; mo EL; nonp	SM); very stly fine S lastic; mic	dense; s AND; so caceous.	trong br	own (7.5YR s; few
- 40 + 40	 		R-9	15 30 42	72	64	5.6	115		- 40 — -			SILTY brown fines; li	SAND W (7.5YR 5 ttle GRA	VITH GRA 5/8); moist WEL; non	VEL (SN ;; mostly plastic; r	Л); very fine SAI nicaceo	dense; strong ND; some us.
- 				10						- 45 —			SAND (10YR nonpla	/ SILT (I 4/4); mo stic; milc	ML); medii ist; mostly ily cement	um dens / fines; lit ted.	e; light y ttle fine	yellowish brown SAND;
	 790 		S-10	10 27 33	60	80				-			SILTY 7/4) to mediun micace	SAND (S brownisi n SAND ous.	SM); very h yellow (′ ; some fin	dense; v 10YR 6/6 es; few 0	ery pale ŝ), moist GRAVEL	e brown (10YR ;; mostly fine to .; nonplastic;
	OUP 924 Sai	DE 5 Ao n Di	LT <i>I</i> ctivi ego	A CON ty Roa o, Calif	ISUL ad, S fornia	TAN Suite a 92	NTS , 103 126	INC	TH OF SU LO WI PR CO	IS SUMI THIS B BSURF CATION TH THE ESENTE	MARY ORING ACE C IS ANI PASS ED IS NS EN	APPLII G AND A CONDIT D MAY AGE O A SIMP ICOUN	ES ONLY AT THE IONS MA CHANGE F TIME. LIFICAT TERED.	(AT THE TIME OF AY DIFFE E AT THI THE DA ION OF 1	E LOCATIO DRILLING ER AT OTH S LOCATIO TA THE ACTU	N j. IER DN AL	F	FIGURE A-15 b

E	BOR	RING	GF	RECC	DRD		PROJEC Rivers	<mark>ст NA</mark> ide C	ME ommi	unity Ho	ospital			PROJECT SD809	NUMBER	2	BORING B-10
SITE LO	OCATION	۱ 	_		_	I						STAR	RT	FINI	SH		SHEET NO.
Prop	osed P	arking	g Gara	age and	Tower	Sites				FTUOD		4/2	2/2024	4/	2/2024		3 of 3
Vello	w Jack	PANY PANY	llina						ING M	tem Διι	aer				Вт		
DRILLI	NG EQUI	PMENT	Г					BORI		. (in)		DEPTH (ft)	GROUN	D ELEV (ft)	DEPTH	ELEV. G	ROUNDWATER (ft)
CME	85 Tru	ick Mo	ounte	d Rig #1	20			8		()	51.5	. ,	838	()	l ▼ / n	na	
SAMPL	ING MET	HOD					NOTES	3							1		
Ham	mer: 14	0 lbs.	, Dro	p: 30 in.	(Autom	atic)	ETR	~ 80	%, N ₆	₀ ~ 80/6	50 * N ~	1.33 * N					
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	°° Z	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG		DES	CRIPTION A	AND CLAS	SSIFICA	TION
- - - 	785 785 		R-11	17 27 50	87	77	2.0			 55		OLD A SILT (to light mediur Total D Ground	LLUVIU SP-SM); yellowis n SAND Depth: 51 dwater N	M: POOR very dense h brown (1 ; few fines; 1½ Feet lot Encoun	LY-GRA e; very pa 0YR 6/4 few GR	DED S/ ale brov); moist AVEL; r	AND WITH vn (10YR 8/3) ; mostly fine to nonplastic.
- - - 60	780 780									- - - 60							
- - - - - - - - - - - - - - - - - - -	 775 									- - - 65							
	 770									-							
	 765 									70							
GR	OUP 924 Sa	DE 5 Ao n Di	LT <i>I</i> ctivi ego	A CON ty Roa o, Calif	ISUL ad, S fornia	TAN uite 921	NTS , 103 126	INC	TH OF SU LO WI PR CO	IS SUMA THIS BO BSURFA CATION TH THE ESENTE NDITION	ARY AP DRING AN CE CON S AND M PASSAGI D IS A SI NS ENCO	PLIES ONL' ND AT THE DITIONS M/ AY CHANGI E OF TIME. MPLIFICAT UNTERED.	Y AT THE TIME OF AY DIFFE E AT THI THE DA ION OF	E LOCATION DRILLING. ER AT OTHI S LOCATIO TA THE ACTUA		F	FIGURE A-15 c

APPENDIX A1 PREVIOUS EXPLORATIONS (2007 to 2013)

Date Drilled: 12/20/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Surface Elevation(ft): 799.0

Driving Weight / Drop: 140 lbs./ 30 in. Logged by: VJR Measured

Measured Depth to Water(ft): 39.0

DRY UNIT WT. (pcf) FIELD MOISTURE (%) SAMPLES LAB/FIELD TESTS BLOWS/6 IN REMARKS GRAPHIC LOG DEPTH (ft) VISUAL CLASSIFICATION DRIVE BULK 14.8 Fill (SM) Silty Sand, fine to coarse with gravel to 1", brown 8 14 19 13.4 120 Ring SA Native (ML) Sandy Silt, fine with medium, light brown 5 345 Ring, Consol. 6.7 101 10 479 11.5 120 Ring, DS 2.3 (SP-SM) Sand, medium to coarse with fine and silt, brown 15 12 18 2.3 114 Ring (SW-SM) Sand with silt, fine to coarse with clay and 5.3 SA gravel to 3", brown 20 38 50/5" SORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ.GDT 1/18/08 3.6 117 Ring 25 40 50/5" 4.1 105 Ring, DS 30 \propto 50/4" 3.5 Ring 104 (SP) Sand, medium to coarse with fine, gravel and cobbles to 4", brown Enclosure PROPOSED HOSPITAL EXPANSION AND PARKING GARAGEOb No. B-la 07881-3 RIVERSIDE, CALIFORNIA

Logged by: VJR

Date Drilled: 12/20/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Surface Elevation(ft): 799.0

Driving Weight / Drop: 140 lbs./ 30 in.

Measured Depth to Water(ft): 39.0

				SAM	PLES		(%)	NT.	
DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	DRIVE	BULK	BLOWS/6 IN	FIELD MOISTURE	DRY UNIT V (pef)	LAB/FIELD TESTS
		(SP) Sand, medium to coarse with fine, gravel and		\times		50/4"	6.0	111	Ring
		cobbles to 4", brown	Groundwate	×		50/4"	17.2	Dist.	Ring
- 45 -				X		21 50/5"	10.5	123	Ring, DS
- 50 - - 50 -				X		50	11.8	128	Ring
- 55 -				×		50	12.4	125	Ring
- 60 -				X		50/4"	15.4	123	Ring
- 65 -		NO BEDROCK REFUSAL AT 70.0' FILL TO 4.0' SLIGHT CAVING GROUNDWATER AT 39.0'		X		50/4"	9.6	134	Ring
		END OF BORING		-		50/1"	N.R.	N.R.	Ring
		PROPOSED HOSPITAL EXPANSION AND RIVERSIDE, CALIFORN	PARKIN IA	G G	AR	AGEO 07	b No. 881-3	Enc B	losure -1b

Date Drilled: 12/20/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 800.0

Logged by: VJR

Measured Depth to Water(ft): 36.0

DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	DRIVE	BULK	BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
		(ML) Sandy Silt, fine to medium with coarse and gravel to 1", light brown	Fill		*****		7.3		SA, MDE, SE
[.				A		5 6 4	2.0		SPT
- 5 -		(ML) Sandy Silt, fine with medium, light brown	Native		*****		7.9		
	-			X		3 2 2			SPT
- 10 -				X		3 3 5			SPT
- 15 -		(SP-SM) Sand, medium to coarse with fine, silt and gravel to 1", brown			****		3.3		
	-			X		5 6 6			SPT
_ 20 -		(SP) Sand, medium to coarse with fine and gravel to 1", brown							
	-			X		19 24 19			SPT
- 25 -									
				X		41 50/5"			SPT
- 30 -		(SP-SM) Sand, fine to coarse with silt and gravel to 1", light brown	-	X		50			SPT
		PROPOSED HOSPITAL EXPANSION AND RIVERSIDE, CALIFORN	PARKIN IA	G G	AR	AGE0 07	b No. 881-3	Enc B	losure -2a

Logged by: VJR

Date Drilled: 12/20/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Surface Elevation(ft): 800.0

Driving Weight / Drop: 140 lbs./ 30 in.

Measured Depth to Water(ft): 36.0

SAMPLES FIELD MOISTURE (%) DRY UNIT WT LAB/FIELD TESTS BLOWS/6 IN REMARKS GRAPHIC LOG DEPTH (ft) VISUAL CLASSIFICATION DRIVE BULK (pcf) (SP-SM) Sand, fine to coarse with silt and gravel to 1", **▼** Groundwater light brown 19 23 32 SPT 40 50/5" SPT END OF BORING NO BEDROCK 45 REFUSAL AT 43.0' FILL TO 4.0' SLIGHT CAVING **GROUNDWATER AT 36.0'** 50 55 1/18/08 BORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ.GDT 60 65 PROPOSED HOSPITAL EXPANSION AND PARKING GARAGEOb No. Enclosure C.H.J. 07881-3 B-2b RIVERSIDE, CALIFORNIA

Logged by: JMZ

Date Drilled: 12/26/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Measured Depth to Water(ft): 76.7



Logged by: JMZ

Date Drilled: 12/26/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Surface Elevation(ft): 839.0

Driving Weight / Drop: 140 lbs./ 30 in.

Measured Depth to Water(ft): 76.7

				SAMPLES	N.	E (%)	TW	D
DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	DRIVE BULK	BLOWS/6	FIELD MOISTUR	DRY UNIT (pcf)	LAB/FIEL TESTS
		(SC) Clayey Sand, fine to medium with silt, red brown				11.7		Cor.
				X	10 18 35	10.8	125	Ring
40 -		(SM) Silty Sand, fine to medium with coarse and gravel to 3/4", light brown				10.0		
	-			X	17 30 50	10.3	126	Ring
45 -		(SP-SM) Sand, fine to coarse with silt and gravel to 1",				1.8		
14 14 13	-	light brown		X	48 43 41	1.8	Dist.	Ring
50 -		(SP) Sand, fine to medium with coarse and gravel to 1", light brown						
53 17 12				\boxtimes	40 50/5"	N.R.	N.R.	Ring
55 -		(SP) Sand, fine to coarse with gravel and cobbles to 4", light brown		X	50	N.R.	N.R.	Ring
· 60 -	-							
	-			M	50/4"	3.0	112	Ring
65 -								
				×	50	3.3	96	Ring
\diamond		PROPOSED HOSPITAL EXPANSION AND RIVERSIDE, CALIFORN	PARKIN IA	IG GAR	AG분이 07	b No. 881-3	Enc B	losure -6b

Date Drilled: 12/26/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Surface Elevation(ft): 839.0

Driving Weight / Drop: 140 lbs./ 30 in.

Logged by: JMZ

Measured Depth to Water(ft): 76.7

Image: Problem of the second secon	REMARKS	M M BULK	50/4"	FIELD MOISTURE	G DRY UNIT V (pcf)	LAB/FIELD Build
(SP) Sand, fine to coarse with gravel and cobbles to 4", light brown 75 (SM) Silty Sand, fine to coarse with clay and gravel 3", brown 80 -	Jundwater	M	50/4"	4.2	105	Ring
75 (SM) Silty Sand, fine to coarse with clay and gravel 3", brown 80	Jundwater	M	50/5"			
				9.6	114	Ring
END OF BORING	W	M	50/2"	N.R.	N.R.	Ring
- 85 - NO BEDROCK REFUSAL AT 83.0' FILL TO 5.0' SLIGHT CAVING GROUNDWATER AT 76.7'						
- 95 -						
C.H.J. PROPOSED HOSPITAL EXPANSION AND PAF	RKING	GAR	AGEO	b No.	Enc	losure

Logged by: JMZ

Date Drilled: 12/26/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Surface Elevation(ft): 825.0

Driving Weight / Drop: 140 lbs./ 30 in.

Measured Depth to Water(ft): N.A.

				SAMPLES		(%)	VT.	
DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	DRIVE BULK	BLOWS/6 IN	FIELD MOISTURE	DRY UNIT V (pcf)	LAB/FIELD TESTS
		Asphalt Base (SM) Silty Sand, fine to medium with coarse and gravel to 3", brown	Fill			6.5		
		(SM) Silty Sand, fine to coarse with gravel to 1", brown	Native			5.8		Cor.
- 3 -				X	6 8 8			SPT
- 10 -				X	3 3 3			SPT
- 15 -				X	2 4 3			SPT
- 20 -		(SP-SM) Sand, fine to medium with coarse and silt, light brown		\boxtimes	2 3 5			SPT
- 25 -		(SP-SM) Sand, fine to coarse with silt and gravel to 1", light brown	-	\boxtimes	6 8 11			SPT
- 30 -		(SP) Sand, fine to coarse, light brown	-	\boxtimes	8 12 19			SPT
		(SP) Sand, fine to coarse with gravel to 1", light brown						
		PROPOSED HOSPITAL EXPANSION AND RIVERSIDE, CALIFORN	PARKIN IA	G GAR	AGE0 07	b No. 881-3	Enc B	losure -7a

Logged by: JMZ

Date Drilled: 12/26/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Surface Elevation(ft): 825.0

Driving Weight / Drop: 140 lbs./ 30 in.

Measured Depth to Water(ft): N.A.

				SAM	PLES		(%)	VT.	
DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	DRIVE	BULK	BLOWS/6 IN	FIELD MOISTURE	DRY UNIT V (pcf)	LAB/FIELD TESTS
-	-	(SP) Sand, fine to coarse with gravel to 1", light brown		\boxtimes		13 26 21			SPT
	-					21			
-]								
- 40 -				\square		10 15			SPT
-	-					17			
	_								
- 45 -	_					14			ODT
						50			SFI
	-								
- 50 -						18			
		END OF BORING		Д		27 29			SPT
	-	NO PEDROCK							
	-	NO REFUSAL							
	_	SLIGHT CAVING							
		NO FREE GROUND WATER							
	-								
- 60 -	-								
	-								
	-								
- 65 -									
-	-								
	-								
			DADED			ACTO	h No	Enc	losure
$\langle \diamondsuit \rangle$		RIVERSIDE, CALIFORN	parkin IA	G G.	AR.	AGE0 07	881-3	B	-7b
EXPLORATORY BORING NO. 8

Date Drilled: 12/21/07

Client: Riverside Community Hospital

Equipment: Limited Access Mole Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Measured Depth to Water(ft): N.A.

Surface Elevation(ft): 810.0

Logged by: TAD

Γ					SAMPLES		(%)	WT.	
	DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION		DRIVE BULK	BLOWS/6 II	FIELD MOISTURE	DRY UNIT (pcf)	LAB/FIELD TESTS
-			(SM) Silty Sand, fine with clay, dark brown	Fill			14.2		
-					X	22 28 30	13.4	122	Ring
	5 -								
			(SM) Silty sand, fine with clay, dark brown	Native	X	7 8 8	4.8	99	Ring
-	10 -								
-						3 3 4	6.0	138	Ring
-	15 -								
		-	(ML) Sandy Silt, fine with clay, dark brown		X	5 6 7	N.R.	N.R.	Ring
8/08	20 -								
U CHUGDT 1/1		-			X	7 8 8	20.8	104	Ring
7881-3.G	25 -		(SP) Sand, fine to coarse with gravel to 3", light brown	-					
OW PER 6 IN 0		-			X	10 15 30	1.6	114	Ring
O EQUIV & BL	- 30 -	-	END OF BORING	_	\boxtimes	40 50	4.6	113	Ring
BORING LOG - N		-	NO BEDROCK, REFUSAL AT 31.0' FILL TO 7.0', SLIGHT CAVING NO FREE GROUNDWATER						
•	C.H.J. PROPOSED HOSPITAL EXPANSION AND PARKING GARAGEOb No. Encl RIVERSIDE, CALIFORNIA 07881-3 B						losure 8-8		

EXPLORATORY BORING NO. 9

Logged by: TAD

Date Drilled: 12/21/07

Surface Elevation(ft): 811.0

Client: Riverside Community Hospital

Equipment: Limited Access Mole Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Measured Depth to Water(ft): N.A.

Γ					SAMPLES			(%)	NT.	
	DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	DRIVE	BULK	BLOWS/6 IN	FIELD MOISTURE	DRY UNIT V (pcf)	LAB/FIELD TESTS
F	-		(SM) Silty Sand, fine, medium brown	Fill		****		8.5		
-	-			-						
-	5 -		(SM) Silty Sand, fine with clay, dark brown		X		14 14 15	8.7	127	Ring, Pass #200
-										
	10 -	-	(ML) Sandy Silt, fine with clay, light brown	Native		***	4	10.7		
-	-	-			A		4 5	4.9	98	Ring
-	-									
	15 -				X		2 3 5	7.6	101	Ring, DS
-	-									
18/08	20 -				X		7 10	8.2	115	Ring
HJ.GDI II	-						11			
1-3.6PJ C	25 -						8			
1 1 1 1	_		(SP) Sand, fine with medium to coarse, light brown	-	A		10 25	1.9	113	Ring
PLOW PER	-				X		9 11 29	5.3	107	Ring
MUN &	30 -]	END OF BORING							
- NO	-	-	NO BEDROCK REFUSAL AT 29.5'							5
AING LO	-	-	FILL TO 9.0' SLIGHT CAVING							
Image:						Enc	losure			
C.H.J. RIVERSIDE, CALIFORNIA 07881-3						E	8-9			
						-				



VELOCITY CORRELATIONS WITH STRATIGRAPHY

DEPTH RANGE (feet)	COMPRESSIONAL VELOCITY (ft/sec)	SHEAR VELOCITY (ft/sec)	STRATIGRAPHY
0 - 12	1020 +/- 7%	280 +/-15% 460 +/-10%	SILTY SAND (SM) WITH GRAVEL FILL
12 - 31	1600 +/-12%	700 +/-13%	SILTY SAND (SM)
31 - 42	3500 +/-10%	1940 +/-12%	SAND (SP) WITH GRAVEL GROUND WATER @ 39 feet
42 - 66	5750 +/-10%	2560 +/-10%	SAND (SP) WITH GRAVEL AND COBBLES

FIGURE 2 BORING B1 DOWNHOLE SEISMIC VELOCITY DATA AND INTERPRETED VELOCITY MODEL WITH STRATIGRAPHIC CORRELATIONS

PROPOSED BED TOWER & PARKING GARAGE RIVERSIDE COMMUNITY HOSPITAL TERRA PHYSICS



PR-2024-001701 (GPA, SPA, RZ, DR) Exhibit 13 - EIR Addendum and appedices