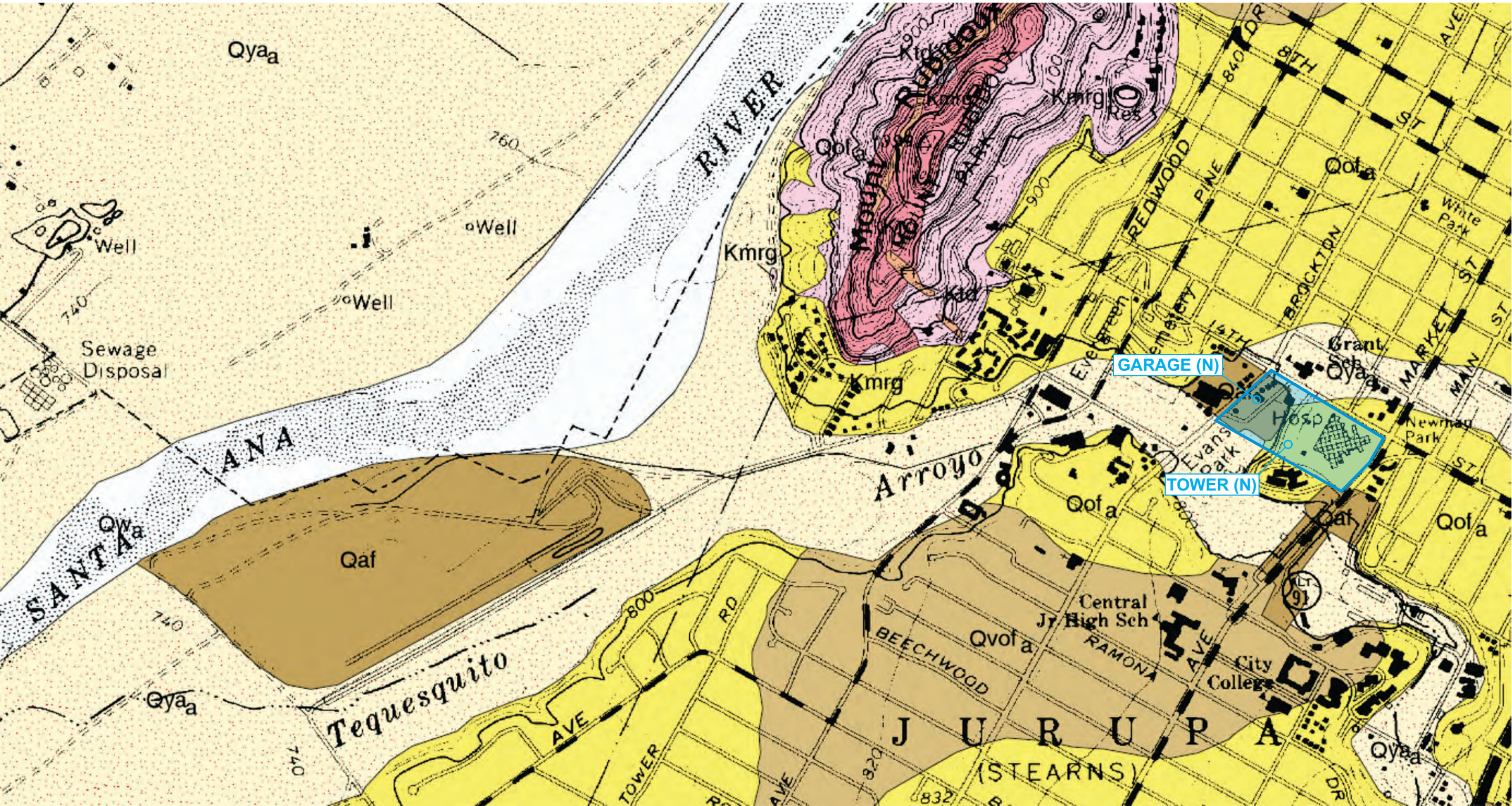
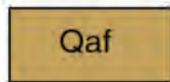
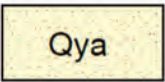




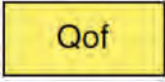
EXPLANATION:



Artificial fill (Holocene):
Mostly silty sand, clayey sand and sandy silt placed in previous grading operations.



Young Alluvium (Holocene):
Unconsolidated alluvium consisting of fine to coarse grained sand and lesser gravel and silt.



Old Alluvium (Pleistocene):
Slightly indurated, sandy alluvial fan deposits associated with the Santa Ana River system.



NO SCALE

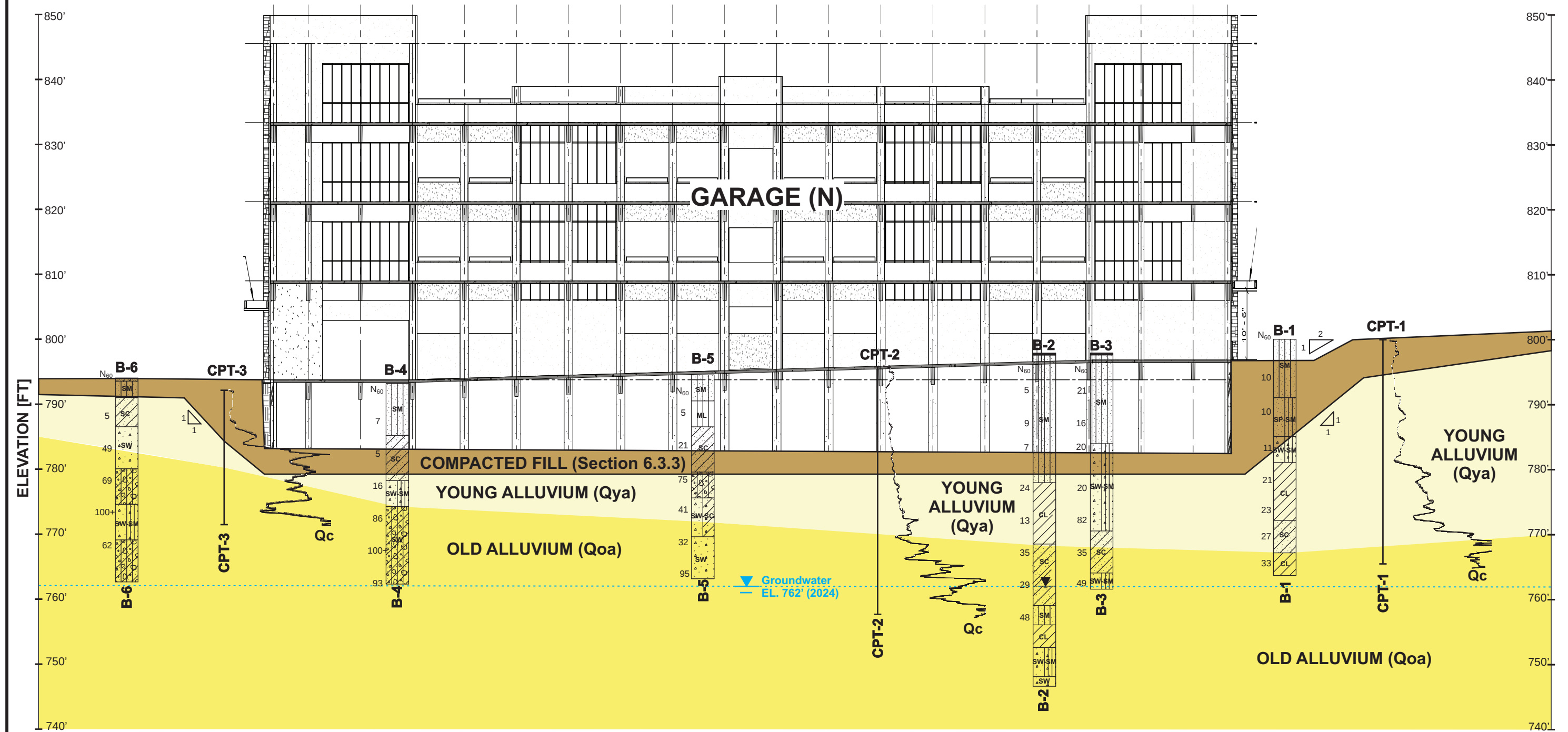


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HCA Design and Construction

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SD809
DOCUMENT NUMBER
24-0011
FIGURE NUMBER
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LOCAL GEOLOGIC MAP

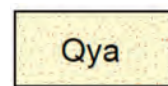
REFERENCE: Morton and Cox (2002). *Geologic Map of the Riverside West 7.5' Quadrant, Riverside, California.*



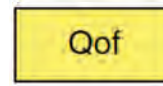
EXPLANATION:



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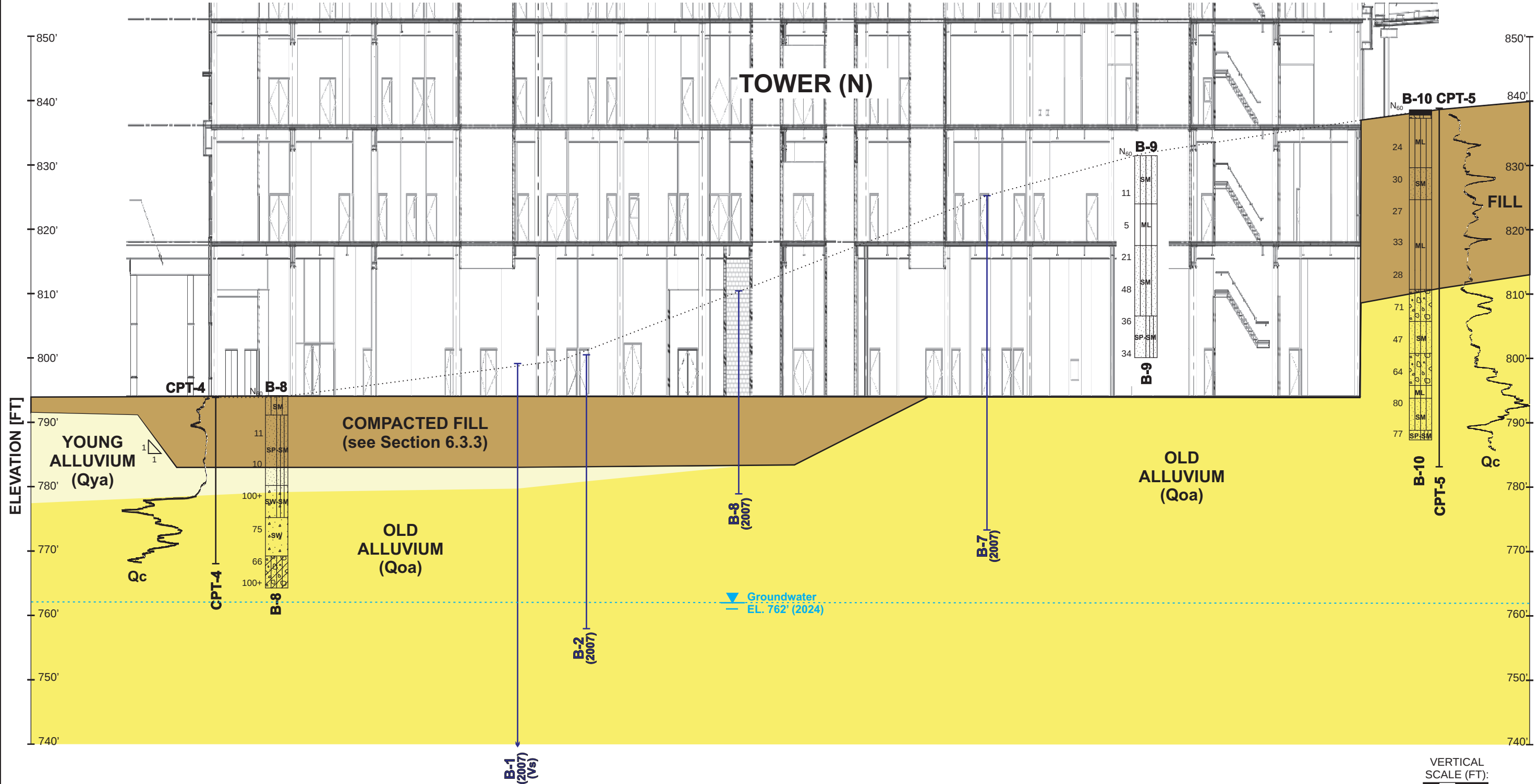
VERTICAL
SCALE (FT):
0 5 10



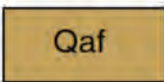
GROUP DELTA CONSULTANTS, INC.
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DOCUMENT NUMBER
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FIGURE NUMBER
4B

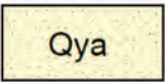
CROSS SECTION A-A'
(GARAGE SITE)



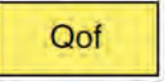
EXPLANATION:



Artificial fill (Holocene):
Mostly silty sand, clayey sand and sandy silt placed in previous grading operations.



Young Alluvium (Holocene):
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Old Alluvium (Pleistocene):
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FIGURE NUMBER
4C

CROSS SECTION B-B' (TOWER SITE)

HISTORICAL SEISMICITY



>M6 EQ



>M6-M5



>M5-M4

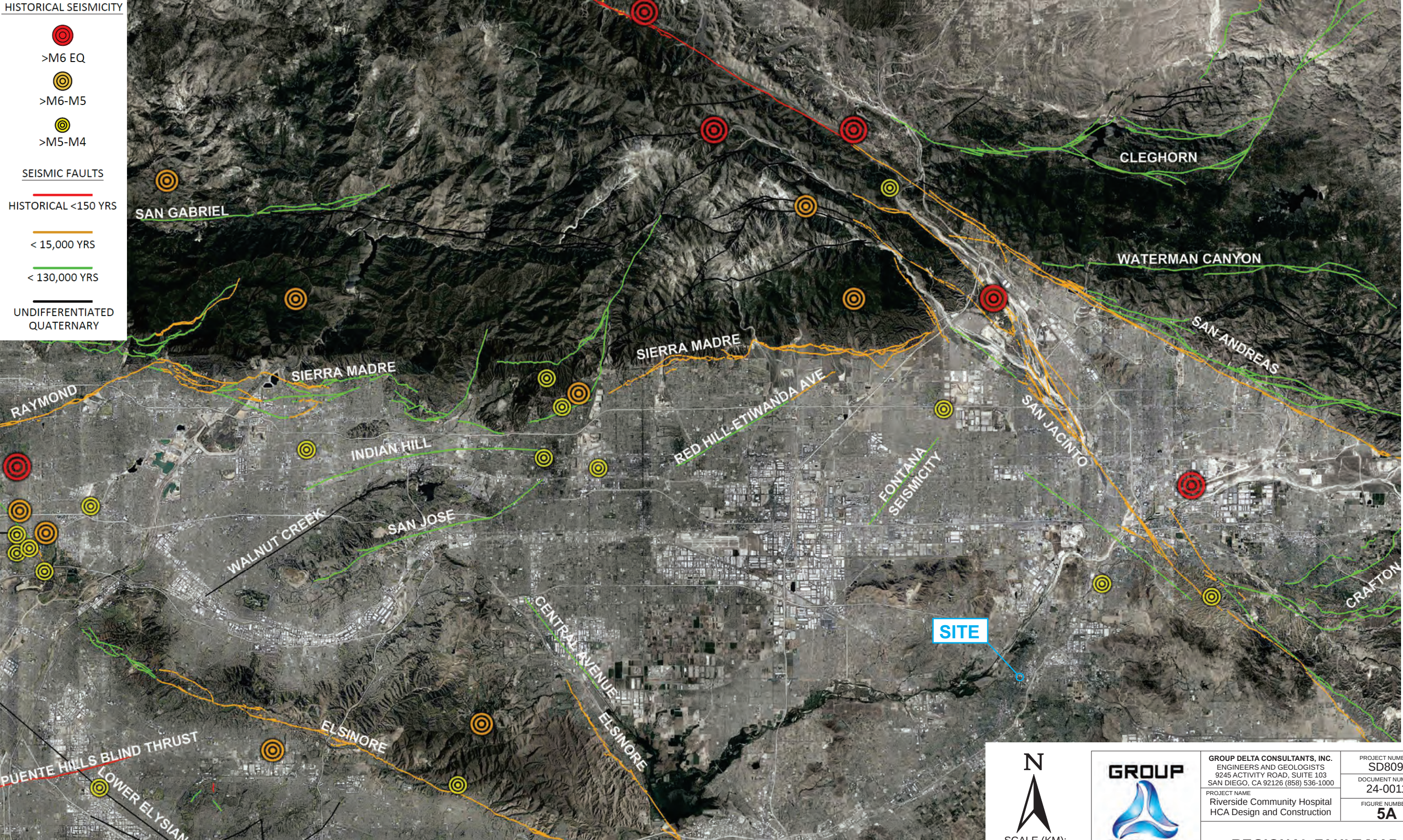
SEISMIC FAULTS

HISTORICAL <150 YRS

< 15,000 YRS

< 130,000 YRS

UNDIFFERENTIATED
QUATERNARY



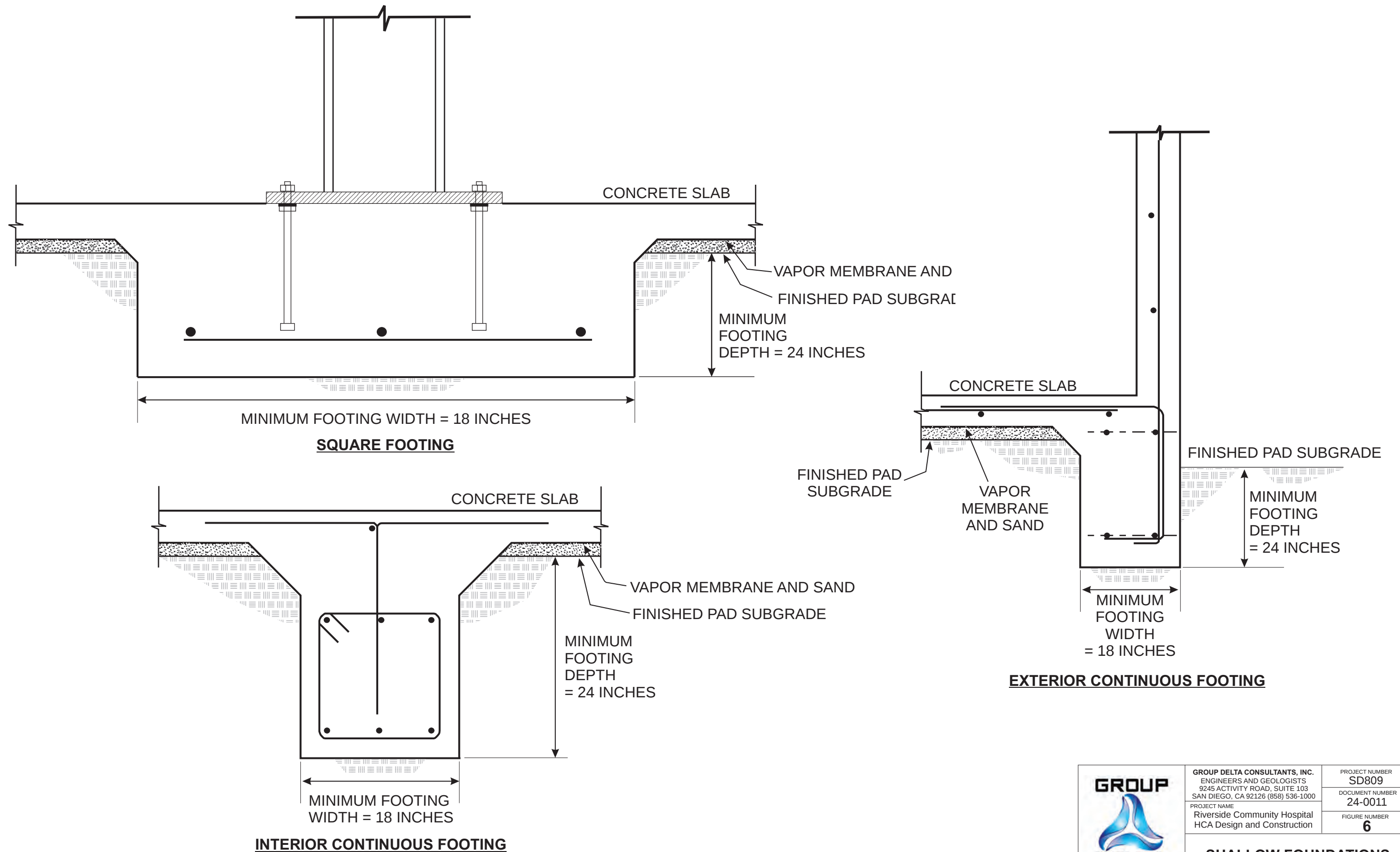
REFERENCE: Google Earth (2022). USGS Quaternary Fault and Fold Database, Imagery Date: May 30.



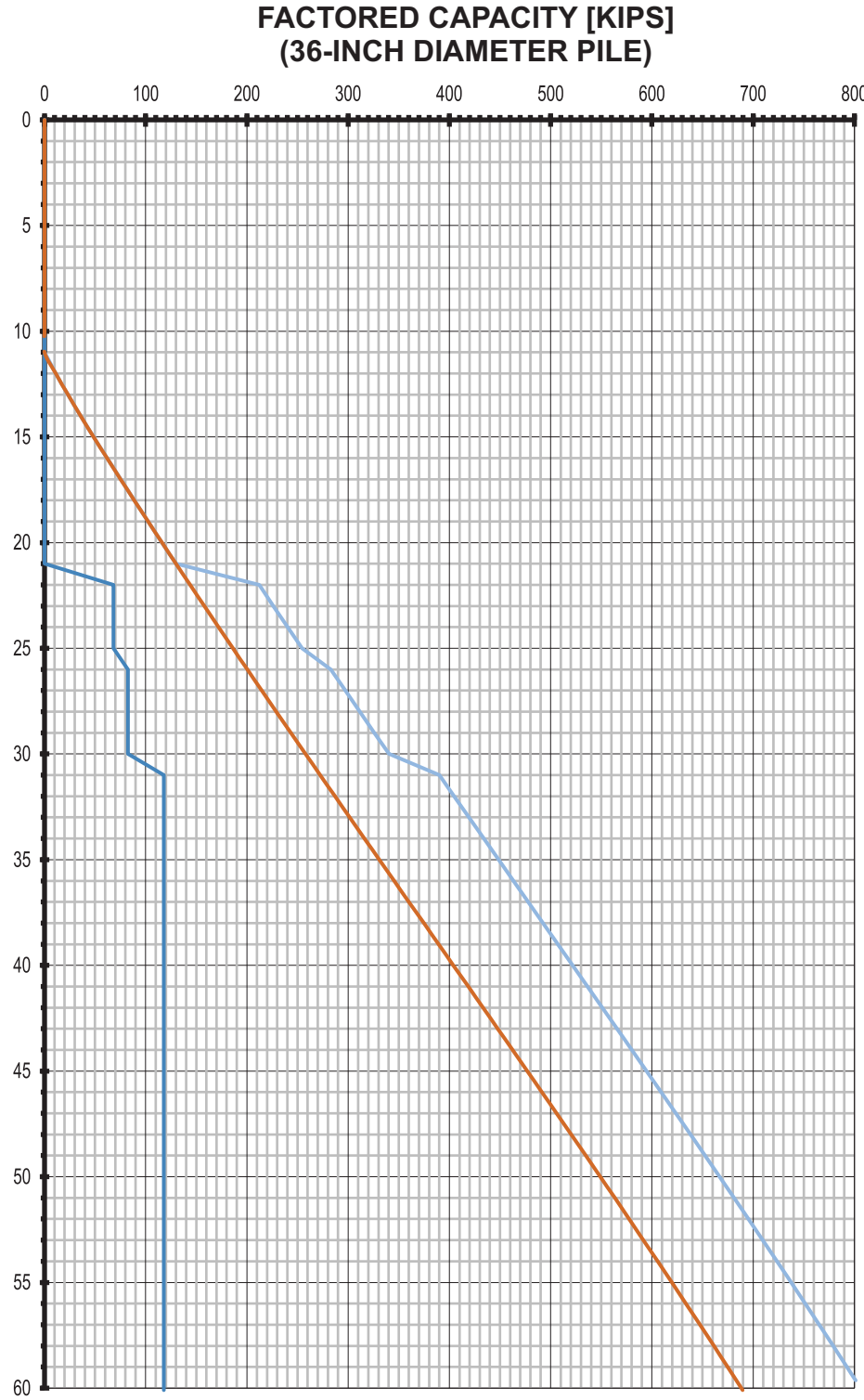
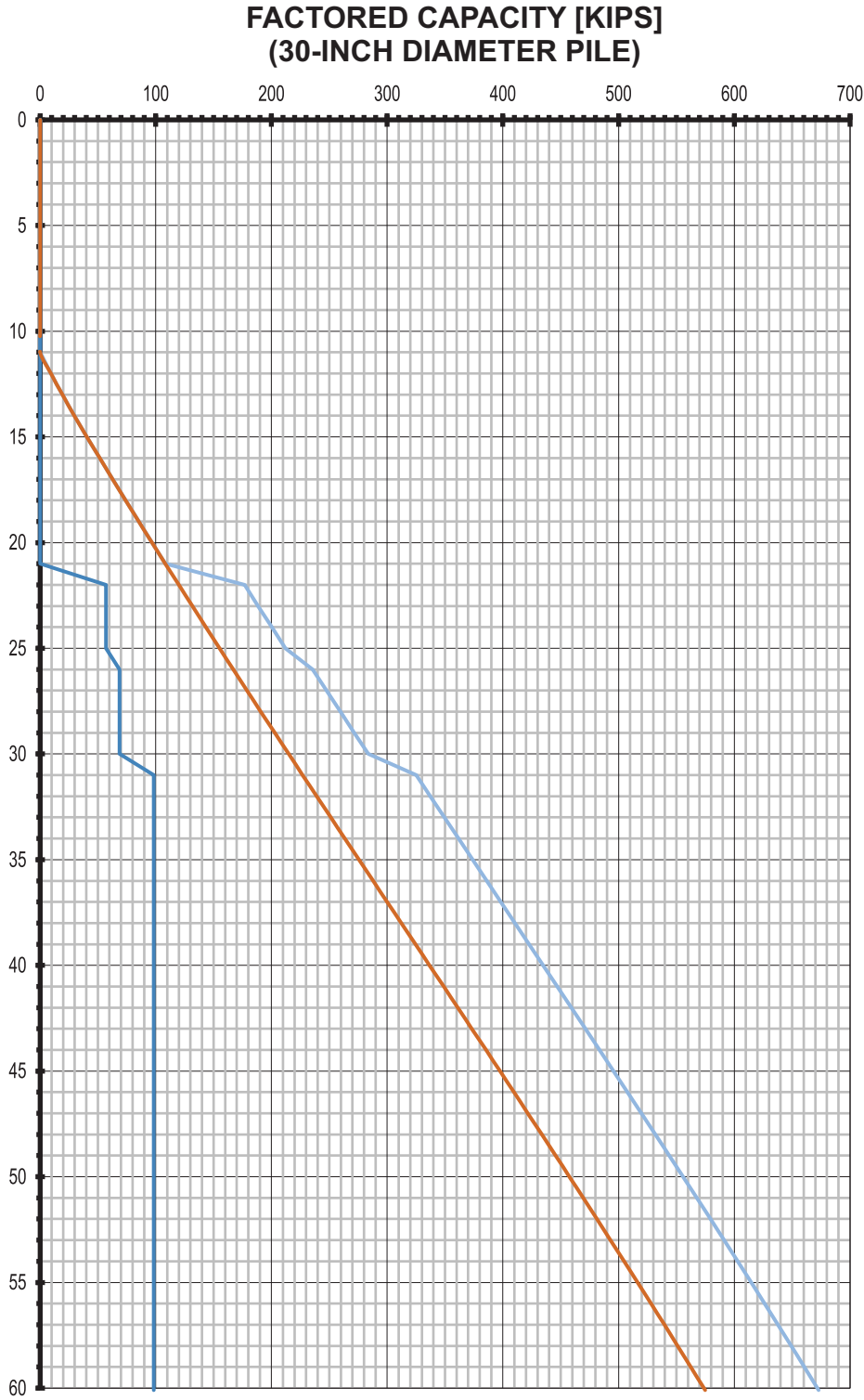
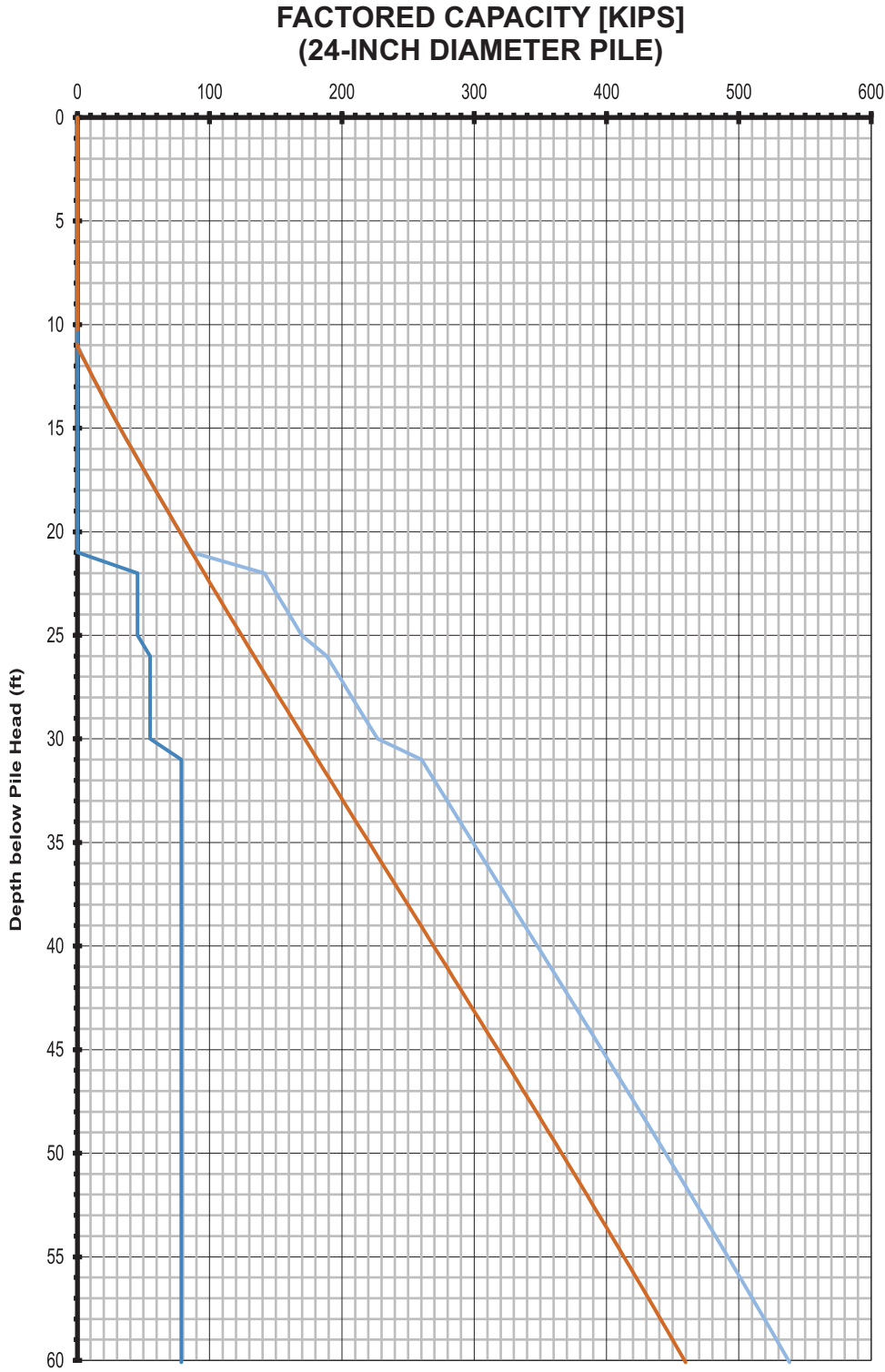
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PROJECT NAME
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REGIONAL FAULT MAP



	GROUP DELTA CONSULTANTS, INC. ENGINEERS AND GEOLOGISTS 9245 ACTIVITY ROAD, SUITE 103 SAN DIEGO, CA 92126 (858) 536-1000	
	PROJECT NAME	PROJECT NUMBER
	Riverside Community Hospital HCA Design and Construction	SD809
		DOCUMENT NUMBER
		24-0011
		FIGURE NUMBER
		6
SHALLOW FOUNDATIONS		



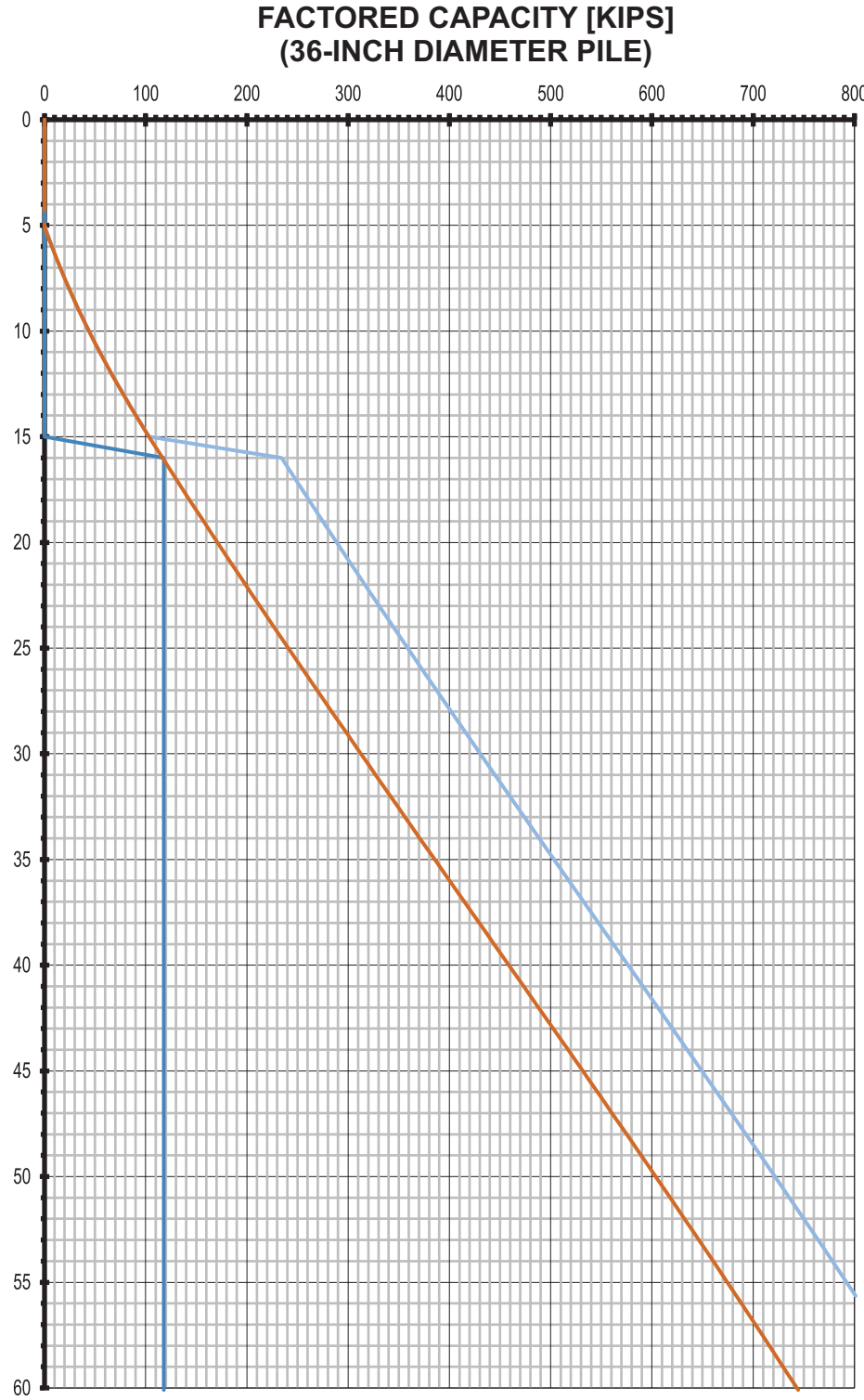
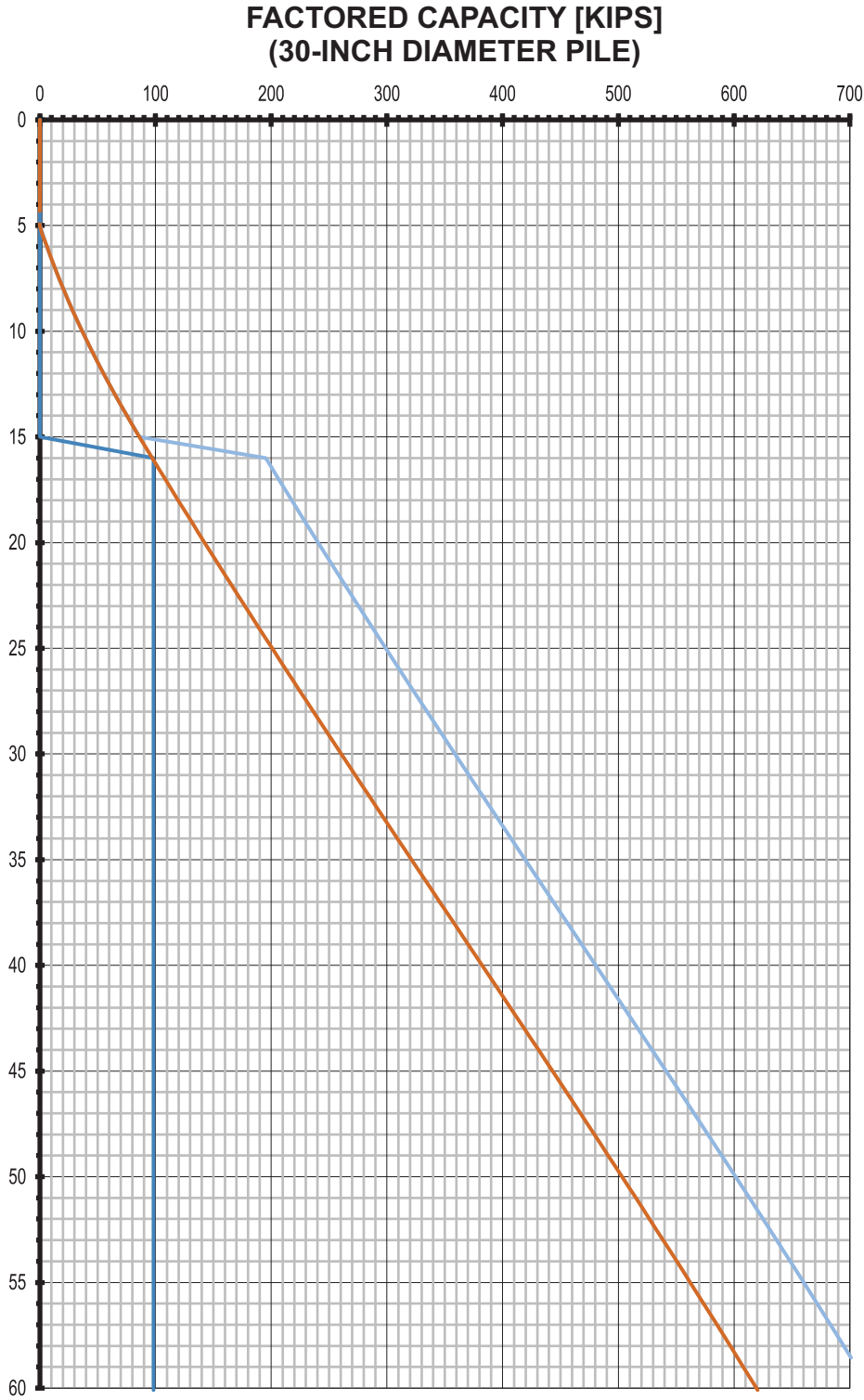
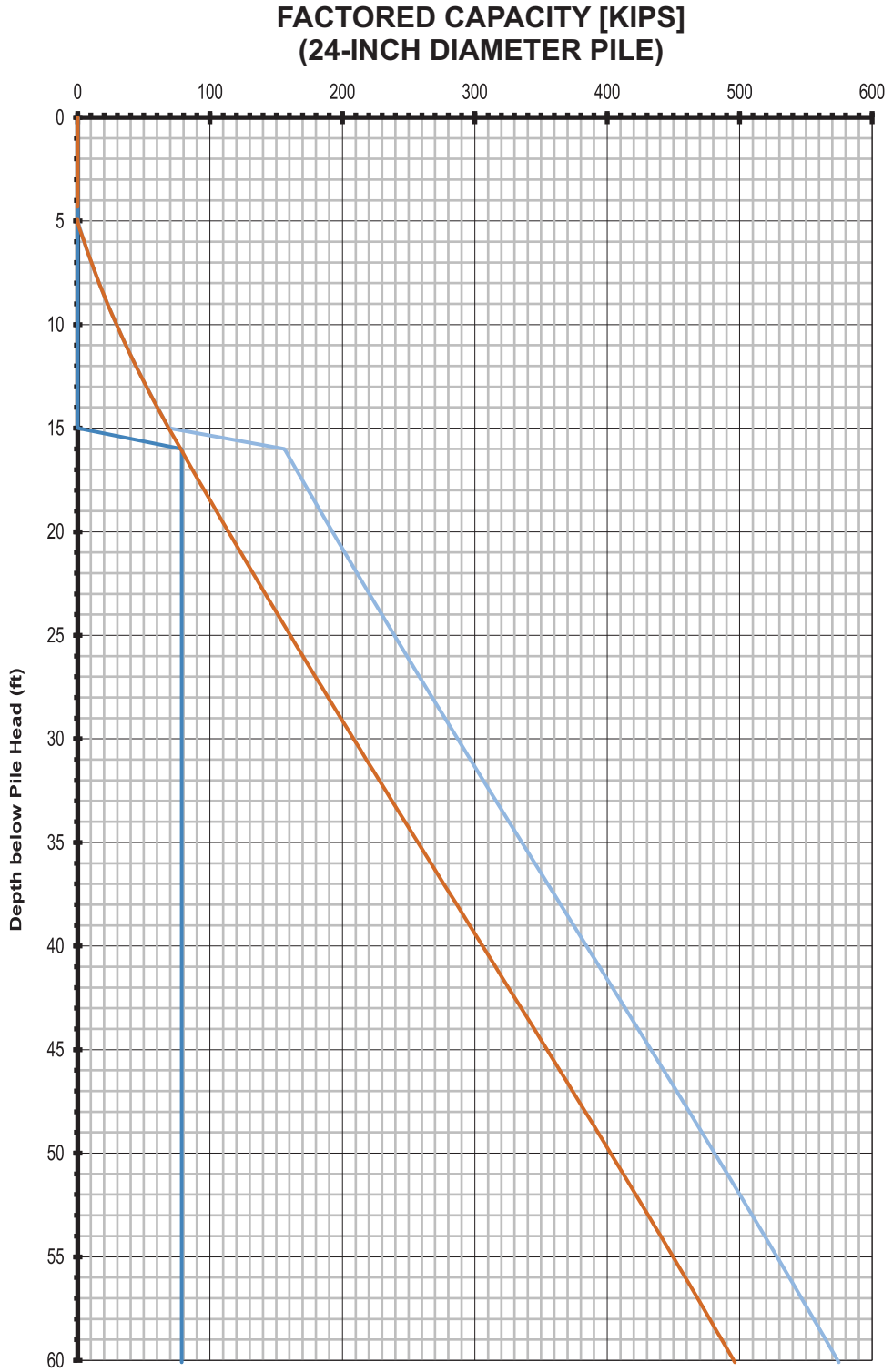
NOTES:

- 1) CIDH pile excavations will need stabilization using drilling slurry and casing.
- 2) All piles should be embedded at least 10-feet into dense Old Alluvium.





GROUP DELTA CONSULTANTS, INC. ENGINEERS AND GEOLOGISTS 9245 ACTIVITY ROAD, SUITE 103 SAN DIEGO, CA 92126 (858) 536-1000 PROJECT NAME Riverside Community Hospital HCA Design and Construction	PROJECT NUMBER SD809
	DOCUMENT NUMBER 24-0011
	FIGURE NUMBER 7A
AXIAL PILE CAPACITY (GARAGE SITE - NORTH)	



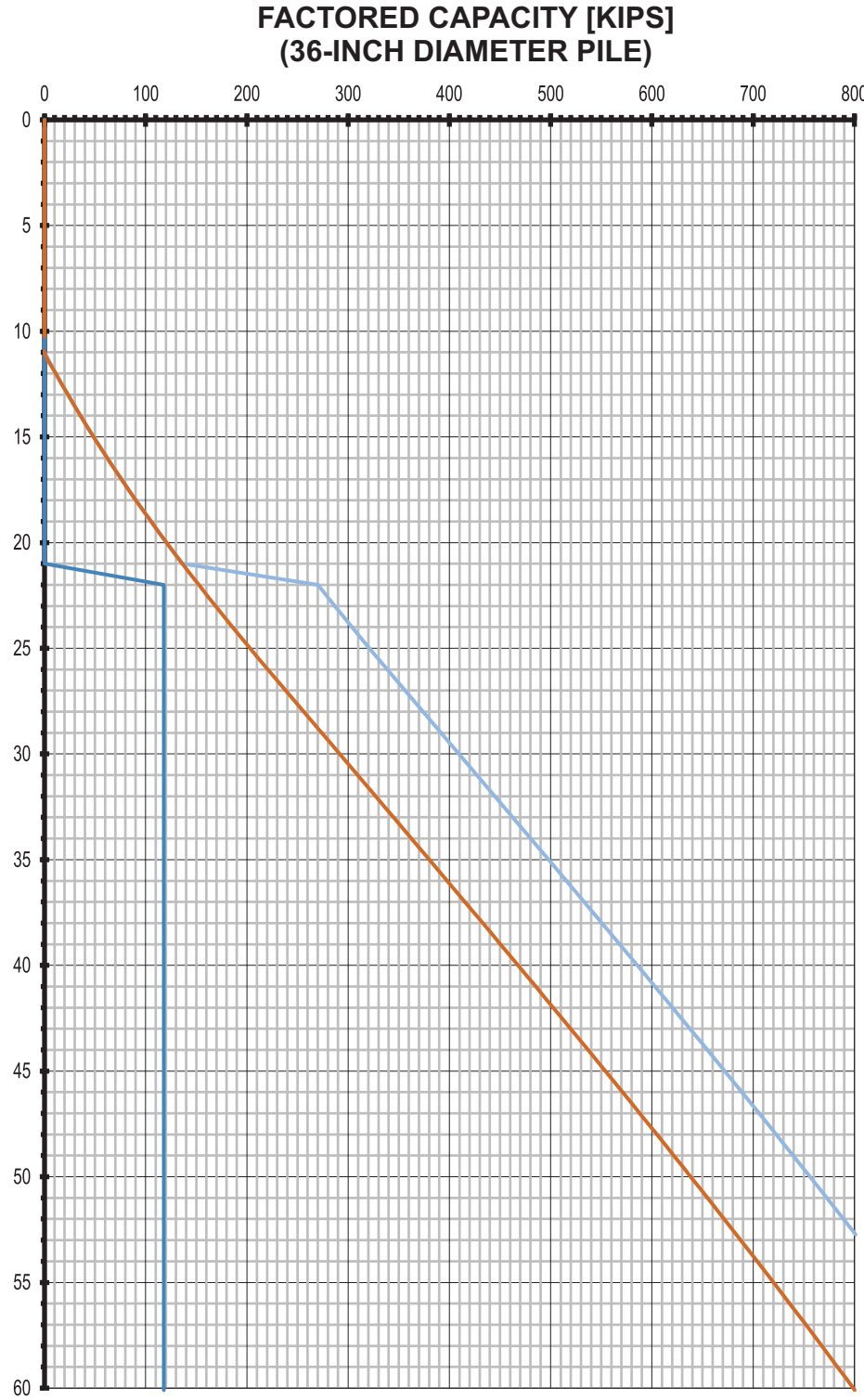
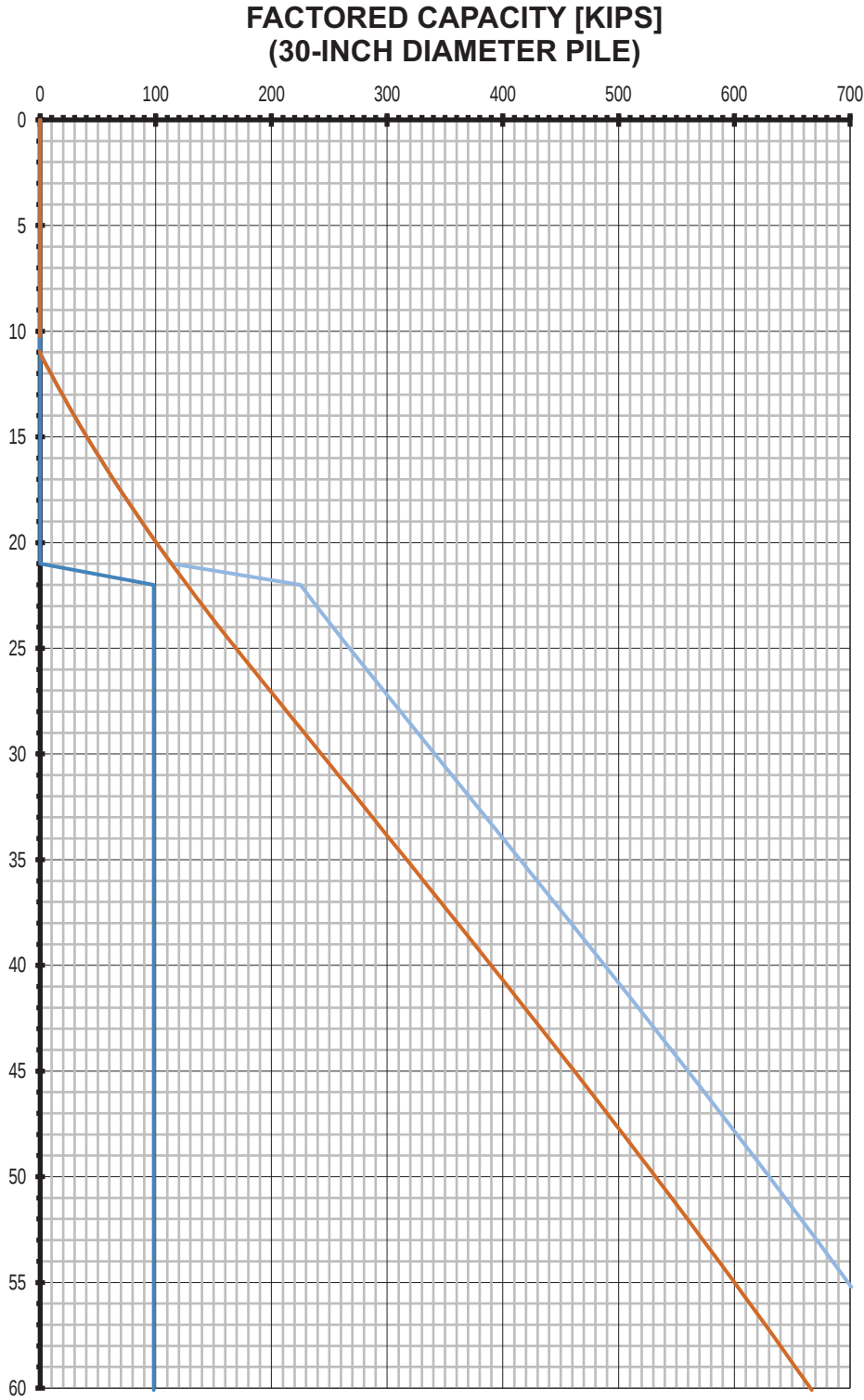
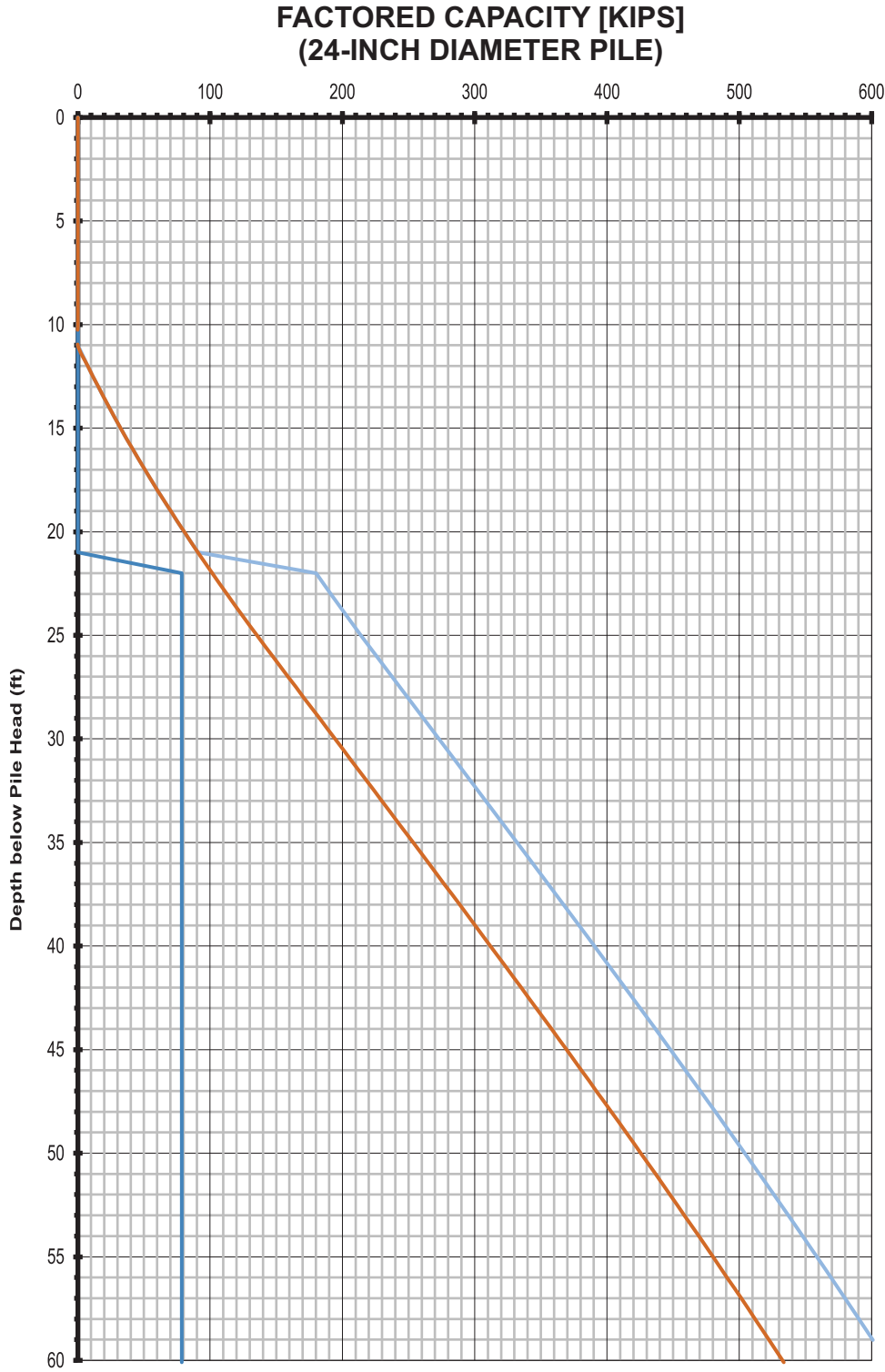
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GROUP DELTA CONSULTANTS, INC. ENGINEERS AND GEOLOGISTS 9245 ACTIVITY ROAD, SUITE 103 SAN DIEGO, CA 92126 (858) 536-1000 PROJECT NAME Riverside Community Hospital HCA Design and Construction	PROJECT NUMBER SD809
	DOCUMENT NUMBER 24-0011
	FIGURE NUMBER 7B
AXIAL PILE CAPACITY (GARAGE SITE - SOUTH)	



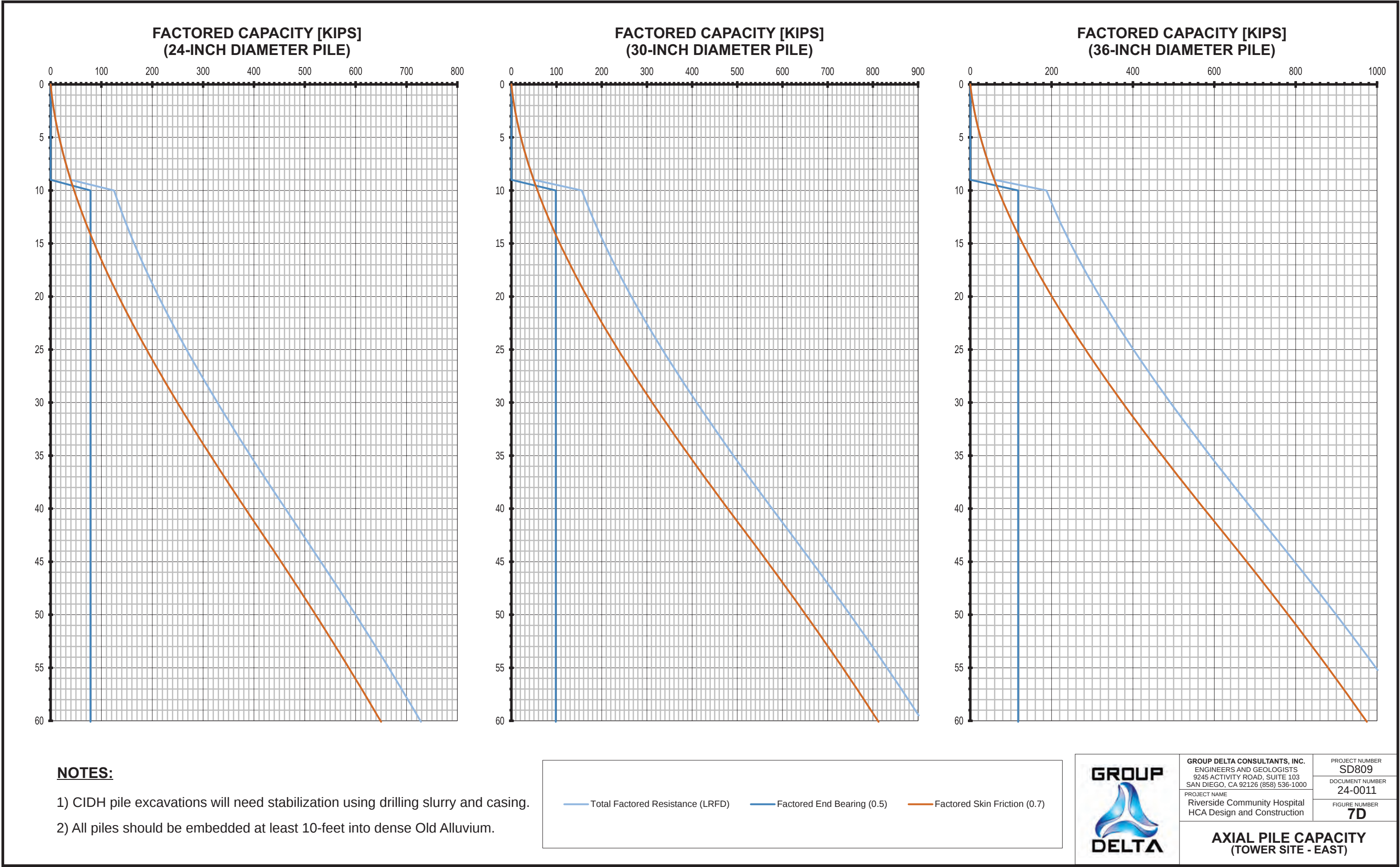
NOTES:

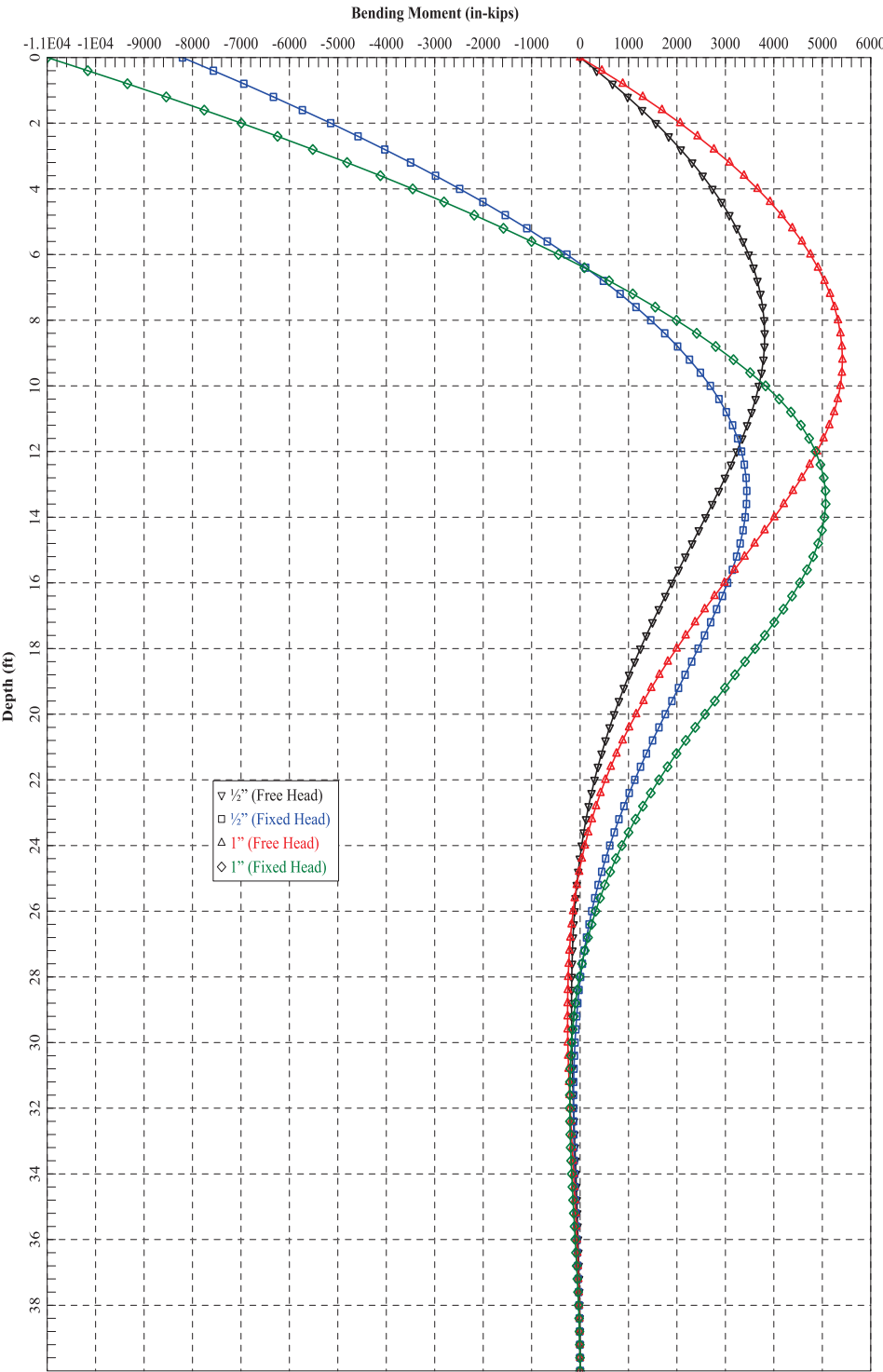
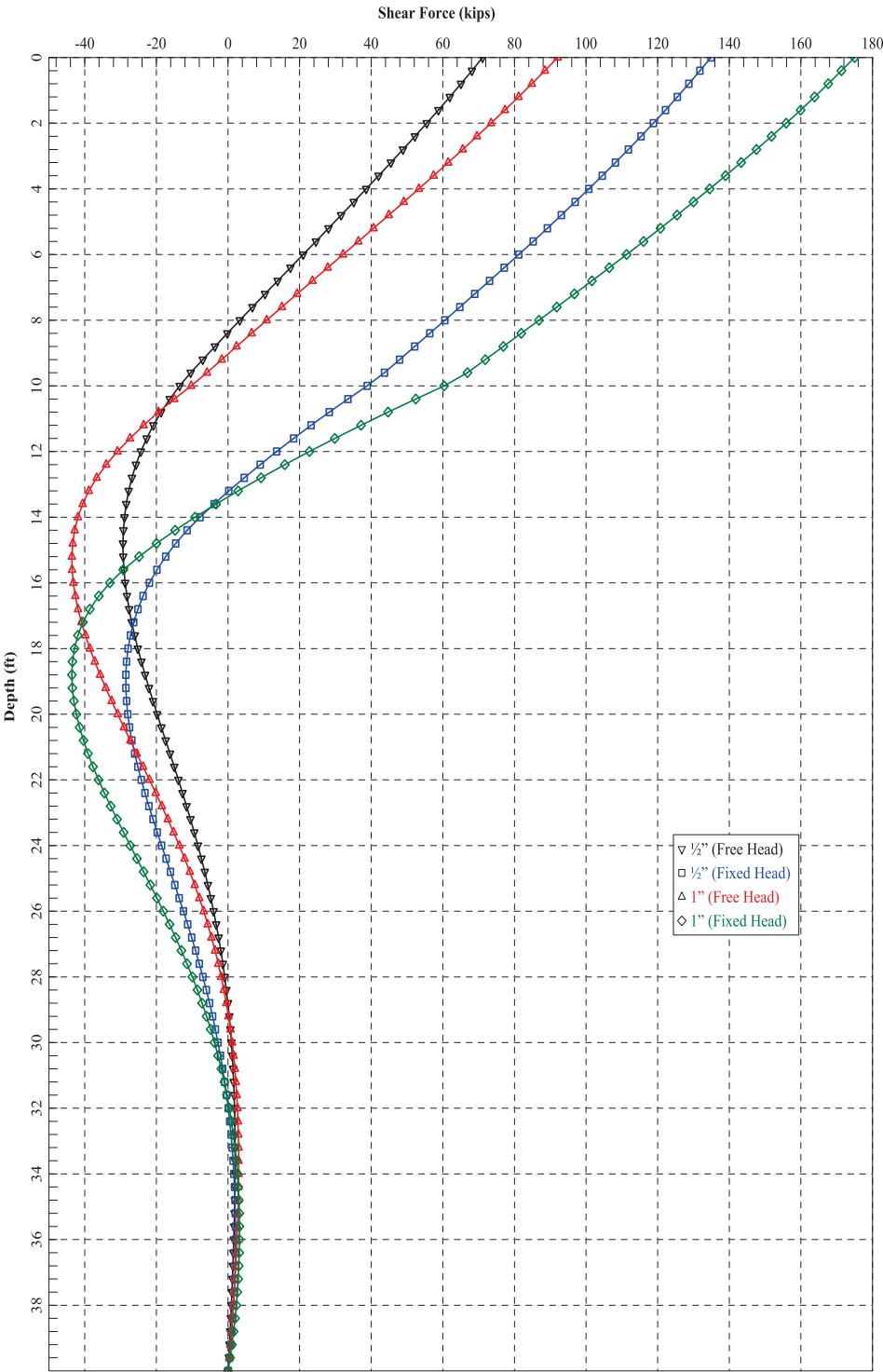
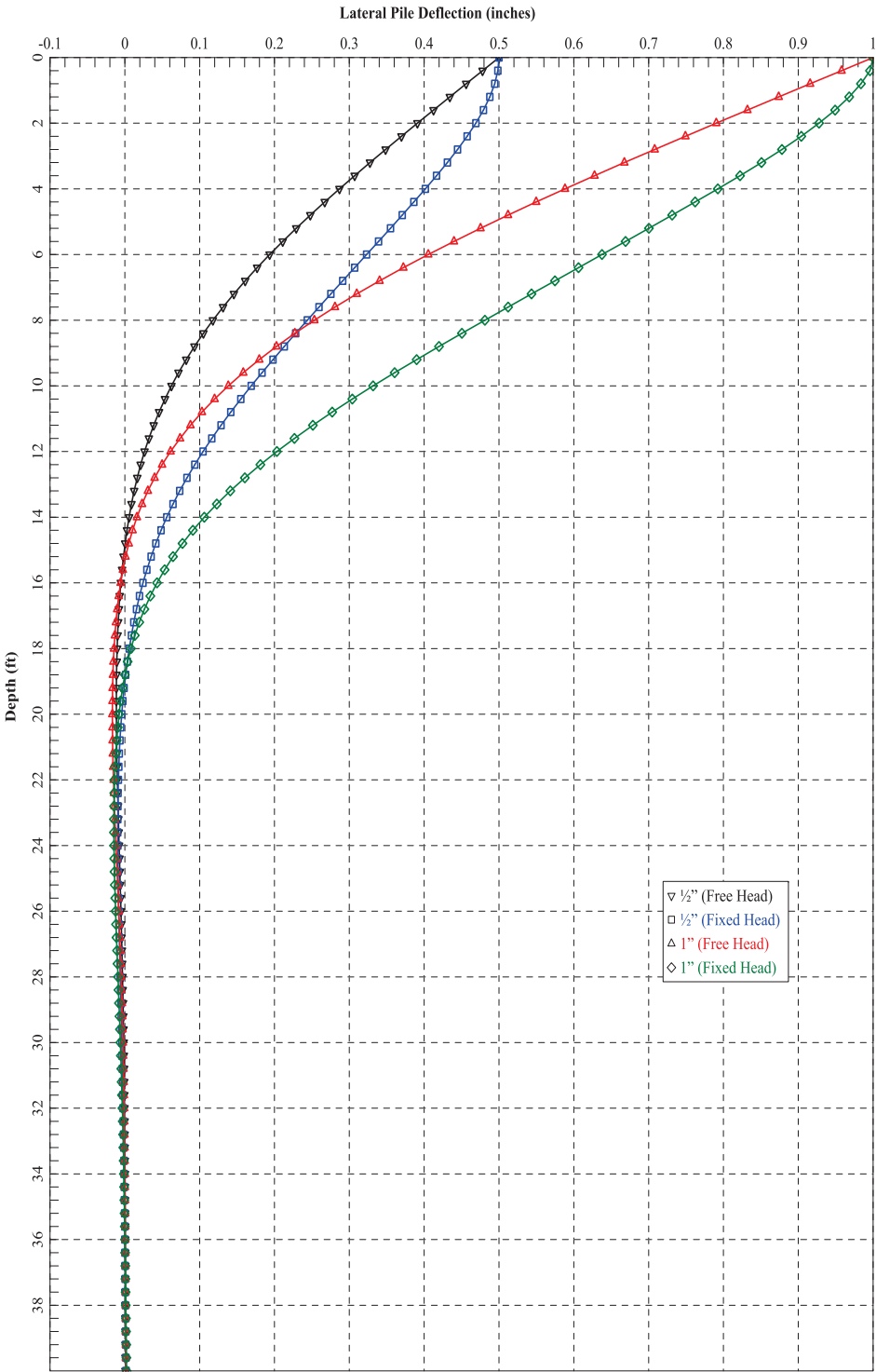
- 1) CIDH pile excavations will need stabilization using drilling slurry and casing.
- 2) All piles should be embedded at least 10-feet into dense Old Alluvium.





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	DOCUMENT NUMBER 24-0011
	FIGURE NUMBER 7C
AXIAL PILE CAPACITY (TOWER SITE - WEST)	





NOTES:

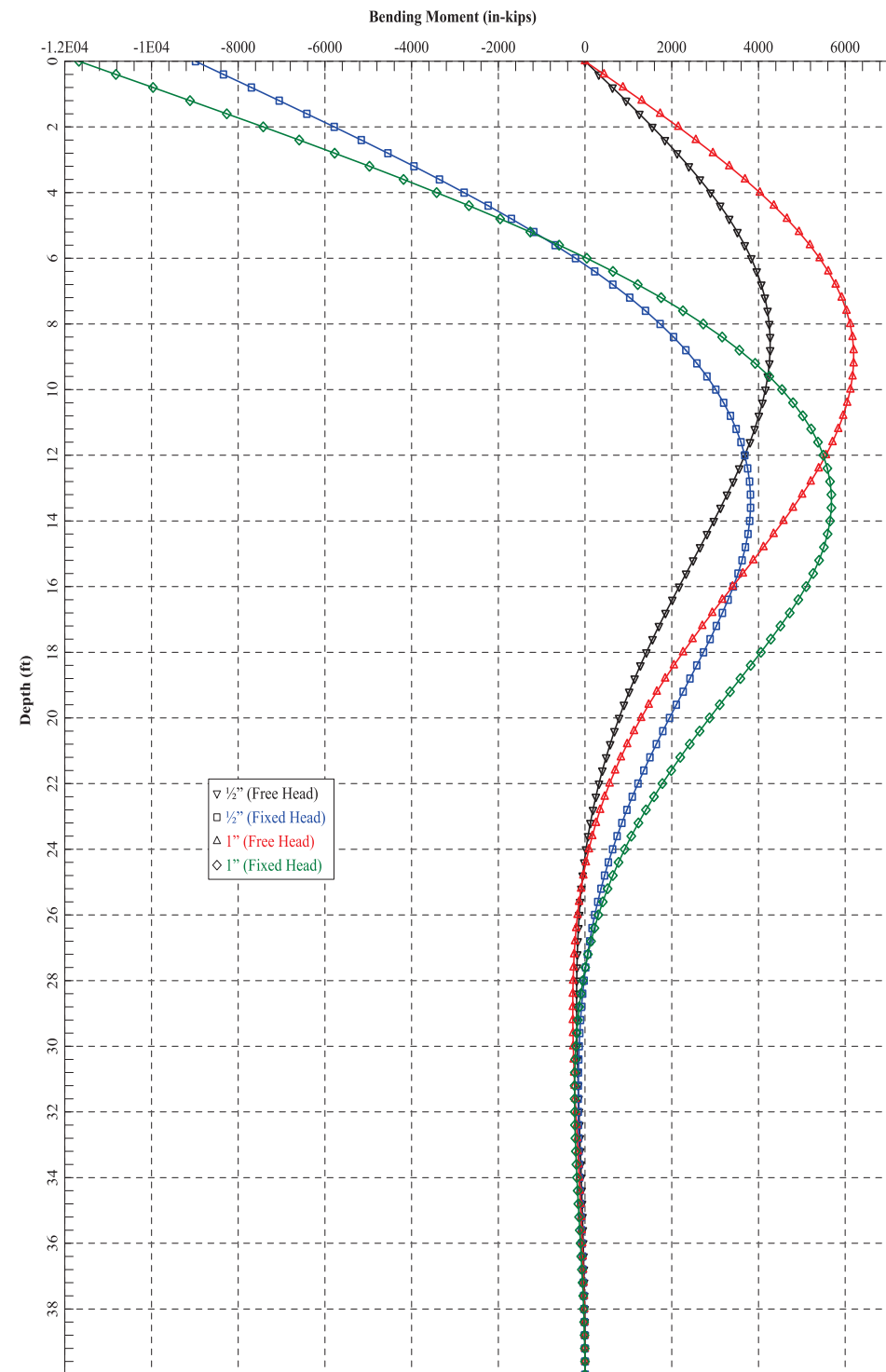
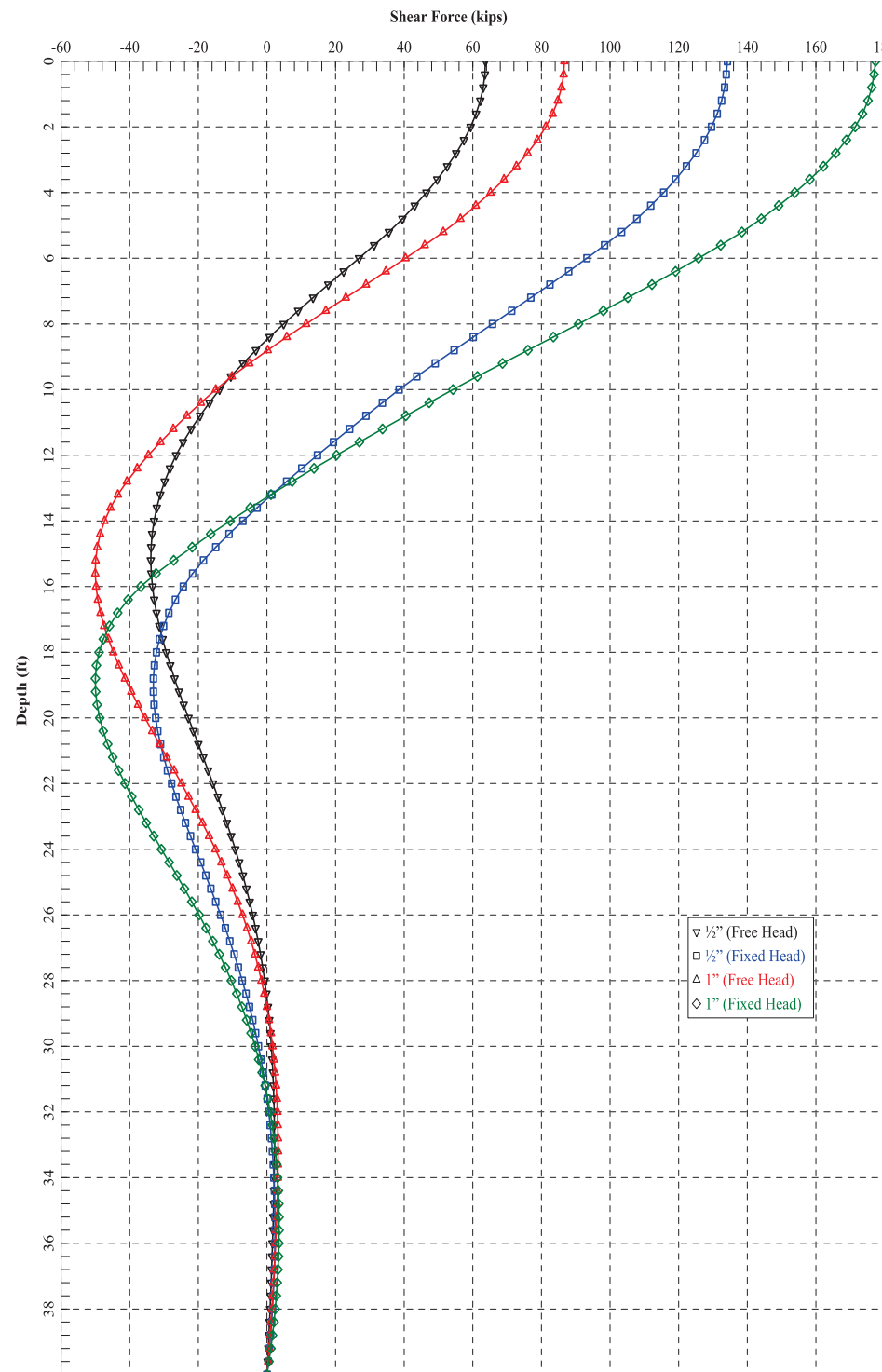
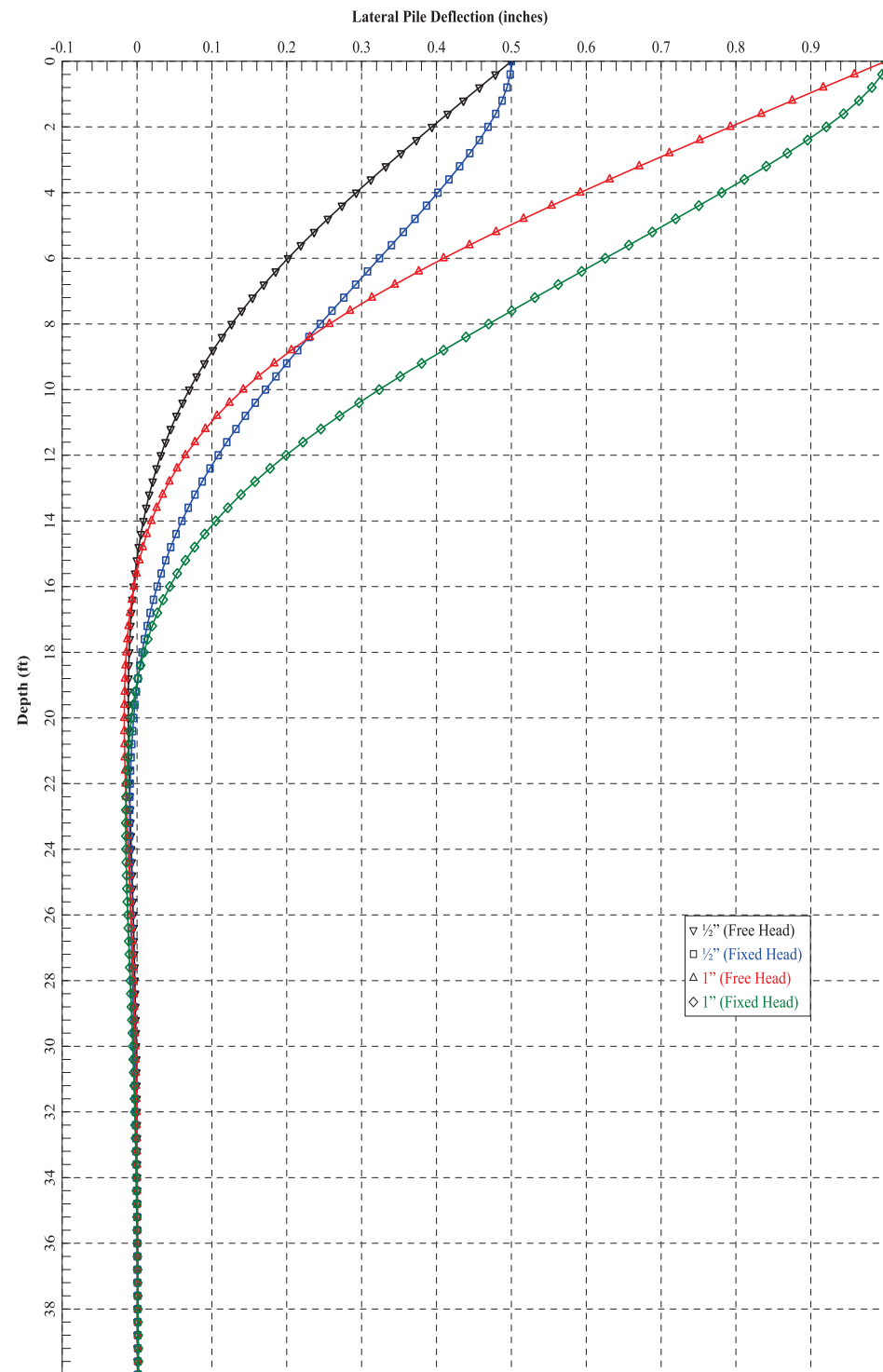
- 1) The approximate lateral response of a single 30-inch diameter, 40-foot long pile is shown in the deflection, shear and moment diagrams provided above.
- 2) The analyses assume at least 10-foot embedment into dense Old Alluvium, with 4,000 psi concrete strength and (6) No. 14 transverse bars (Grade 60).



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PROJECT NAME
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HCA Design and Construction

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DOCUMENT NUMBER
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FIGURE NUMBER
8A

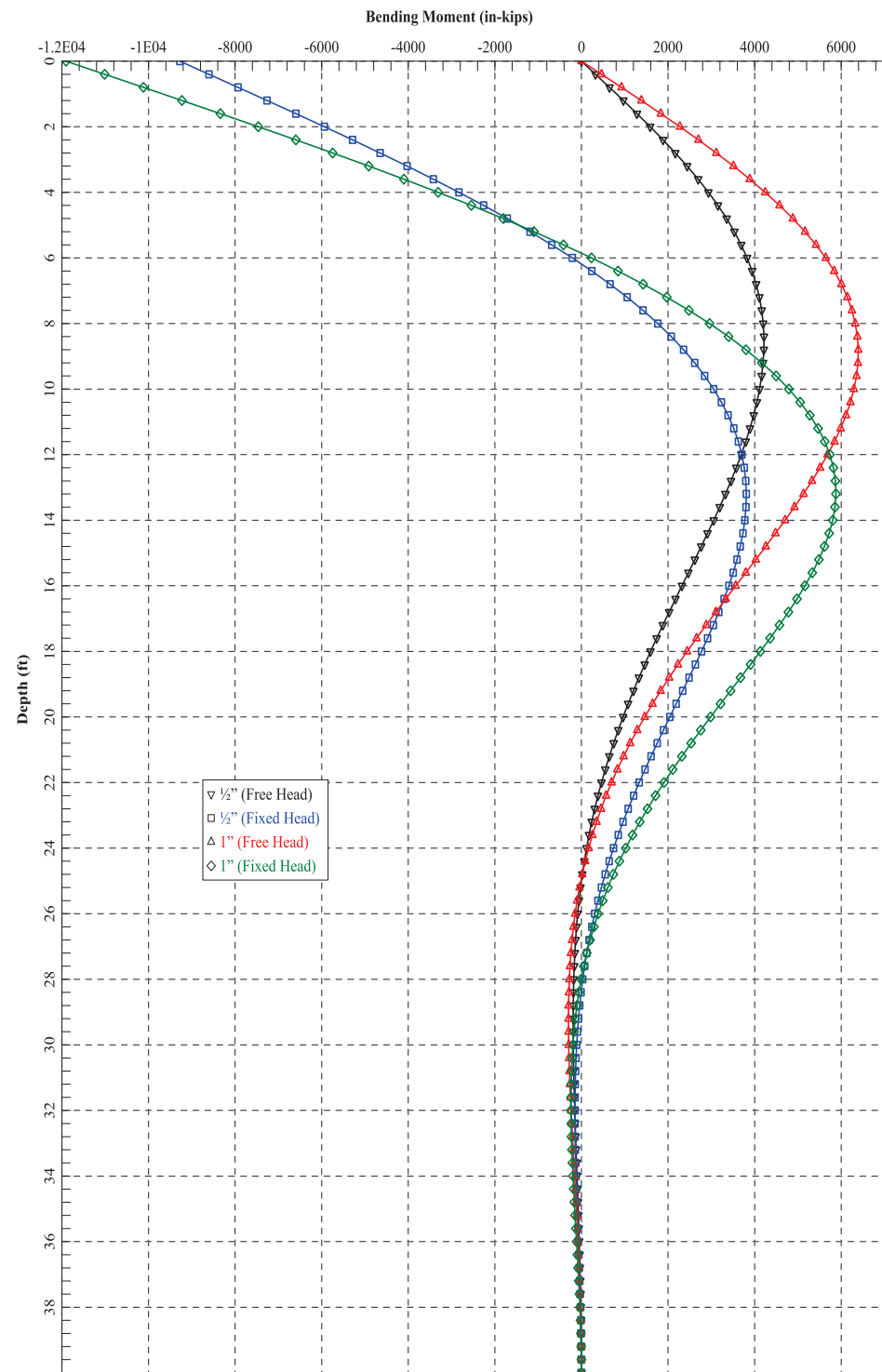
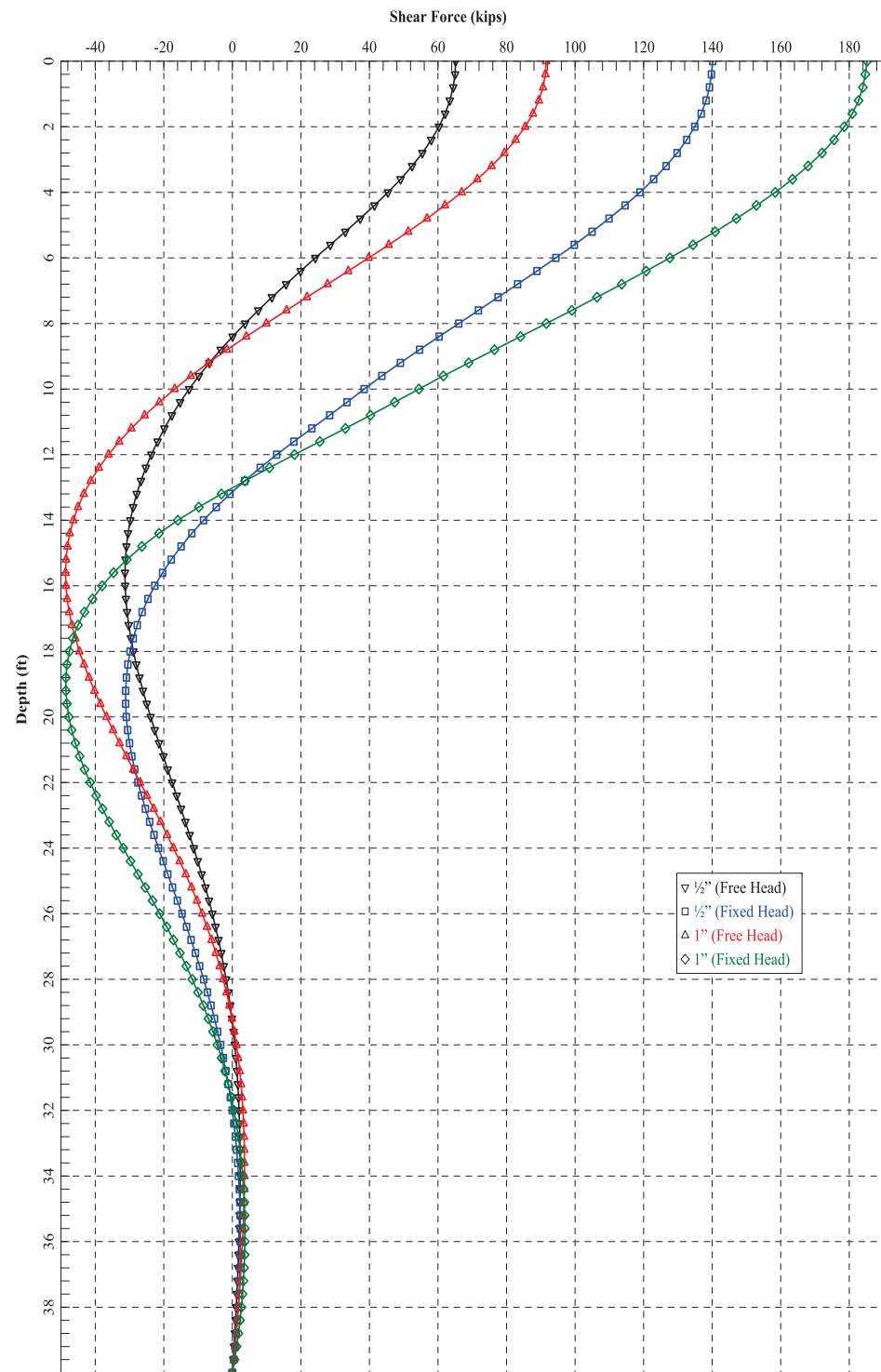
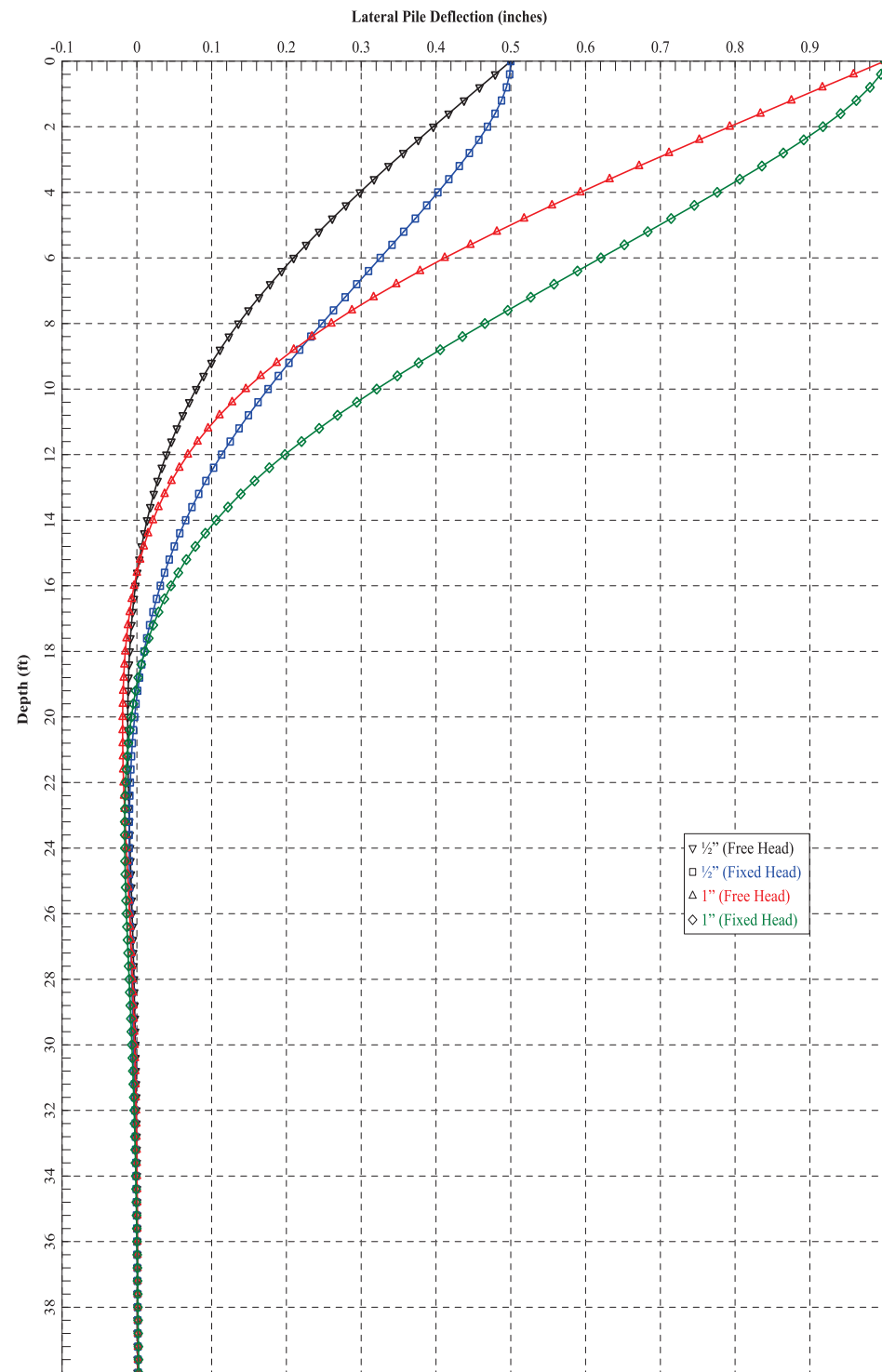
**LATERAL PILE CAPACITY
(GARAGE SITE - NORTH)**



NOTES:

- 1) The approximate lateral response of a single 30-inch diameter, 40-foot long pile is shown in the deflection, shear and moment diagrams provided above.
- 2) The analyses assume at least 10-foot embedment into dense Old Alluvium, with 4,000 psi concrete strength and (6) No. 14 transverse bars (Grade 60).

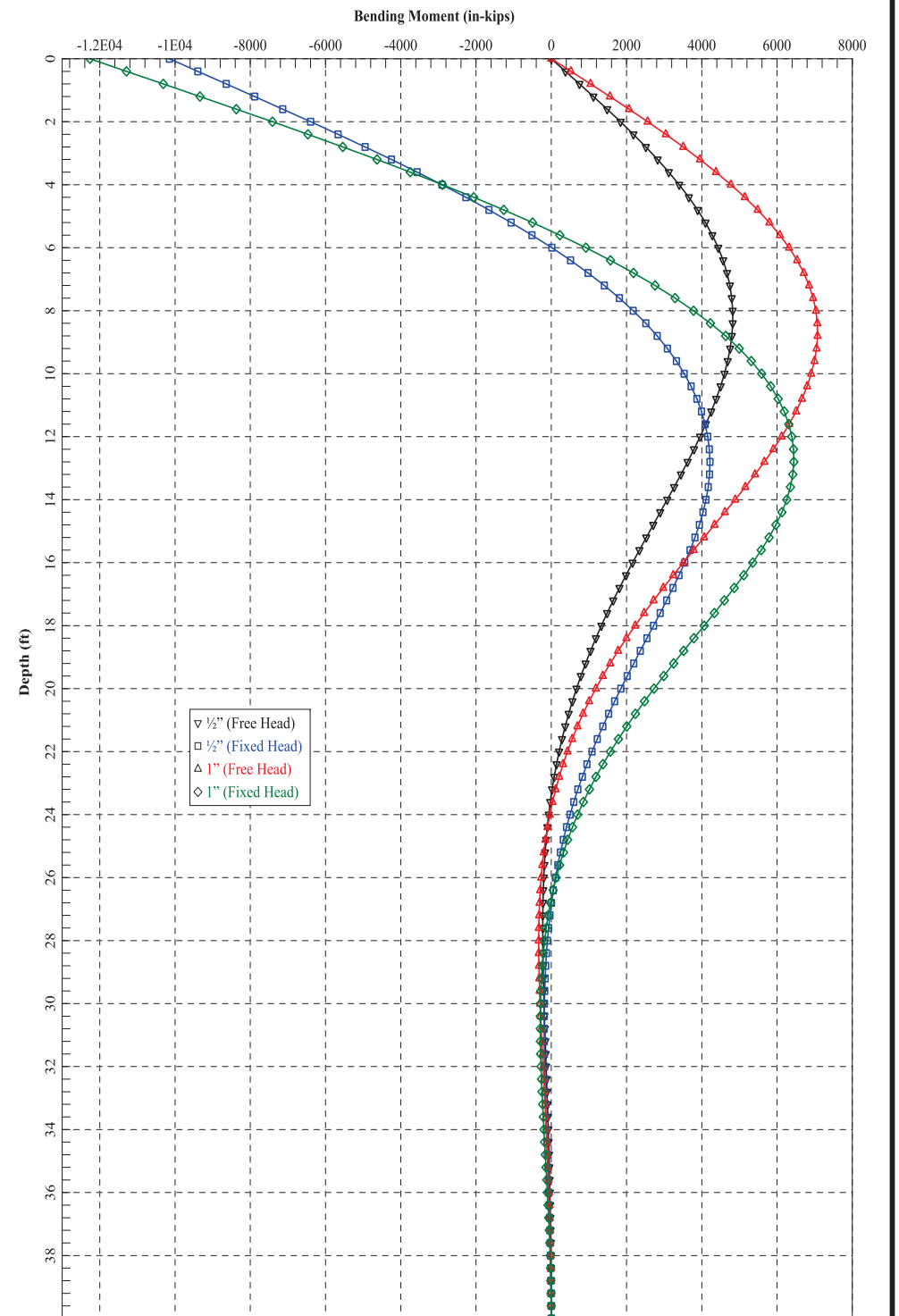
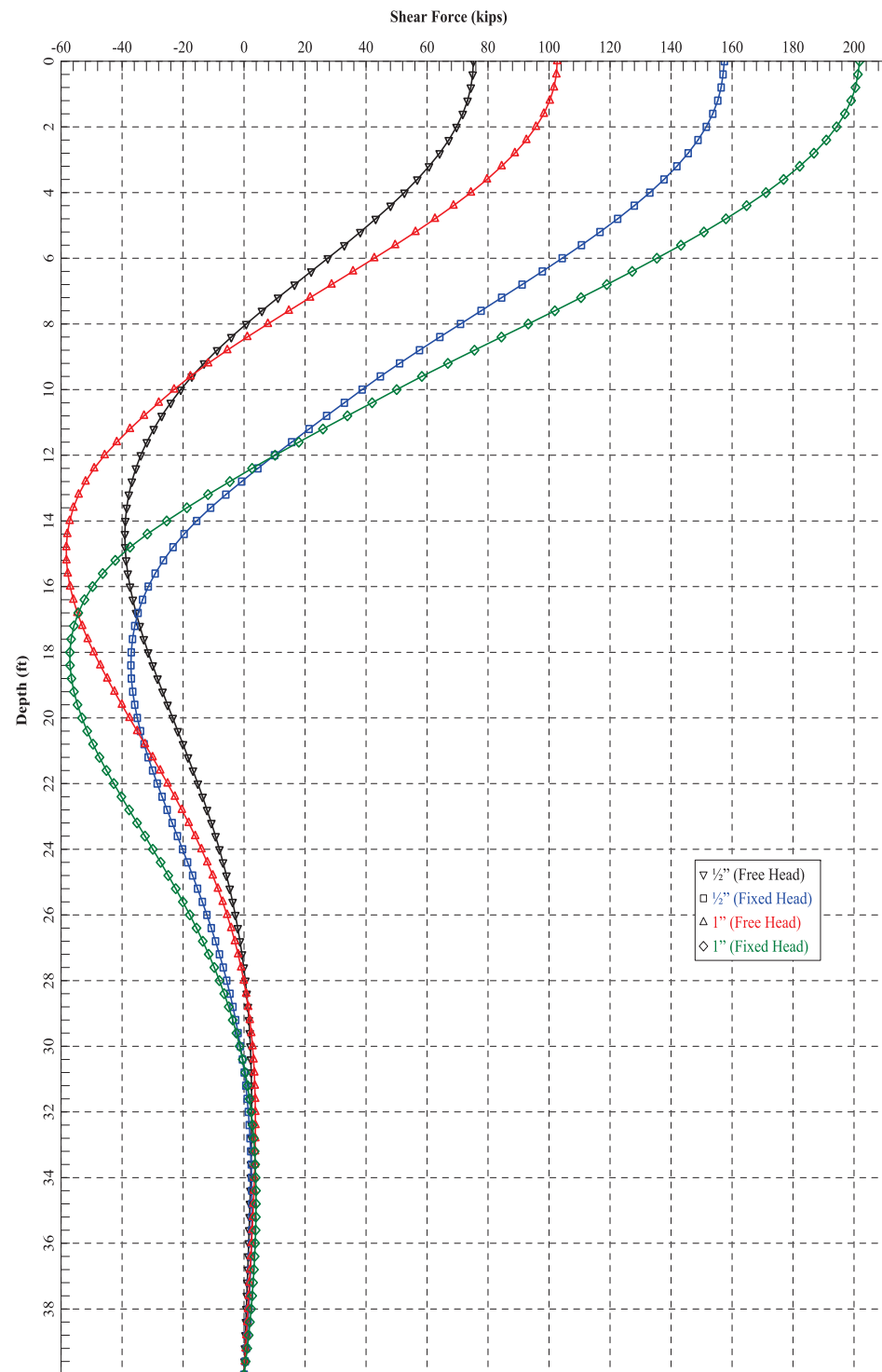
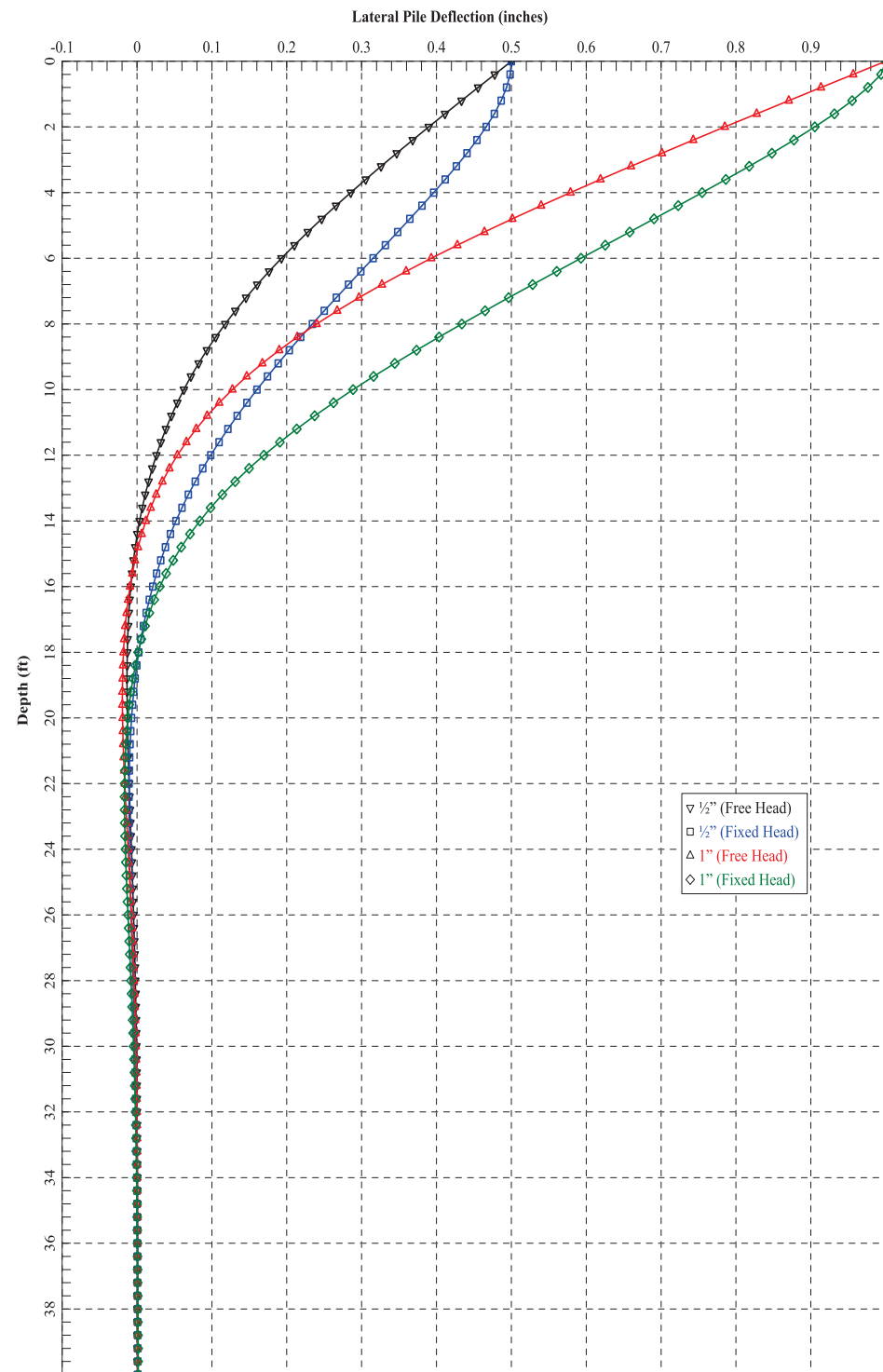
	GROUP DELTA CONSULTANTS, INC. ENGINEERS AND GEOLOGISTS 9245 ACTIVITY ROAD, SUITE 103 SAN DIEGO, CA 92126 (858) 536-1000	PROJECT NUMBER SD809
	PROJECT NAME Riverside Community Hospital HCA Design and Construction	DOCUMENT NUMBER 24-0011
		FIGURE NUMBER 8B
LATERAL PILE CAPACITY (GARAGE SITE - SOUTH)		



NOTES:

- 1) The approximate lateral response of a single 30-inch diameter, 40-foot long pile is shown in the deflection, shear and moment diagrams provided above.
- 2) The analyses assume at least 10-foot embedment into dense Old Alluvium, with 4,000 psi concrete strength and (6) No. 14 transverse bars (Grade 60).

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	PROJECT NAME Riverside Community Hospital HCA Design and Construction		DOCUMENT NUMBER 24-0011
			FIGURE NUMBER 8C
	LATERAL PILE CAPACITY (TOWER SITE - WEST)		



NOTES:

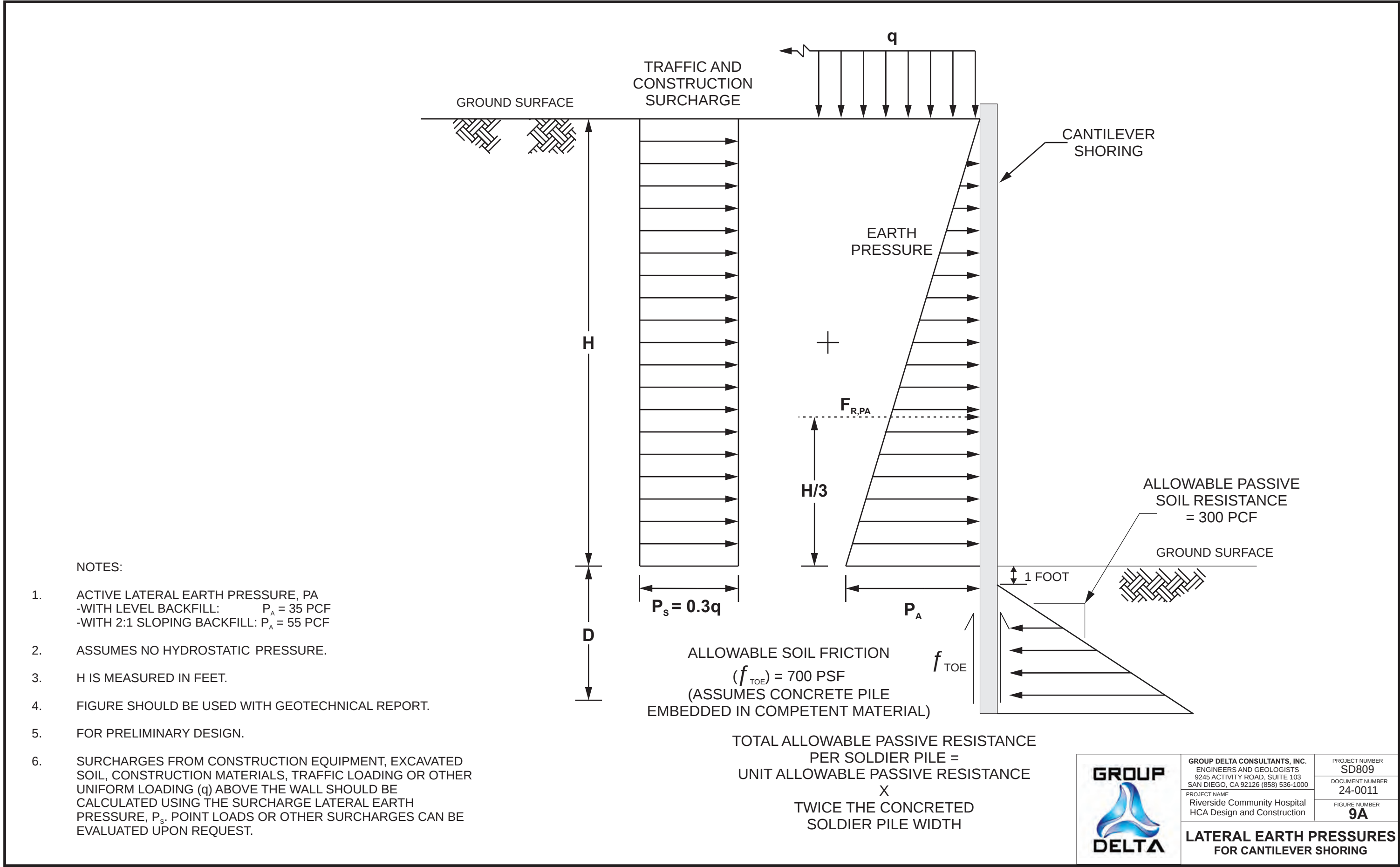
- 1) The approximate lateral response of a single 30-inch diameter, 40-foot long pile is shown in the deflection, shear and moment diagrams provided above.
- 2) These analyses assume the eastern Tower piles are completely embedded within dense Old Alluvium, with a pile cut-off below Elevation 810 feet (MSL).



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HCA Design and Construction

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DOCUMENT NUMBER
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FIGURE NUMBER
8D

**LATERAL PILE CAPACITY
(TOWER SITE - EAST)**



NOTES:

- 1. ACTIVE LATERAL EARTH PRESSURE, P_A
-WITH LEVEL BACKFILL: $P_A = 35 \text{ PCF}$
-WITH 2:1 SLOPING BACKFILL: $P_A = 55 \text{ PCF}$
- 2. ASSUMES NO HYDROSTATIC PRESSURE.
- 3. H IS MEASURED IN FEET.
- 4. FIGURE SHOULD BE USED WITH GEOTECHNICAL REPORT.
- 5. FOR PRELIMINARY DESIGN.
- 6. SURCHARGES FROM CONSTRUCTION EQUIPMENT, EXCAVATED SOIL, CONSTRUCTION MATERIALS, TRAFFIC LOADING OR OTHER UNIFORM LOADING (q) ABOVE THE WALL SHOULD BE CALCULATED USING THE SURCHARGE LATERAL EARTH PRESSURE, P_s . POINT LOADS OR OTHER SURCHARGES CAN BE EVALUATED UPON REQUEST.

ALLOWABLE SOIL FRICTION
(f_{TOE}) = 700 PSF
(ASSUMES CONCRETE PILE
EMBEDDED IN COMPETENT MATERIAL)

TOTAL ALLOWABLE PASSIVE RESISTANCE
PER SOLDIER PILE =
UNIT ALLOWABLE PASSIVE RESISTANCE
X
TWICE THE CONCRETED
SOLDIER PILE WIDTH

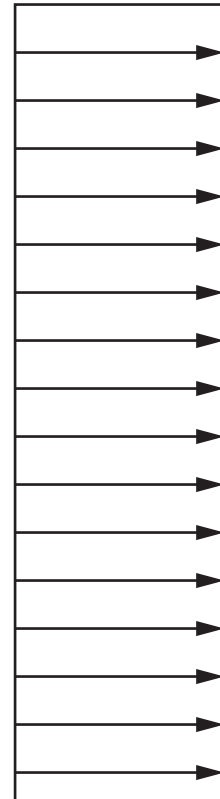


**GROUP
DELTA**

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	DOCUMENT NUMBER 24-0011
	FIGURE NUMBER 9A

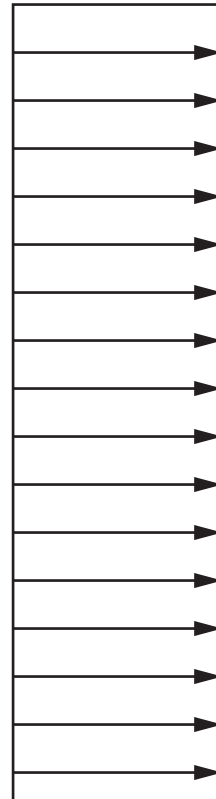
**LATERAL EARTH PRESSURES
FOR CANTILEVER SHORING**

TRAFFIC AND
CONSTRUCTION
SURCHARGE



$$P_s = 0.3q$$

EARTH
PRESSURE



$$P_A$$

NOTES:

1. ACTIVE LATERAL EARTH PRESSURE, P_A
-WITH LEVEL BACKFILL: $P_A = 21H$
-WITH 2:1 SLOPING BACKFILL: $P_A = 35H$
2. ASSUMES NO HYDROSTATIC PRESSURE.
3. VALUES ASSUME SHORED MATERIAL IS GRANULAR ON-SITE SOIL AS DESCRIBED IN THE REPORT OF GEOTECHNICAL INVESTIGATION.
4. H IS MEASURED IN FEET.
5. FIGURE SHOULD BE USED WITH GEOTECHNICAL REPORT.
6. FOR PRELIMINARY DESIGN.
7. SURCHARGES FROM CONSTRUCTION EQUIPMENT, EXCAVATED SOIL, CONSTRUCTION MATERIALS, TRAFFIC LOADING OR OTHER UNIFORM LOADING (q) ABOVE THE WALL SHOULD BE CALCULATED USING THE SURCHARGE LATERAL EARTH PRESSURE, P_s . POINT LOADS OR OTHER SURCHARGES CAN BE EVALUATED UPON REQUEST.

6 INCH
MINIMUM DIAMETER

GROUND SURFACE

ASSUMED
SLIP PLANE

SOLDIER
PILE

15° MINIMUM

MINIMUM UNBONDED
LENGTH: 15 FEET

AFTER TESTING
FILL THIS PORTION
WITH GROUT

H

TIEBACK
ANCHORS

ANCHORS ASSUMED
EFFECTIVE BEHIND
THE SLIP PLANE ONLY

ULTIMATE ANCHOR RESISTANCE
(PRESSURE GROUTED)
= 10 TO 15 PSI

~28°

5 FEET

f_{TOE}

1 FOOT

ALLOWABLE PASSIVE
SOIL RESISTANCE
= 300 PCF

GROUND SURFACE

ALLOWABLE SOIL FRICTION (f_{TOE}) = 700 PSF
(ASSUMES CONCRETE FOOTING EMBEDDED
IN COMPETENT MATERIAL)

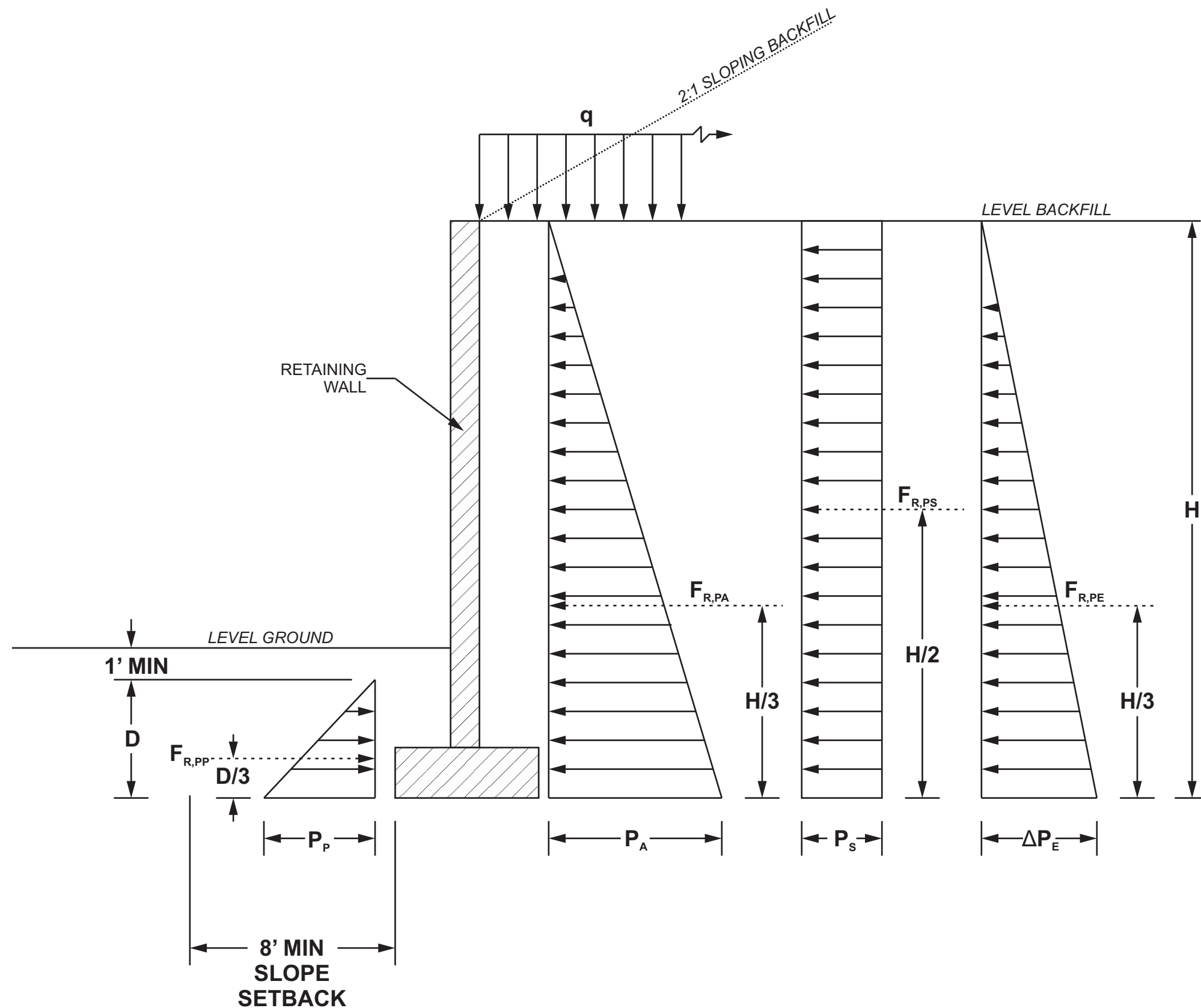
TOTAL ALLOWABLE PASSIVE RESISTANCE
PER SOLDIER PILE =
UNIT ALLOWABLE PASSIVE RESISTANCE
X
TWICE THE CONCRETED
SOLDIER PILE WIDTH



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HCA Design and Construction

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FIGURE NUMBER
9B

LATERAL EARTH PRESSURES
FOR BRACED OR ANCHORED SHORING



NOTES:

- PASSIVE PRESSURES MAY BE INCREASED BY $\frac{1}{3}$ DURING SEISMIC LOADING. THE UPPER 12 INCHES OF MATERIAL NOT PROTECTED BY CONCRETE SLABS OR PAVEMENTS SHOULD NOT BE INCLUDED IN THE ESTIMATION OF PASSIVE RESISTANCE.
- ASSUMES NO HYDROSTATIC PRESSURE. A WALL BACK DRAIN SHOULD BE INSTALLED AS RECOMMENDED IN THE *WALL DRAINAGE DETAIL* FIGURE.
- SURCHARGES FROM CONSTRUCTION EQUIPMENT, EXCAVATED SOIL, TRAFFIC LOADING OR OTHER UNIFORM LOADING ABOVE THE WALL SHOULD BE CALCULATED USING THE SURCHARGE LATERAL EARTH PRESSURE, P_s . POINT LOADS OR OTHER SURCHARGES CAN BE EVALUATED UPON REQUEST.
- SEISMIC INCREMENT LATERAL EARTH PRESSURE (ΔP_E) IS BASED ON A DESIGN-LEVEL PEAK GROUND ACCELERATION OF 0.41g. SEISMIC INCREMENT SHOULD BE APPLIED TO WALLS SIX FEET OR GREATER IN HEIGHT.
- 'H' AND 'D' ARE MEASURED IN FEET.
- PRESSURES ASSUME GRANULAR AND NON-EXPANSIVE SOIL MATERIALS COMPACTED AS RECOMMENDED IN THE GEOTECHNICAL REPORT.

LATERAL EARTH PRESSURES

LATERAL EARTH PRESSURE TYPE	EQUIVALENT FLUID PRESSURE (PCF)	
ACTIVE, P_A	LEVEL BACKFILL	2:1 SLOPING BACKFILL
COMPACTED FILL	35	55
SEISMIC INCREMENT, ΔP_E^*	23	
PASSIVE, P_P^{**}	300	
SURCHARGE, P_s	0.3q	

*SEISMIC PRESSURE, $P_{AE} = P_A + \Delta P_E$

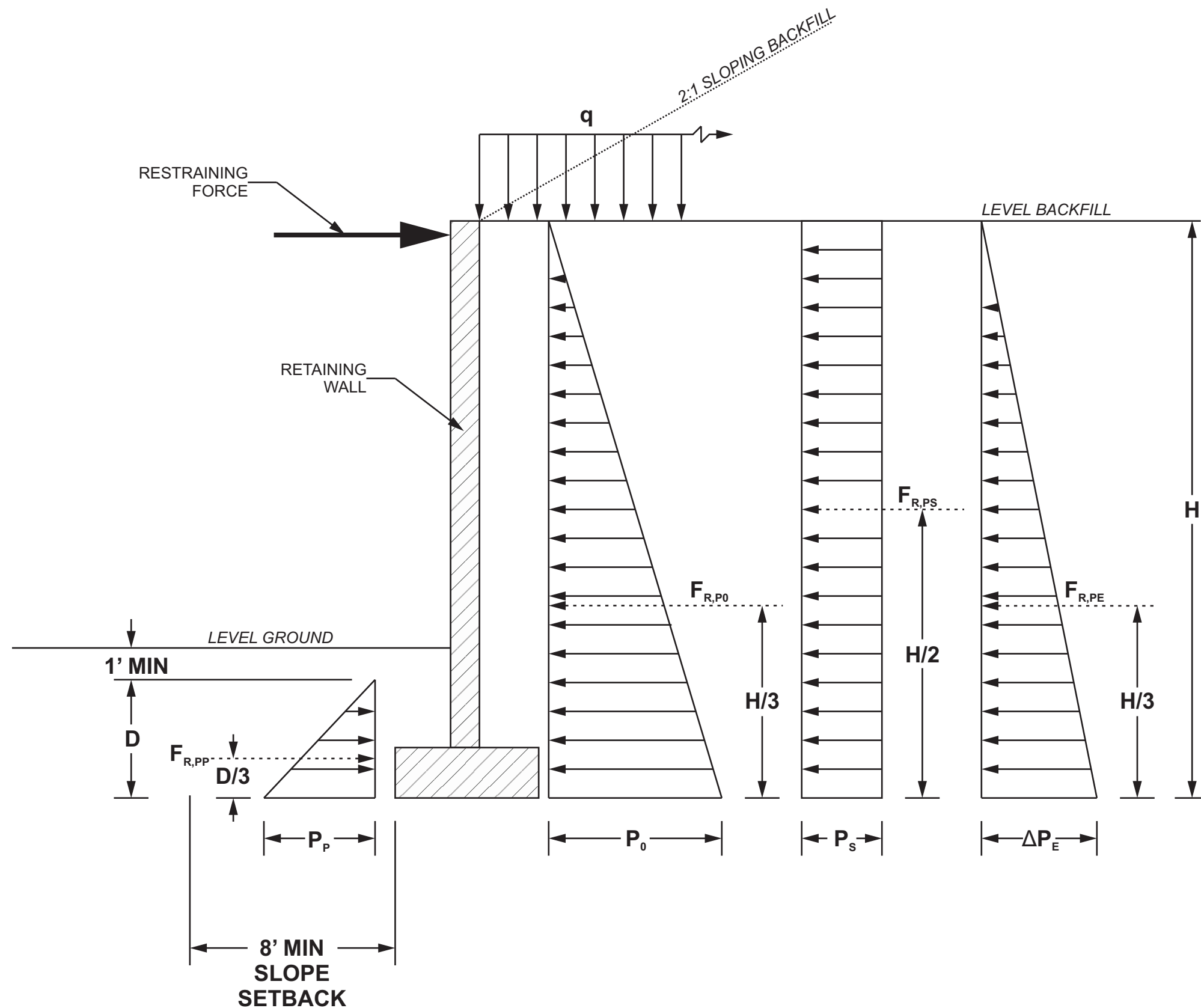
**PASSIVE RESISTANCE VERSUS DISPLACEMENT CURVES CAN BE PROVIDED UPON REQUEST.



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HCA Design and Construction

PROJECT NUMBER
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DOCUMENT NUMBER
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FIGURE NUMBER
9C

**LATERAL EARTH PRESSURES
FOR YIELDING RETAINING WALLS**



NOTES:

- PASSIVE PRESSURES MAY BE INCREASED BY $\frac{1}{3}$ DURING SEISMIC LOADING. THE UPPER 12 INCHES OF MATERIAL NOT PROTECTED BY CONCRETE SLABS OR PAVEMENTS SHOULD NOT BE INCLUDED IN THE ESTIMATION OF PASSIVE RESISTANCE.
- ASSUMES NO HYDROSTATIC PRESSURE. A WALL BACK DRAIN SHOULD BE INSTALLED AS RECOMMENDED IN THE *WALL DRAINAGE DETAIL* FIGURE.
- SURCHARGES FROM CONSTRUCTION EQUIPMENT, EXCAVATED SOIL, TRAFFIC LOADING OR OTHER UNIFORM LOADING ABOVE THE WALL SHOULD BE CALCULATED USING THE SURCHARGE LATERAL EARTH PRESSURE, P_s . POINT LOADS OR OTHER SURCHARGES CAN BE EVALUATED UPON REQUEST.
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- 'H' AND 'D' ARE MEASURED IN FEET.
- PRESSURES ASSUME GRANULAR AND NON-EXPANSIVE SOIL MATERIALS COMPACTED AS RECOMMENDED IN THE GEOTECHNICAL REPORT.

LATERAL EARTH PRESSURES

LATERAL EARTH PRESSURE TYPE	EQUIVALENT FLUID PRESSURE (PCF)	
AT-REST, P_0	LEVEL BACKFILL	2:1 SLOPING BACKFILL
COMPACTED FILL	60	90
SEISMIC INCREMENT, ΔP_E^*	(SEE FIGURE 9C)	
PASSIVE, P_p^{**}	300	
SURCHARGE, P_s	0.5q	

*SEISMIC PRESSURE, $P_{AE} = P_A + \Delta P_E$ (SEE FIGURE 9C)

**PASSIVE RESISTANCE VERSUS DISPLACEMENT CURVES CAN BE PROVIDED UPON REQUEST.

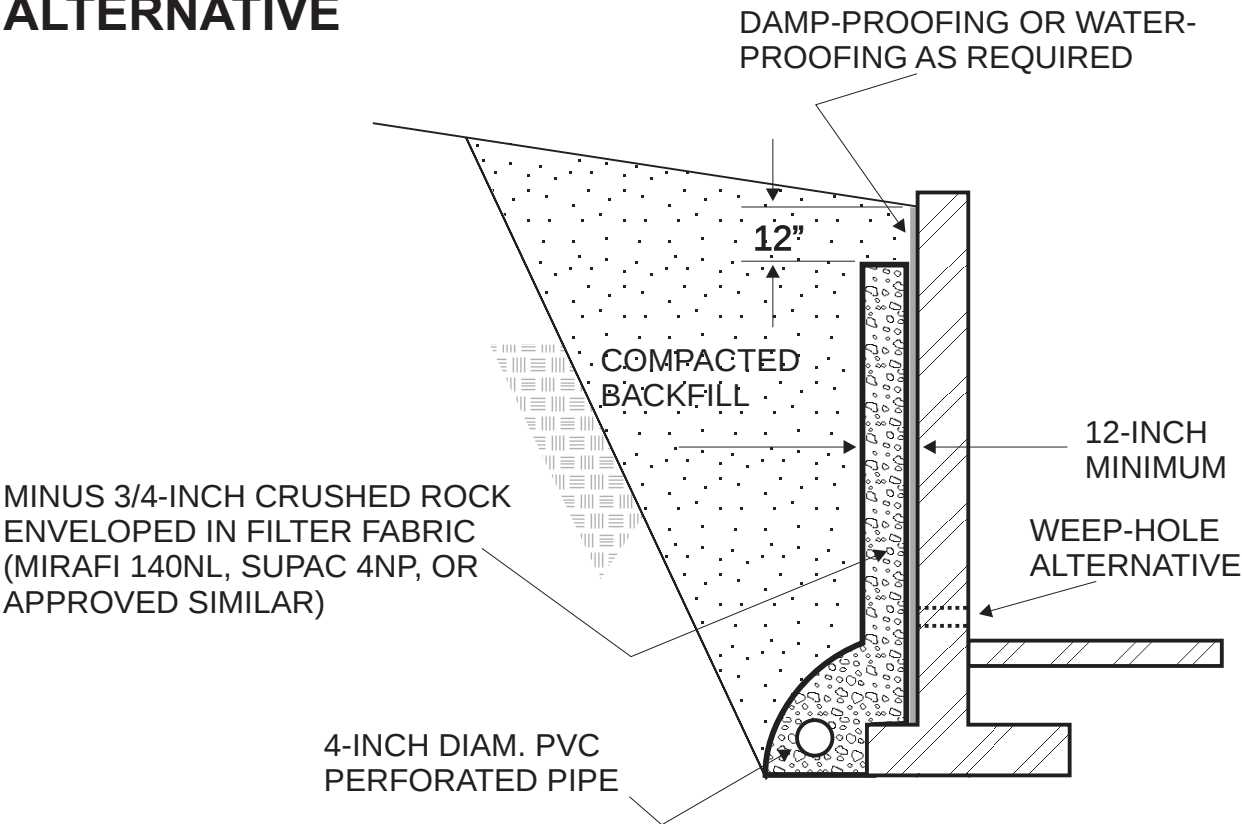


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ENGINEERS AND GEOLOGISTS
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SAN DIEGO, CA 92126 (858) 536-1000
PROJECT NAME
Riverside Community Hospital
HCA Design and Construction

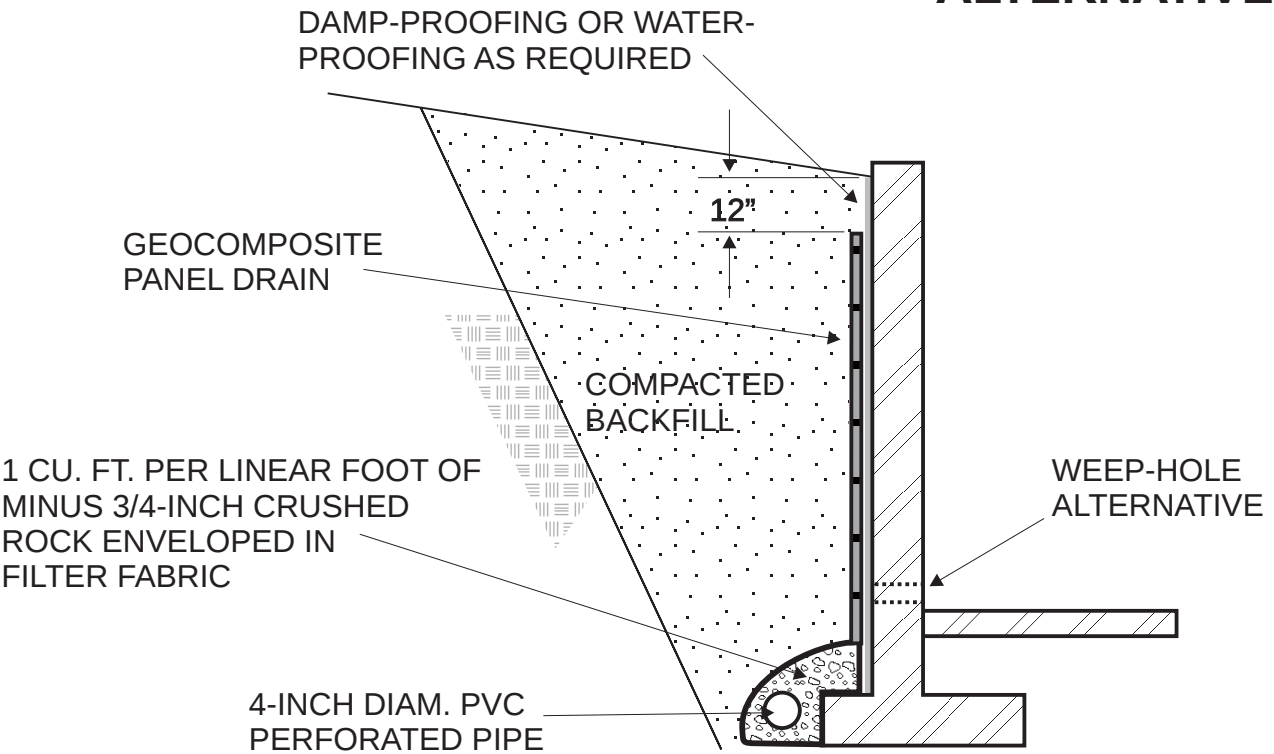
PROJECT NUMBER
SD809
DOCUMENT NUMBER
24-0011
FIGURE NUMBER
9D

**LATERAL EARTH PRESSURES
FOR BRACED RETAINING WALLS**

ROCK AND FABRIC
ALTERNATIVE



PANEL DRAIN
ALTERNATIVE



NOTES

- 1) Perforated pipe should outlet through a solid pipe to a free gravity outfall. Perforated pipe and outlet pipe should have a fall of at least 1%.
- 2) As an alternative to the perforated pipe and outlet, weep-holes may be constructed. Weep-holes should be at least 2 inches in diameter, spaced no greater than 8 feet, and be located just above grade at the bottom of wall.
- 3) Filter fabric should consist of Mirafi 140N, Supac 5NP, Amoco 4599, or similar approved fabric. Filter fabric should be overlapped at least 6-inches.
- 4) Geocomposite panel drain should consist of Miradrain 6000, J-DRain 400, Supac DS-15, or approved similar product.

	GROUP DELTA CONSULTANTS, INC. ENGINEERS AND GEOLOGISTS 9245 ACTIVITY ROAD, SUITE 103 SAN DIEGO, CA 92126 (858) 536-1000	PROJECT NUMBER SD809
	PROJECT NAME Riverside Community Hospital HCA Design and Construction	DOCUMENT NUMBER 24-0011
		FIGURE NUMBER 9E
WALL DRAINAGE DETAILS		

APPENDIX A
FIELD EXPLORATION

APPENDIX A

FIELD EXPLORATION

The field exploration program included a geologic reconnaissance of the site, the advancement of 5 cone penetrometer test (CPT) soundings, the drilling of 10 exploratory borings, and borehole percolation testing at 12 locations. The subsurface explorations and field infiltration tests were completed between March 22nd and April 5th, 2024. The CPT soundings were advanced by Kehoe Testing and Engineering. The exploratory borings were advanced by Yellow Jacket Drilling. The infiltration tests were completed by Group Delta personnel using a hand auger. The exploration locations are shown on the Exploration Plans, Figures 3A to 3C. The CPT data and interpreted soil profiles are presented in Figures A-1 to A-5. Boring Records are provided in Figures A-6 to A-15. The field infiltration test results are discussed in the text of this report and presented in detail in the figures of Appendix D. The subsurface explorations are summarized in the table below.

Exploration ID	Exploration Date	Latitude	Longitude	Ground Surface Elevation [FT]	Exploration Depth [FT]	Figure No.
CPT-1	03/22/24	33.977986°	-117.382803°	800	34.0	A-1
CPT-2	03/22/24	33.977770°	-117.383393°	796	37.5	A-2
CPT-3	03/22/24	33.977144°	-117.383833°	793	20.7	A-3
CPT-4	03/22/24	33.976485°	-117.383034°	794	25.0	A-4
CPT-5	03/22/24	33.976042°	-117.381919°	839	54.7	A-5

B-01	04/04/24	33.978130°	-117.383124°	800	36½	A-6
B-02	04/01/24	33.977907°	-117.383271°	798	51½	A-7
B-03	04/02/24	33.977754°	-117.382979°	798	36½	A-8
B-04	04/04/24	33.977536°	-117.383915°	794	31	A-9
B-05	04/04/24	33.977440°	-117.383294°	796	31½	A-10
B-06	04/03/24	33.977308°	-117.384145°	794	31½	A-11
B-07	04/04/24	33.976992°	-117.383266°	794	28	A-12
B-08	04/01/24	33.976503°	-117.383073°	794	30	A-13
B-09	04/05/24	33.976063°	-117.382178°	832	31½	A-14
B-10	04/02/24	33.976190°	-117.381820°	838	51½	A-15

The 10 CPT soundings were advanced by Kehoe Testing and Engineering in general accordance with ASTM D5778 using a 30-ton truck mounted rig with a 15 cm² cone. Integrated electronic circuitry was used to measure the tip resistance (Qc) and skin friction (Fs) at one-inch intervals while the CPT was advanced into the soil using hydraulic down pressure. Note that each of the CPT soundings was pushed to practical refusal, as indicated by a CPT tip resistance in excess of 700 tons per square foot (TSF). A piezometer located behind the cone tip also measured transient pore pressure (u). The CPT data was used to characterize the soil profile are based on normalized cone resistance and friction ratio interpretations (Robertson, 2010). The first figure for each CPT sounding presents both the raw CPT data and the interpreted soil profile (Figures A-1a to A-5a). The raw CPT data is also shown in more detail in the following Figures A-1b to A-5b for each CPT sounding.



APPENDIX A

FIELD EXPLORATION (Continued)

At the location of soundings CPT-1, CPT-2 and CPT-5, shear wave velocity measurements were collected at 5-foot depth intervals using an air actuated hammer located inside the front jack of the rig. The interval shear wave data measured in each of these three CPT soundings is attached immediately after the interpreted soil parameters. The average shear wave velocity measured within the upper 34 to 35 feet (V_{sd}) at the location of CPT-1 and CPT-2 was 876 to 899 ft/s. If we assume a uniform shear wave velocity of 1,940 ft/s for the Old Alluvium below that depth based on the lower bound of the shear wave velocity measurements conducted by others, the average shear wave velocity for the upper 100 feet of the soil profile (V_{s30}) for the Garage site would be about 1,390 ft/s (CHJ, 2007). This correlates to a 2022 CBC Site Class C for the proposed Garage site.

The average shear wave velocity measured within the upper 55 feet (V_{sd}) of the compacted fill and Old Alluvium along the east end of Tower Site in CPT-5 was 1,390 ft/s. If we again assume a shear wave velocity of 1,940 ft/s for the Old Alluvium below that depth, the average shear wave velocity for the upper 100 feet of the soil profile (V_{s30}) at the east end of the Tower site is estimated at 1,600 ft/s (CHJ, 2007). This also corresponds to a 2022 CBC Site Class C for the proposed Tower site.

The exploratory borings were advanced by Yellow Jacket Drilling using their CME 75 limited access track mounted rig, and their CME 85 truck mounted rig. Disturbed soil samples were collected from the exploratory borings using a 2-inch outside diameter Standard Penetration Test (SPT) sampler. Less disturbed samples were collected using a 3-inch outside diameter ring lined sampler (a modified California sampler). Automatic hammers with calibrated Energy Transfer Ratios (ETR) ranging from approximately 74 to 80 percent were used to collect all of the drive samples. For each sample, the number of blows needed to drive the sampler 12 inches was recorded on the logs. The field blow counts (N) were normalized to approximate a standard 60 percent ETR as shown on the logs (N_{60}). Bulk samples were also collected from the explorations at selected intervals.

The field exploration locations were determined by visually estimating, pacing and taping distances from landmarks shown on the Exploration Plans, Figures 3A to 3C. The locations shown should not be considered more accurate than is implied by the method of measurement used and the scale of the map. The lines designating the interface between differing soil materials on the logs may be abrupt or gradational. Further, soil conditions at locations between the excavations may be substantially different from those at the specific locations we explored. It should be noted that the passage of time may also result in changes in the soil conditions reported in the logs.

Logs for several previous field explorations completed by others are also included in Appendix A1 for reference. These logs include Borings B-1, B-2, B-6, B-7, B-8 and B-9 as well as CPT-1 to CPT-6 at the proposed Tower site (CHJ, 2008). Borings B-1 and B-2 are also included from a previous investigation for the MOB located along the southern edge of the Garage site (CHJ, 2012). The approximate locations of the relevant previous explorations are shown in Figures 3B and 3C.



SOIL IDENTIFICATION AND DESCRIPTION SEQUENCE

Sequence	Identification Components	Refer to Section		Required	Optional
		Field	Lab		
1	Group Name	2.5.2	3.2.2	●	○
2	Group Symbol	2.5.2	3.2.2	●	
	Description Components				
3	Consistency of Cohesive Soil	2.5.3	3.2.3	●	
4	Apparent Density of Cohesionless Soil	2.5.4		●	
5	Color	2.5.5		●	
6	Moisture	2.5.6		●	
	Percent of Proportion of Soil	2.5.7	3.2.4	●	●
7	Particle Size	2.5.8	2.5.8	●	●
	Particle Angularity	2.5.9			○
	Particle Shape	2.5.10			○
8	Plasticity (for fine-grained soil)	2.5.11	3.2.5		○
9	Dry Strength (for fine-grained soil)	2.5.12			○
10	Dilatancy (for fine-grained soil)	2.5.13			○
11	Toughness (for fine-grained soil)	2.5.14			○
12	Structure	2.5.15			○
13	Cementation	2.5.16		●	
14	Percent of Cobbles and Boulders	2.5.17		●	
	Description of Cobbles and Boulders	2.5.18		●	
15	Consistency Field Test Result	2.5.3		●	
16	Additional Comments	2.5.19			○

Minimum Required Sequence:

USCS Group Name (Group Symbol); Consistency or Density; Color; Moisture; Percent of Proportion of Soil; Particle Size; Plasticity (optional).

● = optional for non-Caltrans projects

Where applicable:

Cementation; % cobbles & boulders;
Description of cobbles & boulders;
Consistency field test result

EXPLORATION IDENTIFICATION

Explorations are identified using the following convention

H-YY-NNN

Where:

H: Exploration type code

YY: 2-digit year (where utilized)

NNN: Exploration number

Hole Type Code and Description

Hole Type Code	Description
A	Auger Boring (Hollow or solid stem bucket)
BA	Bucket Auger
CPT	Cone Penetration Test
D	Driven (dynamic cone penetrometer)
HA	Hand Auger
HD	Hand driven (1-inch soil tube)
O	Other (note on LOTB)
P	Rotary Percussion Boring (Air)
R	Rotary drilled boring (Conventional)
RC	Rotary core (self-cased wire-line, continuously sampled)
RW	Rotary cored (self cased wire-line, not continuously sampled)
TP	Test Pit

Description Sequence Examples:

SANDY lean CLAY (CL); very stiff; yellowish brown; moist; mostly fines; some SAND, from fine to medium; few gravels; medium plasticity; PP=2,75.

Well-graded SAND with SILT and GRAVEL and COBBLES (SW-SM); dense; brown; moist; mostly SAND, from fine to coarse; some fine GRAVEL; few fines; weak cementation; 10% GRANITE COBBLES; 3 to 6 inches; hard, subrounded.

Clayey SAND (SC) medium dense, light brown; wet; mostly fine sand; little fines; low plasticity.



GROUP DELTA

EXPLORATION RECORD
LEGEND #1

REFERENCE: Caltrans Soil and Rock Logging, Classification, and Presentation Manual (2010).

GROUP SYMBOLS AND NAMES							
Graphic/Symbol		Group Names		Graphic/Symbol		Group Names	
	GW	Well-graded GRAVEL Well-graded GRAVEL with SAND			CL	Lean CLAY Lean CLAY with SAND Lean CLAY with GRAVEL SANDY lean CLAY SANDY lean CLAY with GRAVEL GRAVELLY lean CLAY GRAVELLY lean CLAY with SAND	
	GP	Poorly graded GRAVEL Poorly graded GRAVEL with SAND					
	GW-GM	Well-graded GRAVEL with SILT Well-graded GRAVEL with SILT and SAND			CL-ML	SILTY CLAY SILTY CLAY with SAND SILTY CLAY with GRAVEL SANDY SILTY CLAY SANDY SILTY CLAY with GRAVEL GRAVELLY SILTY CLAY GRAVELLY SILTY CLAY with SAND	
	GW-GC	Well-graded GRAVEL with CLAY (or SILTY CLAY) Well-graded GRAVEL with CLAY and SAND (or SILTY CLAY and SAND)					
	GP-GM	Poorly graded GRAVEL with SILT Poorly graded GRAVEL with SILT and SAND			ML	SILT SILT with SAND SILT with GRAVEL SANDY SILT SANDY SILT with GRAVEL GRAVELLY SILT GRAVELLY SILT with SAND	
	GP-GC	Poorly graded GRAVEL with CLAY (or SILTY CLAY) Poorly graded GRAVEL with CLAY and SAND (or SILTY CLAY and SAND)					
	GM	SILTY GRAVEL SILTY GRAVEL with SAND			OL	ORGANIC lean CLAY ORGANIC lean CLAY with SAND ORGANIC lean CLAY with GRAVEL SANDY ORGANIC lean CLAY SANDY ORGANIC lean CLAY with GRAVEL GRAVELLY ORGANIC lean CLAY GRAVELLY ORGANIC lean CLAY with SAND	
	GC	CLAYEY GRAVEL CLAYEY GRAVEL with SAND					
	GC-GM	SILTY, CLAYEY GRAVEL SILTY, CLAYEY GRAVEL with SAND			OL	ORGANIC SILT ORGANIC SILT with SAND ORGANIC SILT with GRAVEL SANDY ORGANIC SILT SANDY ORGANIC SILT with GRAVEL GRAVELLY ORGANIC SILT GRAVELLY ORGANIC SILT with SAND	
	SW	Well-graded SAND Well-graded SAND with GRAVEL					
	SP	Poorly graded SAND Poorly graded SAND with GRAVEL			CH	Fat CLAY Fat CLAY with SAND Fat CLAY with GRAVEL SANDY fat CLAY SANDY fat CLAY with GRAVEL GRAVELLY fat CLAY GRAVELLY fat CLAY with SAND	
	SW-SM	Well-graded SAND with SILT Well-graded SAND with SILT and GRAVEL					
	SW-SC	Well-graded SAND with CLAY (or SILTY CLAY) Well-graded SAND with CLAY and GRAVEL (or SILTY CLAY and GRAVEL)			MH	Elastic SILT Elastic SILT with SAND Elastic SILT with GRAVEL SANDY elastic SILT SANDY elastic SILT with GRAVEL GRAVELLY elastic SILT GRAVELLY elastic SILT with SAND	
	SP-SM	Poorly graded SAND with SILT Poorly graded SAND with SILT and GRAVEL					
	SP-SC	Poorly graded SAND with CLAY (or SILTY CLAY) Poorly graded SAND with CLAY and GRAVEL (or SILTY CLAY and GRAVEL)			OH	ORGANIC fat CLAY ORGANIC fat CLAY with SAND ORGANIC fat CLAY with GRAVEL SANDY ORGANIC fat CLAY SANDY ORGANIC fat CLAY with GRAVEL GRAVELLY ORGANIC fat CLAY GRAVELLY ORGANIC fat CLAY with SAND	
	SM	SILTY SAND SILTY SAND with GRAVEL					
	SC	CLAYEY SAND CLAYEY SAND with GRAVEL			OH	ORGANIC elastic SILT ORGANIC elastic SILT with SAND ORGANIC elastic SILT with GRAVEL SANDY elastic ELASTIC SILT SANDY ORGANIC elastic SILT with GRAVEL GRAVELLY ORGANIC elastic SILT GRAVELLY ORGANIC elastic SILT with SAND	
	SC-SM	SILTY, CLAYEY SAND SILTY, CLAYEY SAND with GRAVEL					
	PT	PEAT			OL/OH	ORGANIC SOIL ORGANIC SOIL with SAND ORGANIC SOIL with GRAVEL SANDY ORGANIC SOIL SANDY ORGANIC SOIL with GRAVEL GRAVELLY ORGANIC SOIL GRAVELLY ORGANIC SOIL with SAND	
		COBBLES COBBLES AND BOULDERS BOULDERS					

FIELD AND LABORATORY TESTING	
C	Consolidation (ASTM D 2435)
CL	Collapse Potential (ASTM D 4546)
CP	Compaction Curve (CTM 216)
CR	Corrosion, Sulfates, Chlorides (CTM 643; CTM 417; CTM 422)
CU	Consolidated Undrained Triaxial (ASTM D 4767)
DS	Direct Shear (ASTM D 3080)
EI	Expansion Index (ASTM D 4829)
M	Moisture Content (ASTM D 2216)
OC	Organic Content (ASTM D 2974)
P	Permeability (CTM 220)
PA	Particle Size Analysis (ASTM D 6913, ASTM D 7928)
PI	Liquid Limit, Plastic Limit, Plasticity Index (AASHTO T 89, AASHTO T 90)
PL	Point Load Index (ASTM D 5731)
PM	Pressure Meter
R	R-Value (CTM 301)
SE	Sand Equivalent (CTM 217)
SG	Specific Gravity (AASHTO T 100)
SL	Shrinkage Limit (ASTM D 427)
SW	Swell Potential (ASTM D 4546)
UC	Unconfined Compression - Soil (ASTM D 2166)
UU	Unconfined Compression - Rock (ASTM D 2938)
UW	Unit Weight (ASTM D 2937)
WA	Percent passing the No. 200 Sieve (ASTM D 1140)

SAMPLER GRAPHIC SYMBOLS	
	Standard Penetration Test (SPT)
	Modified California Sampler (2.4" ID, 3" OD)
	Shelby Tube
	Piston Sampler
	NX Rock Core
	HQ Rock Core
	Bulk Sampler
	Other (see remarks)

DRILLING METHOD SYMBOLS			
	Auger Drilling		Rotary Drilling
	Dynamic Cone or Hand Driven		Diamond Core

WATER LEVEL SYMBOLS	
	Static Water Level Reading

Term	Definition	Symbol
Unit Change	Change in geologic unit	_____
Material Change Within Unit	Change of soil classification within geologic unit	-----

REFERENCE: Caltrans Soil and Rock Logging, Classification, and Presentation Manual (2010).

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CONSISTENCY OF COHESIVE SOILS				
Description	Shear Strength (tsf)	Pocket Penetrometer , PP Measurement (tsf)	Torvane, TV, Measurement (tsf)	Vane Shear, VS, Measurement (tsf)
Very Soft	Less than 0.12	Less than 0.25	Less than 0.12	Less than 0.12
Soft	0.12 - 0.25	0.25 - 0.5	0.12 - 0.25	0.12 - 0.25
Medium Stiff	0.25 - 0.5	0.5 - 1	0.25 - 0.5	0.25 - 0.5
Stiff	0.5 - 1	1 - 2	0.5 - 1	0.5 - 1
Very Stiff	1 - 2	2 - 4	1 - 2	1 - 2
Hard	Greater than 2	Greater than 4	Greater than 2	Greater than 2

APPARENT DENSITY OF COHESIONLESS SOILS	
Description	SPT N ₆₀ (blows / 12 inches)
Very Loose	0 - 4
Loose	5 - 9
Medium Dense	10 - 29
Dense	30 -50
Very Dense	Greater than 50

PERCENT OR PROPORTION OF SOILS	
Description	Criteria
Trace	Particles are present but estimated to be less than 5%
Few	5 - 10%
Little	15 - 25%
Some	30 - 45%
Mostly	50 - 100%

CEMENTATION	
Description	Criteria
Weak	Crumbles or breaks with handling or little finger pressure
Moderate	Crumbles or breaks with considerable finger pressure
Strong	Will not crumble or break with finger pressure

MOISTURE	
Description	Criteria
Dry	No discernable moisture
Moist	Moisture present but no free water
Wet	Visible free water

CONSISTENCY OF COHESIVE SOILS*	
Description	SPT N ₆₀ (blows / 12 inches)
Very Soft	0 - 1
Soft	2 - 3
Medium Stiff	4 - 7
Stiff	8 - 14
Very Stiff	15 - 30
Hard	Greater than 30

* Ref: Peck, Hansen, and Thornburn, 1974, "Foundation Engineering," Second Edition.

Note: Only to be used (with caution) when pocket penetrometer or other data on undrained shear strength are unavailable.
 Not allowed by Caltrans Soil and Rock Logging and Classification Manual, 2010.

PARTICLE SIZE		
Description		Size (in)
Boulder		Greater than 12
Cobble		3 - 12
Gravel	Coarse	3/4 - 3
	Fine	1/5 - 3/4
Sand	Coarse	1/16 - 1/5
	Medium	1/64 - 1/16
	Fine	1/300 - 1/64
Silt and Clay		Less than 1/300

PLASTICITY	
Description	Criteria
Nonplastic	A 1/8-in. thread cannot be rolled at any water content
Low	The thread can barely be rolled and the lump cannot be formed when drier than the plastic limit
Medium	The thread is easy to roll and not much time is required to reach the plastic limit. The thread cannot be rerolled after reaching the plastic limit. The lump crumbles when drier than the plastic limit
High	It takes considerable time rolling and kneading to reach the plastic limit. The thread can be rerolled several times after reaching the plastic limit. The lump can be formed without crumbling when drier than the plastic limit.

REFERENCE: Caltrans Soil and Rock Logging, Classification, and Presentation Manual (2010), with the exception of consistency of cohesive soils vs. N₆₀.


GROUP DELTA

EXPLORATION RECORD
LEGEND #3

LEGEND OF ROCK MATERIALS	
	Igneous Rock
	Sedimentary Rock
	Metamorphic Rock

BEDDING SPACING	
Description	Thickness / Spacing
Massive	Greater than 10ft
Very Thickly Bedded	3 ft - 10 ft
Thickly Bedded	1 ft - 3 ft
Moderately Bedded	4 in - 1 ft
Thinly Bedded	1 in - 4 in
Very Thinly Bedded	1/4 in - 1 in
Laminated	Less than 1/4 in

WEATHERING DESCRIPTORS FOR INTACT ROCK						
Diagnostic Features						
Description	Chemical Weathering - Discoloration - Oxidation		Mechanical Weathering and Grain Boundary Conditions	Texture and Leaching		General Characteristics
	Body of Rock	Fracture Surfaces		Texture	Leaching	
Fresh	No discoloration, no oxidized	No discoloration or oxidation	No separation, intact (tight)	No change	No leaching	Hammer rings when crystalline rocks are struck
Slightly Weathered	Discoloration or oxidation is limited to surface of, or short distance from, fractures; some feldspar crystals are dull	Minor to complete discoloration or oxidation of most surfaces	No visible separation, intact (tight)	Preserved	Minor leaching of some soluble minerals	Hammer does not ring when rock is struck. Body of rock not weakened
Moderately Weathered	Discoloration or oxidation extends from fractures usually throughout; Fe-Mg minerals are "rusty"; feldspar crystals are "cloudy"	All fracture surfaces are discolored or oxidized	Partial separation of boundaries visible	Generally Preserved	Soluble minerals may be mostly leached	Hammer does not ring when rock is struck. Body of rock is slightly weakened
Intensely Weathered	Discoloration or oxidation throughout; all feldspars and Fe-Mg minerals are altered to clay to some extent; or chemical alteration produces in situ disaggregation, grain boundary conditions	All fracture surfaces are discolored or oxidized; surfaces friable	Partial separation, rock is friable; in semi-arid conditions, granitics are disaggregated	Texture altered by chemical disintegration (hydration, argillation)	Leaching of soluble minerals may be complete	Dull sound when struck with hammer; usually can be broken with moderate to heavy manual pressure or by light hammer blow without reference to planes of weakness such as incipient or hairline fractures of veinlets. Rock is significantly weakened.
Decomposed	Discolored or oxidized throughout, but resistant minerals such as quartz may be unaltered; all feldspars and Fe-Mg minerals are completely altered to clay		Complete separation of grain boundaries (disaggregated)	Resembles a soil; partial or complete remnant rock structure may be preserved; leaching of soluble minerals usually complete		Can be granulated by hand. Resistant minerals such as quartz may be present as "stringers" or "dikes"

PERCENT CORE RECOVERY (REC)
$\frac{\sum \text{LENGTH OF THE RECOVERED CORE PIECES (IN.)}}{\text{TOTAL LENGTH OF CORE RUN (IN.)}} \times 100$

PERCENT CORE RECOVERY (REC)
$\frac{\sum \text{LENGTH OF THE INTACT CORE PIECES} \geq 4 \text{ IN.}}{\text{TOTAL LENGTH OF CORE RUN (IN.)}} \times 100$
RQD* INDICATES SOUNDNESS CRITERIA NOT MET

ROCK HARDNESS	
Description	Criteria
Extremely Hard	Cannot be scratched with a pocketknife or sharp pick. Can only be chipped with repeated heavy hammer blows.
Very Hard	Cannot be scratched with a pocketknife or sharp pick. Breaks with repeated heavy hammer blows.
Hard	Can be scratched with a pocketknife or sharp pick with difficulty (heavy pressure). Breaks with heavy hammer blows.
Moderately Hard	Can be scratched with a pocketknife or sharp pick with light or moderate pressure. Breaks with moderate hammer blows.
Moderately Soft	Can be grooved 1/16 in. deep with a pocketknife or sharp pick with moderate or heavy pressure. Breaks with light hammer blow or heavy manual pressure.
Soft	Can be grooved or gouged easily with a pocketknife or sharp pick with light pressure, can be scratched with fingernail. Breaks with light moderate manual pressure.
Very Soft	Can be readily indented, grooved or gouged with fingernail, or carved with a pocketknife. Breaks with light manual pressure.

FRACTURE DENSITY	
Description	Observed Fracture Density
Unfractured	No fractures
Very Slightly Fractured	Core lengths greater than 3 ft
Slightly Fractured	Core lengths mostly from 1 to 3 ft
Moderately Fractured	Core lengths mostly 4 in. to 1 ft.
Intensely Fractured	Core lengths mostly from 1 to 4 in.
Very Intensely Fractured	Mostly chips and fragments.



REFERENCE: Caltrans Soil and Rock Logging, Classification, and Presentation Manual (2010).



GROUP DELTA

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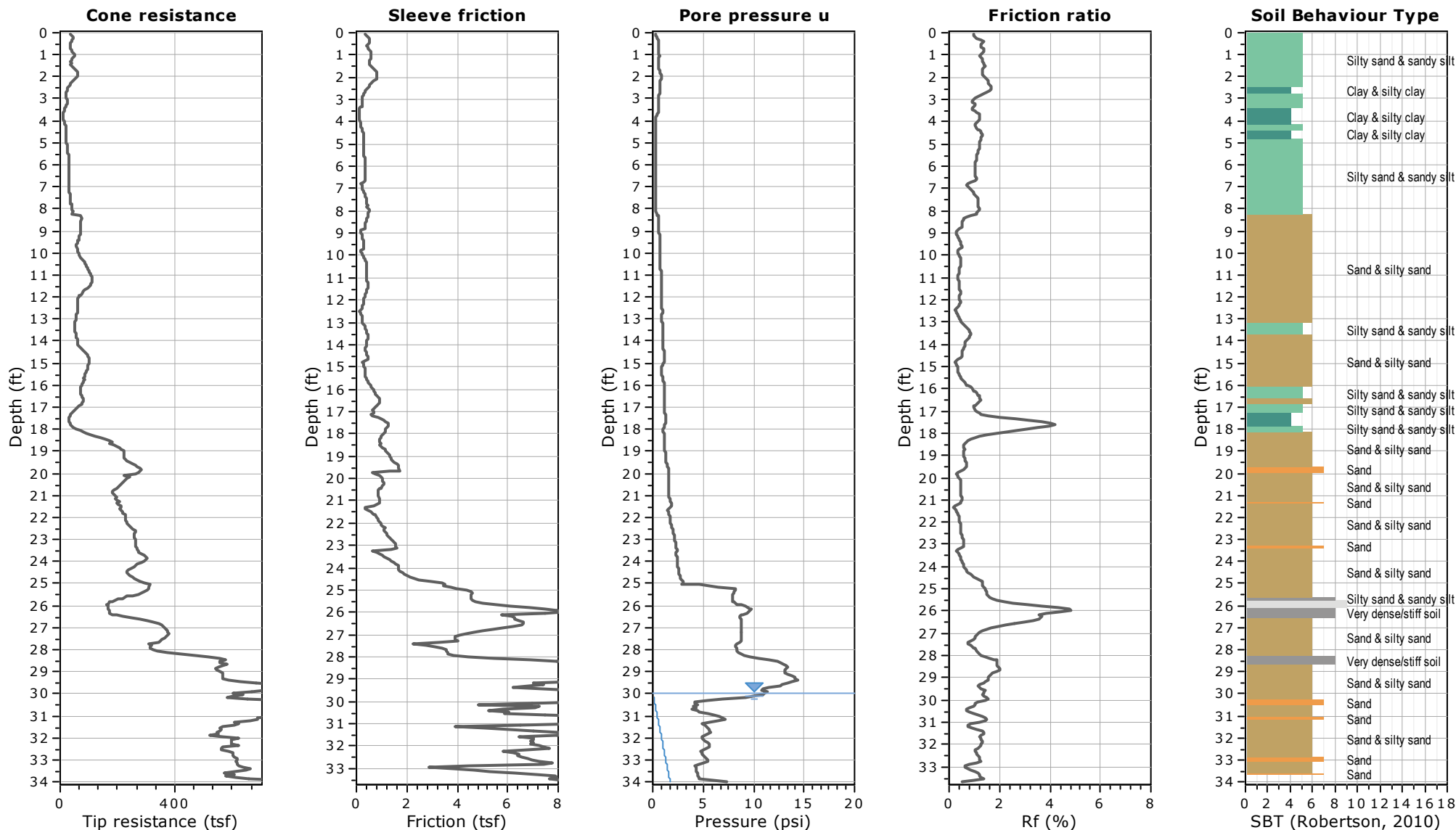
Project: Riverside Community Hospital

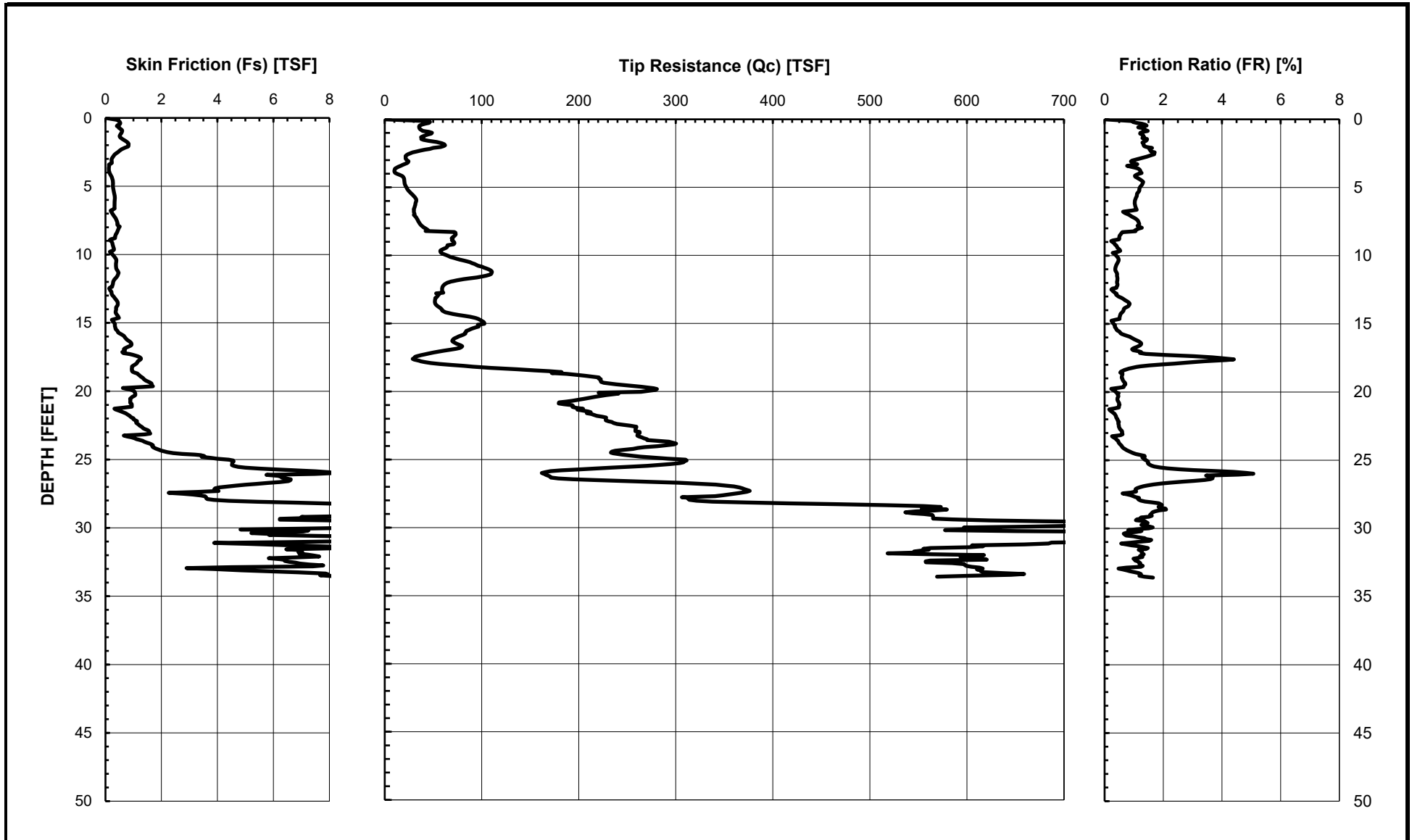
Location: 4468 Brockton Avenue, Riverside, California 92501

CPT-1

Total depth: 33.99 ft, Date: 3/22/2024

Surface Elevation: 800.00 ft





GROUP DELTA

CONE PENETROMETER DATA (CPT-1)

Document No. 24-0011

Project No. SD809

FIGURE A-1b



GROUP DELTA

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San Diego, California 92126
www.GroupDelta.com

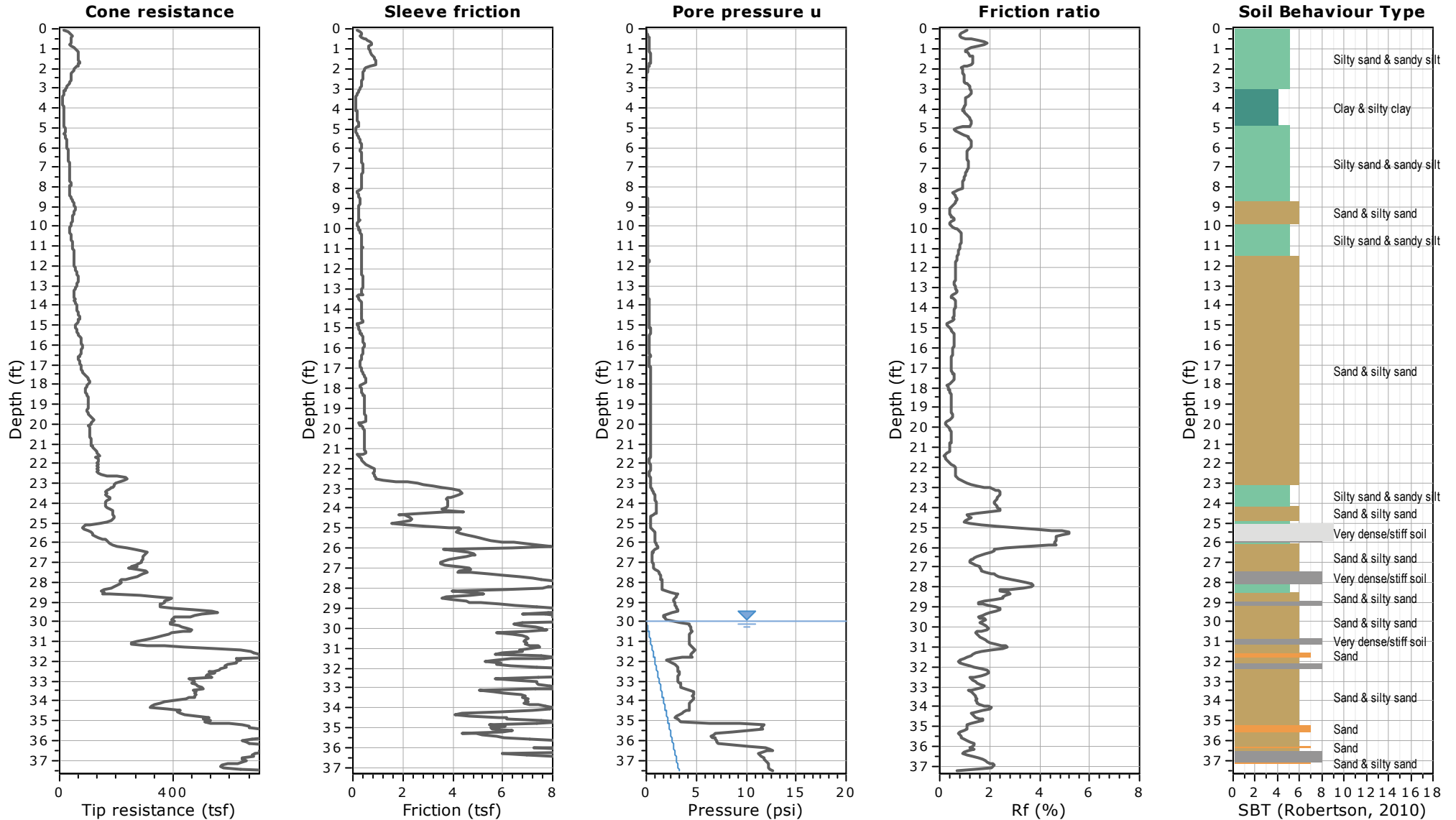
Project: Riverside Community Hospital

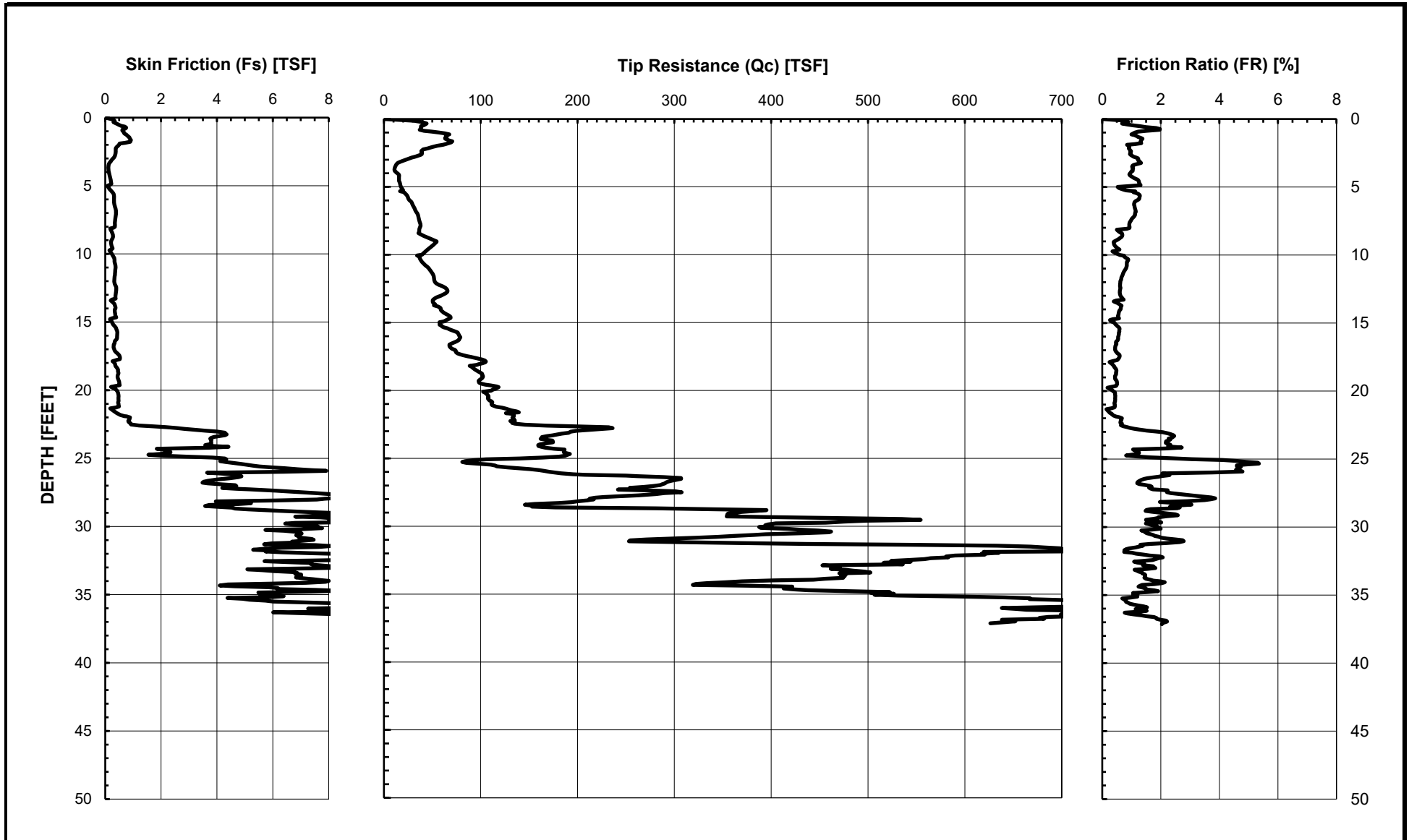
Location: 4468 Brockton Avenue, Riverside, California 92501

CPT-2

Total depth: 37.54 ft, Date: 3/22/2024

Surface Elevation: 796.00 ft





GROUP DELTA

CONE PENETROMETER DATA (CPT-2)

Document No. 24-0011

Project No. SD809

FIGURE A-2b



GROUP DELTA

Group Delta Consultants
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San Diego, California 92126
www.GroupDelta.com

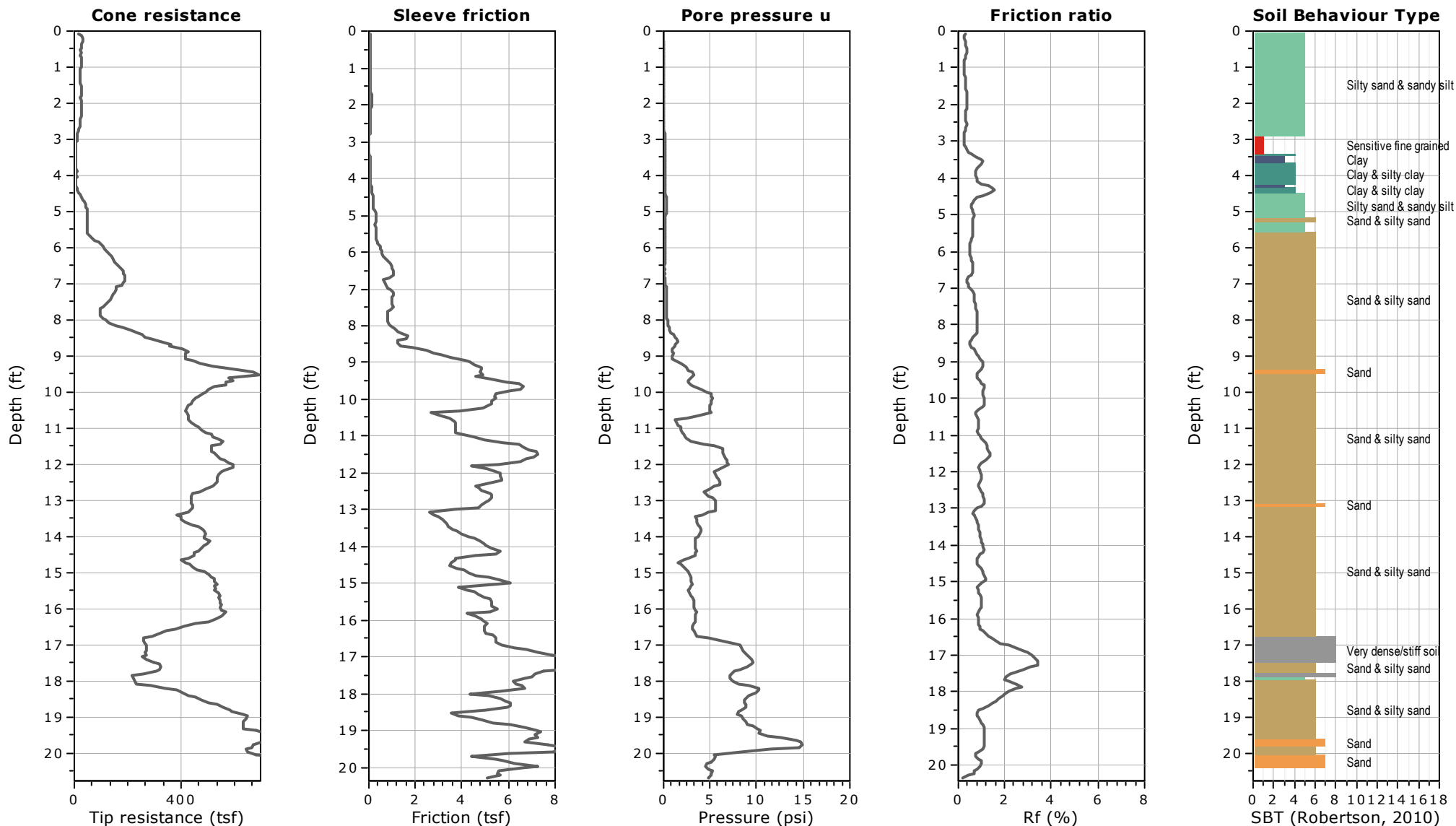
Project: Riverside Community Hospital

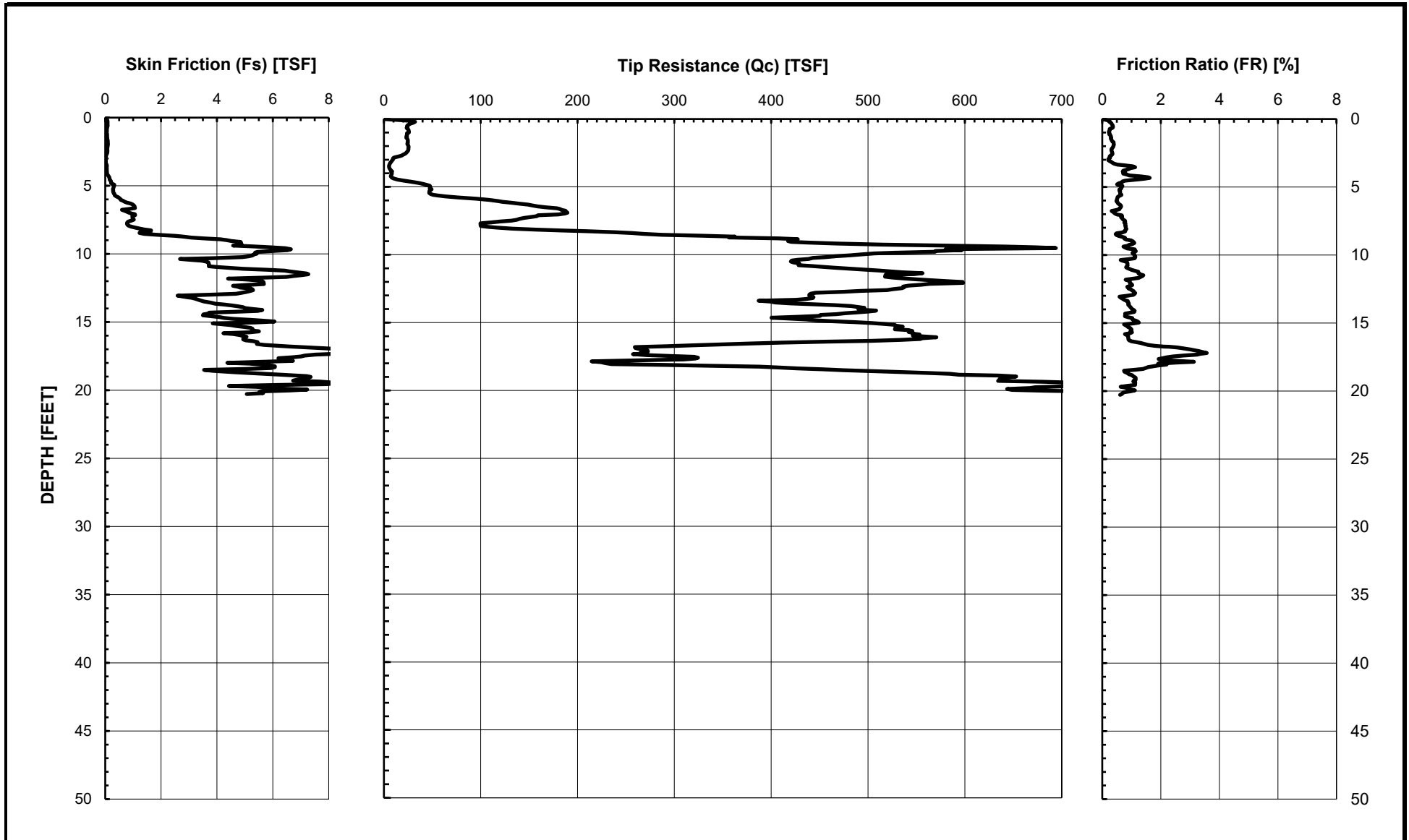
Location: 4468 Brockton Avenue, Riverside, California 92501

CPT-3

Total depth: 20.67 ft, Date: 3/22/2024

Surface Elevation: 793.00 ft





GROUP DELTA

CONE PENETROMETER DATA (CPT-3)

Document No. 24-0011

Project No. SD809

FIGURE A-3b



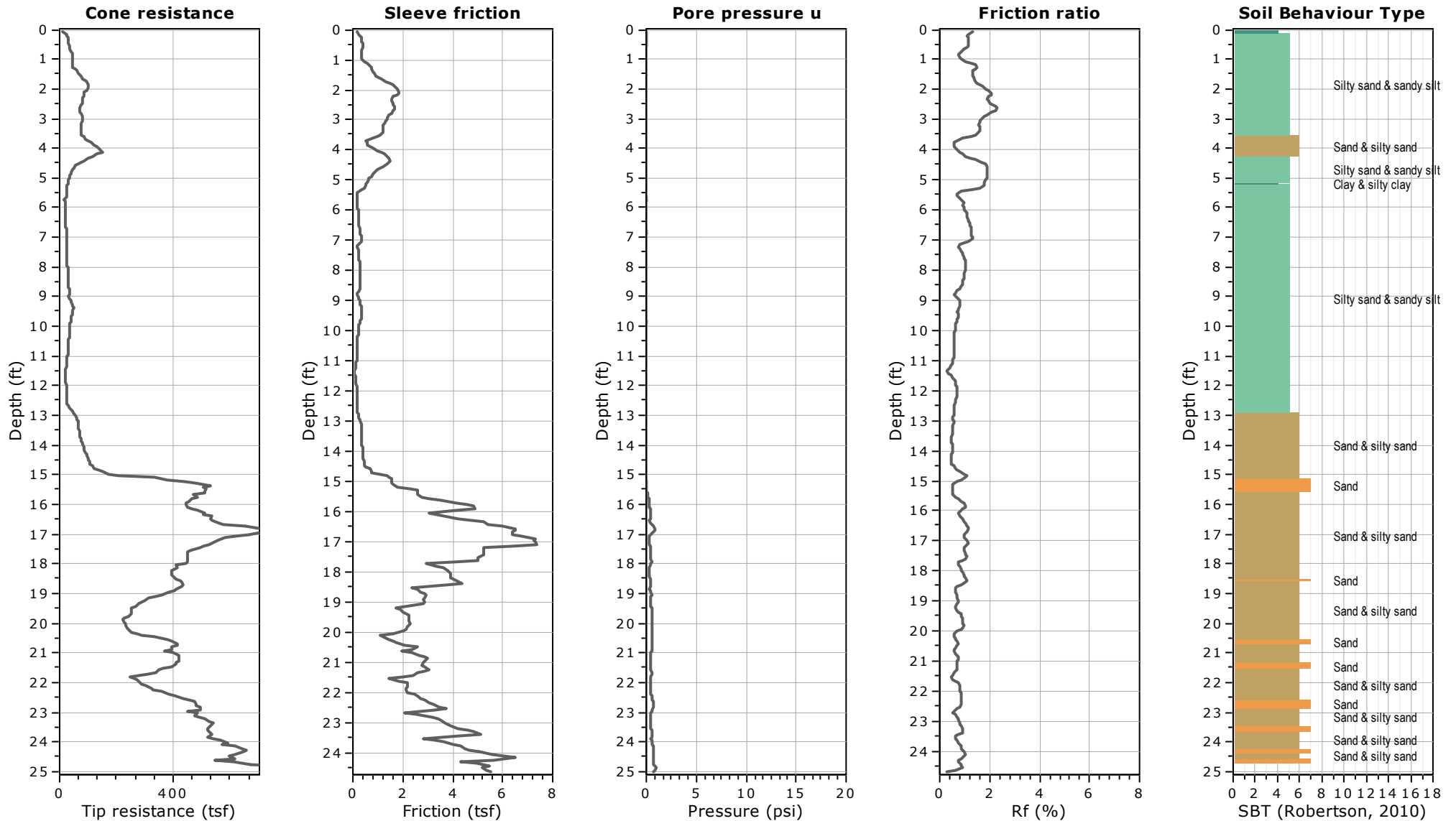
9245 Activity Road, Suite 103
San Diego, California 92126
www.GroupDelta.com

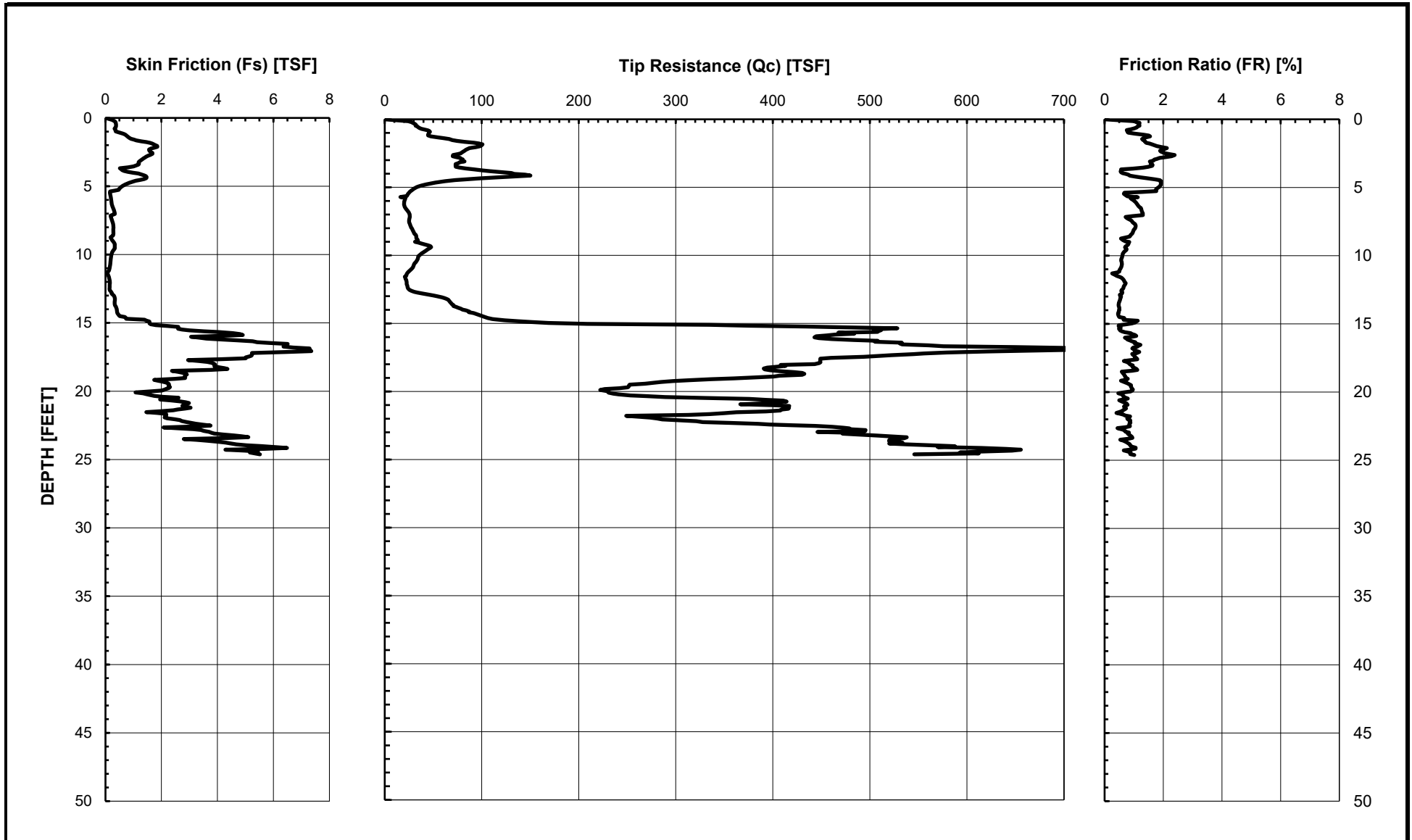
Location: 4468 Brockton Avenue, Riverside, California 92501

CPT-4

Total depth: 25.01 ft, Date: 3/22/2024

Surface Elevation: 794.00 ft





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CONE PENETROMETER DATA (CPT-4)

Document No. 24-0011

Project No. SD809

FIGURE A-4b



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San Diego, California 92126
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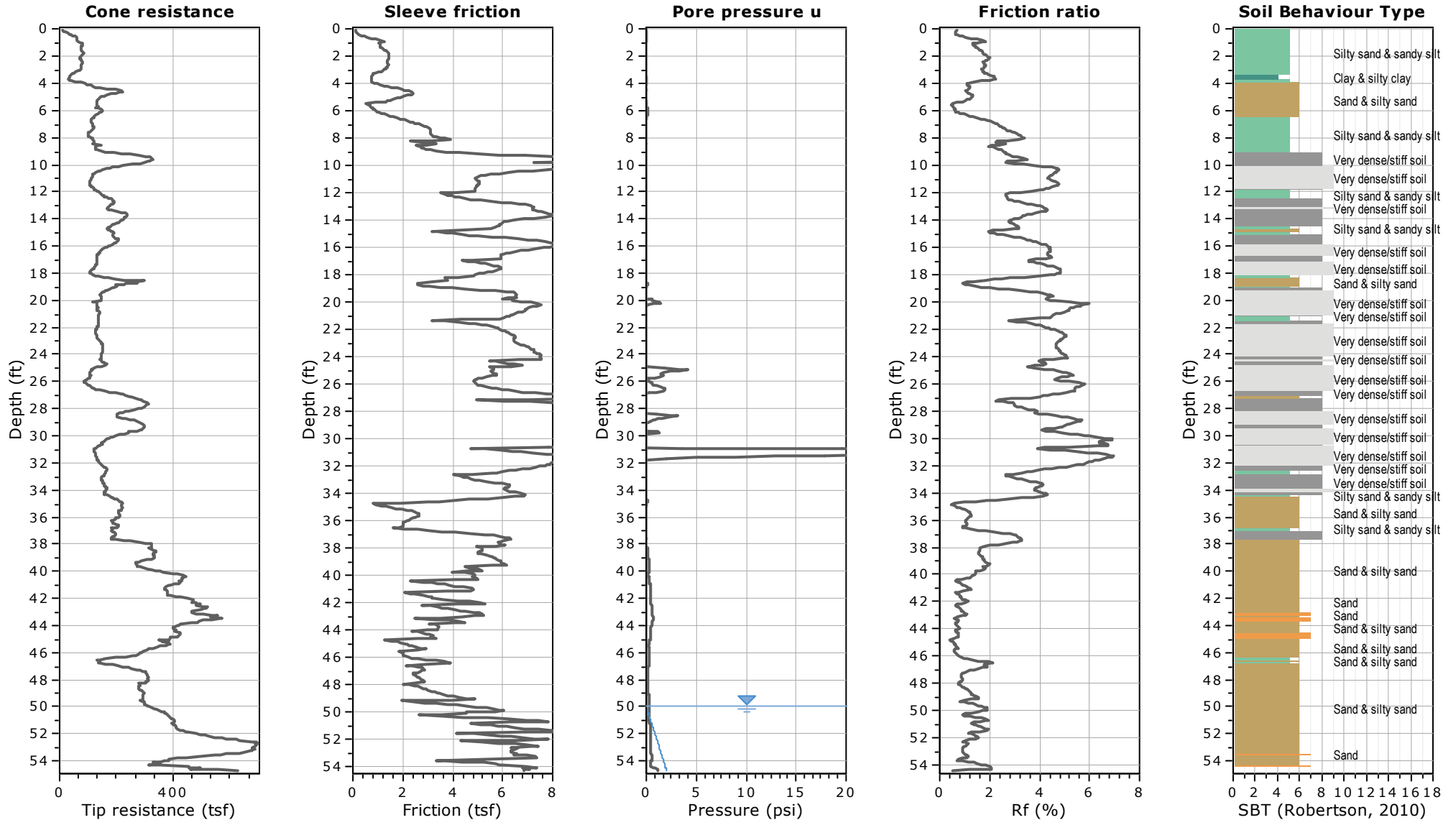
Project: Riverside Community Hospital

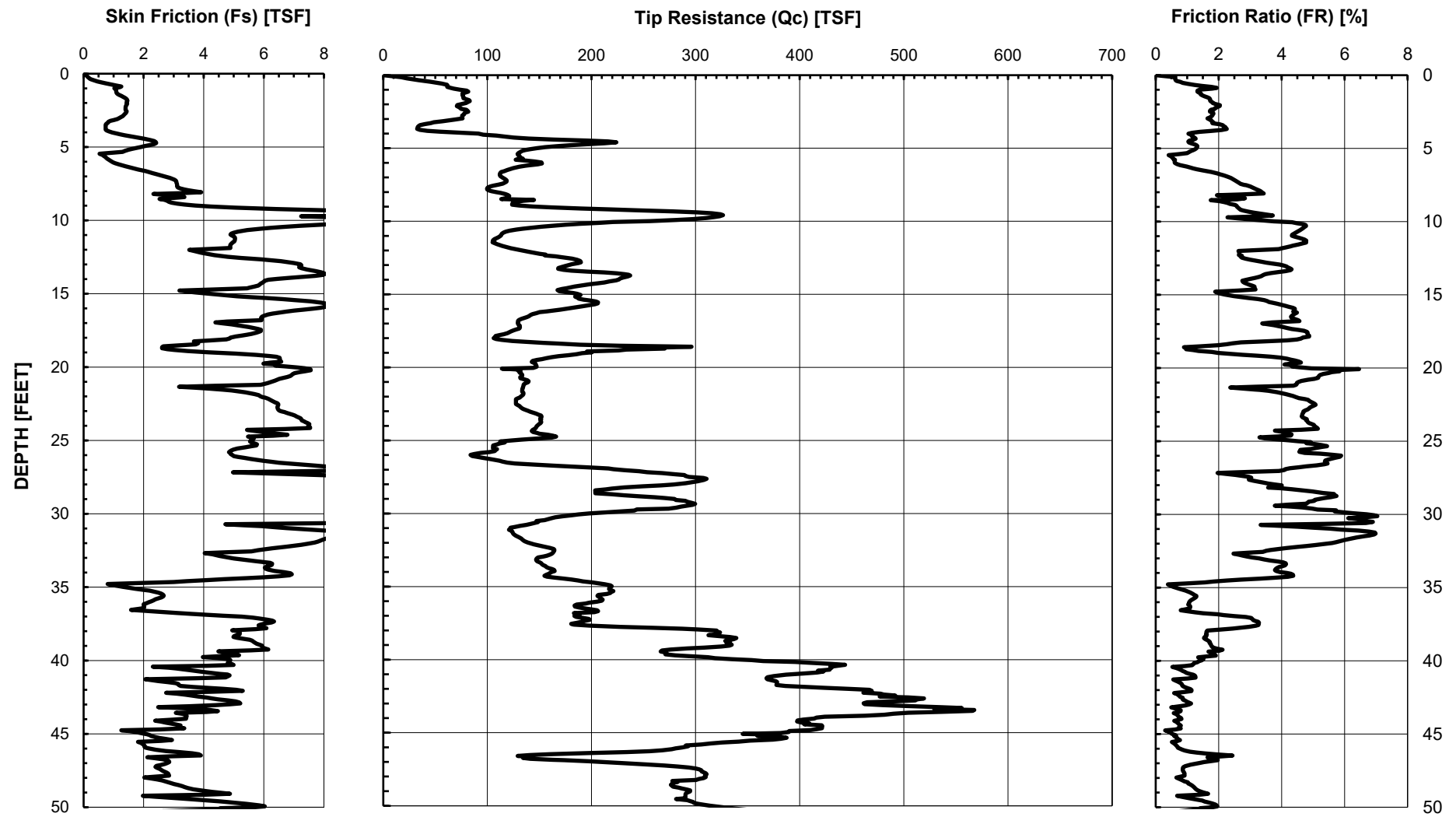
Location: 4468 Brockton Avenue, Riverside, California 92501

CPT-5

Total depth: 54.73 ft, Date: 3/22/2024

Surface Elevation: 839.00 ft





GROUP DELTA

CONE PENETROMETER DATA (CPT-5)

Document No. 24-0011

Project No. SD809

FIGURE A-5b

Group Delta Consultants
Riverside Community Hospital
Riverside, CA

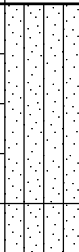
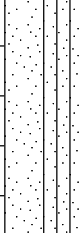
CPT Shear Wave Measurements

Location	Tip Depth (ft)	Geophone Depth (ft)	Travel Distance (ft)	S-Wave Arrival (msec)	S-Wave Velocity from Surface (ft/sec)	Interval S-Wave Velocity (ft/sec)
CPT-1	5.02	4.02	4.49	5.86	766	
	10.04	9.04	9.26	13.64	679	613
	15.03	14.03	14.17	20.74	683	692
	20.01	19.01	19.11	27.28	701	756
	25.03	24.03	24.11	31.36	769	1225
	30.02	29.02	29.09	35.44	821	1220
	33.92	32.92	32.98	37.44	881	1946
CPT-2	5.02	4.02	4.49	4.24	1059	
	10.01	9.01	9.23	13.50	684	512
	15.03	14.03	14.17	19.80	716	785
	20.01	19.01	19.11	26.12	732	782
	25.03	24.03	24.11	32.00	754	850
	30.02	29.02	29.09	35.22	826	1545
	35.01	34.01	34.07	37.78	902	1945
CPT-5	5.02	4.02	4.49	2.46	1825	
	10.01	9.01	9.23	5.94	1554	1362
	15.06	14.06	14.20	8.96	1585	1646
	20.05	19.05	19.15	13.04	1469	1214
	25.03	24.03	24.11	16.26	1483	1540
	30.02	29.02	29.09	20.44	1423	1190
	35.01	34.01	34.07	24.28	1403	1297
	39.99	38.99	39.04	26.98	1447	1842
	45.01	44.01	44.06	31.46	1400	1119
	50.03	49.03	49.07	35.20	1394	1341
	54.69	53.69	53.73	38.74	1387	1315

Shear Wave Source Offset - 2 ft

S-Wave Velocity from Surface = Travel Distance/S-Wave Arrival
Interval S-Wave Velocity = (Travel Dist2-Travel Dist1)/(Time2-Time1)

GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD						PROJECT NAME Riverside Community Hospital				PROJECT NUMBER SD809		BORING B-01			
SITE LOCATION Proposed Parking Garage and Tower Sites										START 4/4/2024		FINISH 4/4/2024		SHEET NO. 1 of 2	
DRILLING COMPANY Yellow Jacket Drilling						DRILLING METHOD Hollow Stem Auger				LOGGED BY JWJ		CHECKED BY MAF			
DRILLING EQUIPMENT CME 75 Limited Access Rig #142						BORING DIA. (in) 8		TOTAL DEPTH (ft) 36.5		GROUND ELEV (ft) 800		DEPTH/ELEV. GROUNDWATER (ft) ▼ / na			
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)						NOTES ETR ~ 74%, N ₆₀ ~ 74/60 * N ~ 1.23 * N									
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION			
			B-1						PA			FILL: SILTY SAND (SM); loose to medium dense; dark yellowish brown (10YR 4/4); moist; mostly fine to medium SAND; some fines; trace GRAVEL; nonplastic. (1% Gravel; 60% Sand; 39% Fines)			
5	795		S-2	2 4 4	8	10				5		YOUNG ALLUVIUM: SILTY SAND (SM); medium dense; dark yellowish brown (10YR 4/4); moist; mostly fine to medium SAND; some fines; trace GRAVEL; nonplastic.			
10	790		R-3	3 5 7	12	10	9.9	107		10		POORLY-GRADED SAND WITH SILT (SP-SM); medium dense; dark yellowish brown (10YR 4/6); moist; mostly fine to medium SAND; little fines; trace GRAVEL; nonplastic; micaceous.			
15	785		S-4	3 4 5	9	11				15		WELL-GRADED SAND WITH SILT (SW-SM); medium dense; light yellowish brown (10YR 6/4) to dark yellowish brown (10YR 4/6); moist; mostly fine to coarse SAND; few fines; trace GRAVEL; nonplastic; micaceous.			
20	780		R-5	7 11 15	26	21	22.4	102	PA PI C	20		LEAN CLAY (CL); very stiff; yellowish brown (10YR 4/6); moist; mostly fines; little fine SAND; low plasticity. (0% Gravel; 11% Sand; 89% Fines) (LL~37; PL~19; PI~18)			
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.				FIGURE A-6 a	

GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD							PROJECT NAME Riverside Community Hospital				PROJECT NUMBER SD809		BORING B-01		
SITE LOCATION Proposed Parking Garage and Tower Sites										START 4/4/2024		FINISH 4/4/2024		SHEET NO. 2 of 2	
DRILLING COMPANY Yellow Jacket Drilling							DRILLING METHOD Hollow Stem Auger				LOGGED BY JWJ		CHECKED BY MAF		
DRILLING EQUIPMENT CME 75 Limited Access Rig #142							BORING DIA. (in) 8		TOTAL DEPTH (ft) 36.5		GROUND ELEV (ft) 800		DEPTH/ELEV. GROUNDWATER (ft) ▼ / na		
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)							NOTES ETR ~ 74%, N ₆₀ ~ 74/60 * N ~ 1.23 * N								
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION			
			S-6	5 9 10	19	23						YOUNG ALLUVIUM: SANDY LEAN CLAY (CL); very stiff; yellowish brown (10YR 5/6); moist; mostly fines; little fine SAND; low plasticity. PP~2 TSF			
30	770		R-7	8 13 20	33	27	16.0	117		30		CLAYEY SAND (SC); medium dense; yellowish brown (10YR 5/4); moist; mostly fine to coarse SAND; some fines; low plasticity.			
35	765		S-8	3 9 18	27	33				35		OLD ALLUVIUM: SANDY LEAN CLAY (CL); hard; yellowish brown (10YR 4/6); moist; mostly fines; some fine to medium SAND; low plasticity; micaceous.			
40	760									40		Total Depth: 36½ Feet Groundwater Not Encountered			
45	755									45					
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.				FIGURE A-6 b	

GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD							PROJECT NAME Riverside Community Hospital			PROJECT NUMBER SD809		BORING B-02		
SITE LOCATION Proposed Parking Garage and Tower Sites									START 4/1/2024		FINISH 4/1/2024		SHEET NO. 1 of 3	
DRILLING COMPANY Yellow Jacket Drilling							DRILLING METHOD Hollow Stem Auger			LOGGED BY JWJ		CHECKED BY MAF		
DRILLING EQUIPMENT CME 85 Truck Mounted Rig #120							BORING DIA. (in) 8		TOTAL DEPTH (ft) 51.5		GROUND ELEV (ft) 798		DEPTH/ELEV. GROUNDWATER (ft) ▼ 36.0 / 762.0	
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)							NOTES ETR ~ 80%, N ₆₀ ~ 80/60 * N ~ 1.33 * N							
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION		
												PAVEMENT: 2-inches Asphalt Concrete.		
5	795		B-1									FILL: SILTY SAND (SM); loose to medium dense; dark yellowish brown (10YR 4/4); moist; mostly fine to medium SAND; some fines; trace fine GRAVEL; nonplastic; micaceous.		
			S-2	1 2 2	4	5				5				
	790													
10			R-3	2 5 5	10	9	3.2	109	PA DS	10		(3% Gravel; 83% Sand; 14% Fines)		
	785													
15			S-4	1 2 3	5	7				15				
	780													
20			R-5	4 12 15	27	24	22.6	106	PA PI C	20		LEAN CLAY (CL); hard; dark yellowish brown (10YR 4/4); moist; mostly fines; trace fine SAND; medium plasticity. (0% Gravel; 3% Sand; 97% Fines) (LL~46; PL~22; PI~24)		
	775													
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.			FIGURE A-7 a	

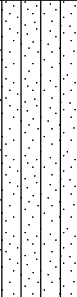
GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD							PROJECT NAME Riverside Community Hospital				PROJECT NUMBER SD809		BORING B-02		
SITE LOCATION Proposed Parking Garage and Tower Sites										START 4/1/2024		FINISH 4/1/2024		SHEET NO. 2 of 3	
DRILLING COMPANY Yellow Jacket Drilling							DRILLING METHOD Hollow Stem Auger				LOGGED BY JWJ		CHECKED BY MAF		
DRILLING EQUIPMENT CME 85 Truck Mounted Rig #120							BORING DIA. (in) 8		TOTAL DEPTH (ft) 51.5		GROUND ELEV (ft) 798		DEPTH/ELEV. GROUNDWATER (ft) ▼ 36.0 / 762.0		
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)							NOTES ETR ~ 80%, N ₆₀ ~ 80/60 * N ~ 1.33 * N								
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION			
			S-6	4 4 6	10	13						YOUNG ALLUVIUM: SANDY LEAN CLAY (CL); very stiff; dark yellowish brown (10YR 3/6); moist; mostly fines; little fine SAND; medium plasticity. PP ~ 3 TSF			
30	770		R-7	8 17 22	39	35	11.0	123		30		OLD ALLUVIUM: CLAYEY SAND (SC); dense; dark yellowish brown (10YR 4/4); moist; mostly fine to coarse SAND; some fines; trace GRAVEL; medium plasticity.			
	765														
35	760		S-8	4 8 14	22	29				35		CLAYEY SAND (SC); medium dense to dense; dark yellowish brown (10YR 4/4); saturated; mostly fine to coarse SAND; some fines; trace GRAVEL; medium plasticity.			
40	755		R-9	15 24 30	54	48	12.6	119		40		SILTY SAND (SM); dense; dark yellowish brown (10YR 4/6); saturated; mostly fine to medium SAND; some fines; trace fine GRAVEL; nonplastic.			
45	750		R-10			---				45		SANDY LEAN CLAY (CL); stiff; dark yellowish brown (10YR 3/6); saturated; mostly fines; little fine SAND; medium plasticity.			
												WELL-GRADED SAND WITH SILT (SW-SM); dense; very pale brown (10YR 8/4) to brownish yellow (10YR 6/6), saturated; mostly fine to coarse SAND; few GRAVEL; trace fines; nonplastic; micaceous.			
												Samples highly disturbed by rapid groundwater flow into the bottom of the hollow-stem auger.			
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.				FIGURE A-7 b	

GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD							PROJECT NAME Riverside Community Hospital				PROJECT NUMBER SD809		BORING B-02		
SITE LOCATION Proposed Parking Garage and Tower Sites										START 4/1/2024		FINISH 4/1/2024		SHEET NO. 3 of 3	
DRILLING COMPANY Yellow Jacket Drilling							DRILLING METHOD Hollow Stem Auger				LOGGED BY JWJ		CHECKED BY MAF		
DRILLING EQUIPMENT CME 85 Truck Mounted Rig #120							BORING DIA. (in) 8		TOTAL DEPTH (ft) 51.5		GROUND ELEV (ft) 798		DEPTH/ELEV. GROUNDWATER (ft) ▼ 36.0 / 762.0		
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)							NOTES ETR ~ 80%, N ₆₀ ~ 80/60 * N ~ 1.33 * N								
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION			
			S-11			---						OLD ALLUVIUM: WELL-GRADED SAND (SW); dense; light yellowish brown (10YR 6/4); saturated; mostly fine to medium SAND; few fines; nonplastic. Total Depth: 51½ Feet Groundwater Depth: 36 Feet Groundwater initially observed at 46 feet when the bottom of the borehole heaved.			
745										55					
55															
740										60					
60															
735										65					
65															
730										70					
70															
725															
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.				FIGURE A-7 c	

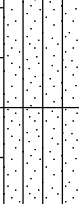
GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD							PROJECT NAME Riverside Community Hospital				PROJECT NUMBER SD809		BORING B-03		
SITE LOCATION Proposed Parking Garage and Tower Sites										START 4/2/2024		FINISH 4/2/2024		SHEET NO. 1 of 2	
DRILLING COMPANY Yellow Jacket Drilling							DRILLING METHOD Hollow Stem Auger				LOGGED BY JWJ		CHECKED BY MAF		
DRILLING EQUIPMENT CME 75 Limited Access Rig #142							BORING DIA. (in) 8		TOTAL DEPTH (ft) 36.5		GROUND ELEV (ft) 798		DEPTH/ELEV. GROUNDWATER (ft) ▼ / na		
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)							NOTES ETR ~ 80%, N ₆₀ ~ 80/60 * N ~ 1.33 * N								
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION			
												PAVEMENT: 2-inches Asphalt Concrete.			
5	795		B-1						PA EI CR	5		FILL: SILTY SAND (SM); loose to medium dense; yellowish brown (10YR 5/6); moist; mostly fine to medium SAND; some fines; trace GRAVEL; nonplastic; micaceous. (4% Gravel; 49% Sand; 47% Fines)			
			R-2	6 11 13	24	21	3.1	104							
	790														
10			S-3	4 6 6	12	16				10		YOUNG ALLUVIUM: SILTY SAND (SM); medium dense; yellowish brown (10YR 5/6); moist; mostly fine to coarse SAND; little fines; nonplastic.			
	785														
15			R-4	5 11 12	23	20	4.2	106		15		WELL-GRADED SAND WITH SILT (SW-SM); medium dense; very pale brown (10 YR 7/4) to yellowish brown (10YR 5/6); moist; mostly fine to coarse SAND; few fines; trace GRAVEL; nonplastic; micaceous.			
	780														
20			S-5	6 8 7	15	20				20		Few GRAVEL.			
	775														
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.			FIGURE A-8 a		

GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD				PROJECT NAME Riverside Community Hospital				PROJECT NUMBER SD809		BORING B-03		
SITE LOCATION Proposed Parking Garage and Tower Sites						START 4/2/2024		FINISH 4/2/2024		SHEET NO. 2 of 2		
DRILLING COMPANY Yellow Jacket Drilling				DRILLING METHOD Hollow Stem Auger				LOGGED BY JWJ		CHECKED BY MAF		
DRILLING EQUIPMENT CME 75 Limited Access Rig #142				BORING DIA. (in) 8		TOTAL DEPTH (ft) 36.5		GROUND ELEV (ft) 798		DEPTH/ELEV. GROUNDWATER (ft) ▼ / na		
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)				NOTES ETR ~ 80%, N ₆₀ ~ 80/60 * N ~ 1.33 * N								
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION
			R-6	11 30 40	70	62	6.2	114				OLD ALLUVIUM: WELL-GRADED SAND WITH SILT (SW-SM); very dense; very pale brown (10YR 7/3) to dark yellowish brown (10YR 4/6); moist; mostly fine to coarse SAND; few fines; nonplastic; micaceous.
30	770		S-7	7 12 14	26	35				30		CLAYEY SAND (SC); dense; dark yellowish brown (10YR 4/6); moist; mostly fine to medium SAND; some fines; trace GRAVEL; low plasticity; micaceous.
35	765		R-8	11 19 36	55	49	11.6	122		35		WELL-GRADED SAND WITH SILT (SW-SM); dense; yellowish brown (10YR 5/6); moist; mostly fine to coarse SAND; little fines; trace GRAVEL; nonplastic; micaceous
	760											Total Depth: 36½ Feet Groundwater Not Encountered
40										40		
	755											
45										45		
	750											
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.		FIGURE A-8 b

GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD				PROJECT NAME Riverside Community Hospital				PROJECT NUMBER SD809		BORING B-04		
SITE LOCATION Proposed Parking Garage and Tower Sites						START 4/4/2024		FINISH 4/4/2024		SHEET NO. 1 of 2		
DRILLING COMPANY Yellow Jacket Drilling				DRILLING METHOD Hollow Stem Auger				LOGGED BY JWJ		CHECKED BY MAF		
DRILLING EQUIPMENT CME 75 Limited Access Rig #142				BORING DIA. (in) 8		TOTAL DEPTH (ft) 31		GROUND ELEV (ft) 794		DEPTH/ELEV. GROUNDWATER (ft) ▼ / na		
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)				NOTES ETR ~ 74%, N ₆₀ ~ 74/60 * N ~ 1.23 * N								
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION
			B-1									FILL: SILTY SAND (SM); loose to medium dense; dark yellowish brown (10YR 4/4); moist; mostly fine to coarse SAND; some fines; nonplastic.
5	790		R-2	3 4 4	8	7	9.4	112	PA DS	5		YOUNG ALLUVIUM: SILTY SAND (SM); loose; yellowish brown (10YR 5/6); moist; mostly fine to medium SAND; some fines; nonplastic. (0% Gravel; 65% Sand; 35% Fines)
10	785		S-3	2 2 2	4	5				10		CLAYEY SAND (SC); loose; dark yellowish brown (10YR 4/4); moist; mostly fine to medium SAND; some fines; nonplastic.
15	780		R-4	5 9 11	20	16	2.6	111		15		WELL-GRADED SAND WITH SILT (SW-SM); medium dense; yellowish brown (10YR 5/6); moist; mostly fine to medium SAND; few fines; trace fine GRAVEL; nonplastic.
20	775		S-5	14 27 43	70	86				20		OLD ALLUVIUM: WELL-GRADED SAND WITH GRAVEL (SW); very dense; strong brown (7.5YR 5/8); moist; mostly fine SAND; little GRAVEL; few fines; nonplastic; micaceous.
	770											
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.		FIGURE A-9 a

BORING RECORD				PROJECT NAME Riverside Community Hospital				PROJECT NUMBER SD809				BORING B-04																					
SITE LOCATION Proposed Parking Garage and Tower Sites								START 4/4/2024				FINISH 4/4/2024				SHEET NO. 2 of 2																	
DRILLING COMPANY Yellow Jacket Drilling								DRILLING METHOD Hollow Stem Auger								LOGGED BY JWJ				CHECKED BY MAF													
DRILLING EQUIPMENT CME 75 Limited Access Rig #142								BORING DIA. (in) 8				TOTAL DEPTH (ft) 31				GROUND ELEV (ft) 794				DEPTH/ELEV. GROUNDWATER (ft) ▼ / na													
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)								NOTES ETR ~ 74%, N ₆₀ ~ 74/60 * N ~ 1.23 * N																									
DEPTH (feet)		ELEVATION (feet)		SAMPLE TYPE		SAMPLE NO.		PENETRATION RESISTANCE (BLOWS / 6 IN)		BLOW/FT "N"		N ₆₀		MOISTURE (%)		DRY DENSITY (pcf)		OTHER TESTS		DEPTH (feet)		GRAPHIC LOG		DESCRIPTION AND CLASSIFICATION									
30		765				R-6		40 50 (4")		100+		100+								30				OLD ALLUVIUM: WELL-GRADED SAND WITH GRAVEL (SW); very dense; strong brown (7.5YR 5/8); moist; mostly fine SAND; little GRAVEL; few fines; nonplastic; micaceous.									
35		760				S-7		16 50		76		93												Total Depth: 31 Feet Groundwater Not Encountered									
40		755																															
45		750																															
745																																	

GROUP DELTA CONSULTANTS, INC.

9245 Activity Road, Suite 103

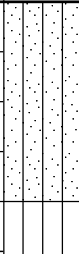
San Diego, California 92126

THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.

FIGURE

A-9 b

GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD							PROJECT NAME Riverside Community Hospital				PROJECT NUMBER SD809		BORING B-05		
SITE LOCATION Proposed Parking Garage and Tower Sites										START 4/4/2024		FINISH 4/4/2024		SHEET NO. 1 of 2	
DRILLING COMPANY Yellow Jacket Drilling							DRILLING METHOD Hollow Stem Auger				LOGGED BY JWJ		CHECKED BY MAF		
DRILLING EQUIPMENT CME 75 Limited Access Rig #142							BORING DIA. (in) 8		TOTAL DEPTH (ft) 31.5		GROUND ELEV (ft) 796		DEPTH/ELEV. GROUNDWATER (ft) ▼ / na		
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)							NOTES ETR ~ 74%, N ₆₀ ~ 74/60 * N ~ 1.23 * N								
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION			
	795		B-1									FILL: SILTY SAND (SM); loose to medium dense; dark yellowish brown (10YR 4/4); moist; mostly fine to medium SAND; some fines; nonplastic. Contains trash and demolition debris.			
5	790		R-2	1 2 4	6	5	12.6	106	PA	5		YOUNG ALLUVIUM: SANDY SILT (ML); loose; dark yellowish brown (10YR 4/6); moist; mostly fines; some fine to medium SAND; nonplastic; micaceous. (0% Gravel; 33% Sand; 67% Fines)			
10	785		S-3	6 7 10	17	21				10		CLAYEY SAND (SC); medium dense; dark yellowish brown (10YR 4/4); moist; mostly fine to medium SAND; some fines; low plasticity; micaceous.			
15	780		R-4	10 32 60	92	75	6.5	127		15		OLD ALLUVIUM: CLAYEY SAND WITH GRAVEL (SC); very dense; yellowish brown (10YR 5/6); moist; mostly fine to coarse SAND; some fines; little GRAVEL; nonplastic; micaceous.			
20	775		S-5	10 13 20	33	41				20		WELL-GRADED SAND WITH CLAY (SW-SC); dense; very pale brown (10YR 8/4) to yellowish brown (10YR 5/8); moist; mostly fine to coarse SAND; little fines; few GRAVEL; nonplastic; micaceous.			
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.				FIGURE A-10 a	

GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD							PROJECT NAME Riverside Community Hospital				PROJECT NUMBER SD809		BORING B-05		
SITE LOCATION Proposed Parking Garage and Tower Sites										START 4/4/2024		FINISH 4/4/2024		SHEET NO. 2 of 2	
DRILLING COMPANY Yellow Jacket Drilling							DRILLING METHOD Hollow Stem Auger				LOGGED BY JWJ		CHECKED BY MAF		
DRILLING EQUIPMENT CME 75 Limited Access Rig #142							BORING DIA. (in) 8		TOTAL DEPTH (ft) 31.5		GROUND ELEV (ft) 796		DEPTH/ELEV. GROUNDWATER (ft) ▼ / na		
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)							NOTES ETR ~ 74%, N ₆₀ ~ 74/60 * N ~ 1.23 * N								
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION			
	770		R-6	9 17 22	39	32	13.5	109				OLD ALLUVIUM: WELL-GRADED SAND WITH GRAVEL (SW); dense; very pale brown (10YR 8/2) to yellowish brown (10YR 5/4); moist; mostly fine to medium SAND; some GRAVEL; few fines; nonplastic; micaceous.			
30	765		S-7	10 27 50	77	95				30		Very pale brown (10YR 8/3) to light yellowish brown (10YR 6/4); very dense.			
												Total Depth: 31½ Feet Groundwater Not Encountered			
35	760									35					
40	755									40					
45	750									45					
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.				FIGURE A-10 b	

GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

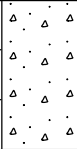
BORING RECORD				PROJECT NAME Riverside Community Hospital				PROJECT NUMBER SD809		BORING B-06				
SITE LOCATION Proposed Parking Garage and Tower Sites						START 4/3/2024		FINISH 4/4/2024		SHEET NO. 1 of 2				
DRILLING COMPANY Yellow Jacket Drilling				DRILLING METHOD Hollow Stem Auger				LOGGED BY JWJ		CHECKED BY MAF				
DRILLING EQUIPMENT CME 75 Limited Access Rig #142				BORING DIA. (in) 8		TOTAL DEPTH (ft) 31.5		GROUND ELEV (ft) 794		DEPTH/ELEV. GROUNDWATER (ft) ▼ / na				
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)				NOTES ETR ~ 74%, N ₆₀ ~ 74/60 * N ~ 1.23 * N										
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION		
												PAVEMENT: 4-inches Portland Cement Concrete.		
			B-1						PA EI CR R	5		FILL: SILTY SAND (SM); loose to medium dense; dark yellowish brown (10YR 3/6); moist; mostly fines; some fine to medium SAND; nonplastic; micaceous.		
	790		S-2	2 2 2	4	5				5		YOUNG ALLUVIUM: CLAYEY SAND (SC); loose; dark yellowish brown (10YR 4/4); moist; mostly fine to medium SAND; some fines; low plasticity. Sample has the odor of decaying vegetation. (0% Gravel; 55% Sand; 45% Fines)		
	785		R-3	10 24 36	60	49	13.1	114		10			WELL-GRADED SAND (SW); dense; very pale brown (10YR 8/4) to yellowish brown (10YR 5/6); moist; mostly fine to coarse SAND; little GRAVEL; trace fines; nonplastic.	
	780													
			S-4	12 26 30	56	69				15		OLD ALLUVIUM: SILTY SAND WITH GRAVEL (SM); very dense; yellowish brown (10YR 5/6); moist; mostly fine to medium SAND; some fines; little GRAVEL; nonplastic.		
			B-5											
	775													
			R-6	26 50 (5")	100+	100+				20		WELL-GRADED SAND WITH SILT (SW-SM); very dense; very pale brown (10YR 8/3) to brownish yellow (10YR 6/8); moist; mostly fine to coarse SAND; few fines; few GRAVEL; nonplastic; micaceous.		
	770													
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.			FIGURE A-11 a	

GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD							PROJECT NAME Riverside Community Hospital				PROJECT NUMBER SD809		BORING B-06		
SITE LOCATION Proposed Parking Garage and Tower Sites										START 4/3/2024		FINISH 4/4/2024		SHEET NO. 2 of 2	
DRILLING COMPANY Yellow Jacket Drilling							DRILLING METHOD Hollow Stem Auger				LOGGED BY JWJ		CHECKED BY MAF		
DRILLING EQUIPMENT CME 75 Limited Access Rig #142							BORING DIA. (in) 8		TOTAL DEPTH (ft) 31.5		GROUND ELEV (ft) 794		DEPTH/ELEV. GROUNDWATER (ft) ▼ / na		
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)							NOTES ETR ~ 74%, N ₆₀ ~ 74/60 * N ~ 1.23 * N								
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION			
30	765		S-7 B-8	7 25 25	50	62				30		OLD ALLUVIUM: SILTY SAND WITH GRAVEL (SM); very dense; yellowish brown (10YR 5/6); moist; mostly fine to medium SAND; some fines; little GRAVEL; nonplastic.			
35	760									35		Total Depth: 31½ Feet Groundwater Not Encountered			
40	755									40					
45	750									45					
	745														
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.				FIGURE A-11 b	

BORING RECORD				PROJECT NAME Riverside Community Hospital				PROJECT NUMBER SD809				BORING B-07																					
SITE LOCATION Proposed Parking Garage and Tower Sites								START 4/4/2024				FINISH 4/4/2024				SHEET NO. 1 of 2																	
DRILLING COMPANY Yellow Jacket Drilling								DRILLING METHOD Hollow Stem Auger								LOGGED BY JWJ				CHECKED BY MAF													
DRILLING EQUIPMENT CME 75 Limited Access Rig #142								BORING DIA. (in) 8				TOTAL DEPTH (ft) 28				GROUND ELEV (ft) 794				DEPTH/ELEV. GROUNDWATER (ft) ▼ / na													
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)								NOTES ETR ~ 74%, N ₆₀ ~ 74/60 * N ~ 1.23 * N																									
DEPTH (feet)		ELEVATION (feet)		SAMPLE TYPE		SAMPLE NO.		PENETRATION RESISTANCE (BLOWS / 6 IN)		BLOW/FT "N"		N ₆₀		MOISTURE (%)		DRY DENSITY (pcf)		OTHER TESTS		DEPTH (feet)		GRAPHIC LOG		DESCRIPTION AND CLASSIFICATION									
5		790		B-1		S-2		2 3 5		8		10		15.7		113		PA		5				FILL: CLAYEY SAND (SC); loose to medium dense; dark yellowish brown (10YR 3/6); moist to wet; mostly fine to medium SAND; some fines; little GRAVEL; low plasticity.									
		785		R-3		2 4 5		9		7												YOUNG ALLUVIUM: SILTY SAND (SM); loose to medium dense; yellowish brown (10YR 4/6); moist; mostly fine SAND; some fines; micaceous; nonplastic.											
		780		S-4		3 7 15		22		27												SANDY SILT (ML); loose; dark yellowish brown (10YR 4/6); moist; mostly fines; some fine to medium SAND; trace GRAVEL; nonplastic; micaceous. (1% Gravel; 39% Sand; 60% Fines)											
		775		R-5		16 34 50		84		69												WELL-GRADED SAND WITH SILT (SW-SM); medium dense; very pale brown (10YR 7/3) to yellowish brown (10YR 5/6); moist; mostly fine to coarse SAND; trace fines; trace GRAVEL; nonplastic.											
		770																						OLD ALLUVIUM: WELL-GRADED SAND (SW); very dense; very pale brown (10YR 8/3) to brownish yellow (10YR 6/8); moist; mostly fine to coarse SAND; few GRAVEL; trace fines; nonplastic; micaceous.									
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126																THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.										FIGURE A-12 a							

GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD							PROJECT NAME Riverside Community Hospital				PROJECT NUMBER SD809		BORING B-07		
SITE LOCATION Proposed Parking Garage and Tower Sites										START 4/4/2024		FINISH 4/4/2024		SHEET NO. 2 of 2	
DRILLING COMPANY Yellow Jacket Drilling							DRILLING METHOD Hollow Stem Auger				LOGGED BY JWJ		CHECKED BY MAF		
DRILLING EQUIPMENT CME 75 Limited Access Rig #142							BORING DIA. (in) 8		TOTAL DEPTH (ft) 28		GROUND ELEV (ft) 794		DEPTH/ELEV. GROUNDWATER (ft) ▼ / na		
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)							NOTES ETR ~ 74%, N ₆₀ ~ 74/60 * N ~ 1.23 * N								
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION			
			S-6	21 29 20	49	60						OLD ALLUVIUM: WELL-GRADED SAND (SW); very dense; very pale brown (10YR 8/3) to brownish yellow (10YR 6/8); moist; mostly fine to coarse SAND; few GRAVEL; trace fines; nonplastic; micaceous.			
30	765									30		Total Depth: 28 Feet Groundwater Not Encountered			
35	760									35					
40	755									40					
45	750									45					
	745														
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.				FIGURE A-12 b	

GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD				PROJECT NAME Riverside Community Hospital				PROJECT NUMBER SD809				BORING B-08			
SITE LOCATION Proposed Parking Garage and Tower Sites								START 4/1/2024		FINISH 4/1/2024		SHEET NO. 1 of 2			
DRILLING COMPANY Yellow Jacket Drilling						DRILLING METHOD Hollow Stem Auger				LOGGED BY JWJ		CHECKED BY MAF			
DRILLING EQUIPMENT CME 75 Limited Access Rig #142						BORING DIA. (in) 8		TOTAL DEPTH (ft) 30		GROUND ELEV (ft) 794		DEPTH/ELEV. GROUNDWATER (ft) ▼ / na			
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)						NOTES ETR ~ 80%, N ₆₀ ~ 80/60 * N ~ 1.33 * N									
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION			
			B-1									FILL: SILTY SAND (SM); loose to medium dense; brown (10YR 4/3); moist; mostly fine to coarse SAND; some fines; little GRAVEL; nonplastic; micaceous. (14% Gravel; 44% Sand; 42% Fines)			
5	790		S-2	2 4 4	8	11				5		YOUNG ALLUVIUM: POORLY-GRADED SAND WITH SILT (SP-SM); medium dense; yellowish brown (10YR 5/6); moist; mostly fine to medium SAND; few fines; trace GRAVEL; nonplastic; micaceous.			
10	785		R-3	3 4 7	11	10	7.2	116		10					
15	780		S-4	19 50 (5")	79	100+				15		OLD ALLUVIUM: WELL-GRADED SAND WITH SILT (SW-SM); very dense; very pale brown (10YR 8/4) to yellowish brown (10YR 5/6); moist; mostly fine to coarse SAND; few fines; trace GRAVEL; nonplastic; micaceous.			
20	775		R-5	15 35 50	85	75	5.3	107		20		WELL-GRADED SAND (SW); very dense; very pale brown (10YR 8/4) to yellowish brown (10YR 5/6); moist; mostly fine to coarse SAND; little GRAVEL; trace fines; nonplastic; micaceous.			
	770														
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.				FIGURE A-13 a	

BORING RECORD				PROJECT NAME Riverside Community Hospital				PROJECT NUMBER SD809				BORING B-08																															
SITE LOCATION Proposed Parking Garage and Tower Sites								START 4/1/2024				FINISH 4/1/2024				SHEET NO. 2 of 2																											
DRILLING COMPANY Yellow Jacket Drilling								DRILLING METHOD Hollow Stem Auger								LOGGED BY JWJ				CHECKED BY MAF																							
DRILLING EQUIPMENT CME 75 Limited Access Rig #142								BORING DIA. (in) 8				TOTAL DEPTH (ft) 30				GROUND ELEV (ft) 794				DEPTH/ELEV. GROUNDWATER (ft) ▼ / na																							
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)								NOTES ETR ~ 80%, N ₆₀ ~ 80/60 * N ~ 1.33 * N																																			
DEPTH (feet)		ELEVATION (feet)		SAMPLE TYPE		SAMPLE NO.		PENETRATION RESISTANCE (BLOWS / 6 IN)		BLOW/FT "N"		N ₆₀		MOISTURE (%)		DRY DENSITY (pcf)		OTHER TESTS		DEPTH (feet)		GRAPHIC LOG		DESCRIPTION AND CLASSIFICATION																			
30		765		R-6		24 50		74		66		3.2		112		30		30		30				OLD ALLUVIUM: WELL-GRADED GRAVEL WITH SAND (GW); very dense; white (10YR 8/8) to very pale brown (10YR 7/4); moist; mostly GRAVEL; some fine to coarse SAND; trace fines; nonplastic.																			
		760		S-7		22 50 (5")		82		100+														Total Depth: 30 Feet Groundwater Not Encountered																			
35		755														35																											
40		750														40																											
45		745														45																											
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126																								THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.										FIGURE A-13 b									

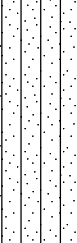
GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD							PROJECT NAME Riverside Community Hospital			PROJECT NUMBER SD809		BORING B-09			
SITE LOCATION Proposed Parking Garage and Tower Sites									START 4/5/2024		FINISH 4/5/2024		SHEET NO. 1 of 2		
DRILLING COMPANY Yellow Jacket Drilling							DRILLING METHOD Hollow Stem Auger			LOGGED BY JWJ		CHECKED BY MAF			
DRILLING EQUIPMENT CME 75 Limited Access Rig #142							BORING DIA. (in) 8		TOTAL DEPTH (ft) 31.5		GROUND ELEV (ft) 832		DEPTH/ELEV. GROUNDWATER (ft) ▼ / na		
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)							NOTES ETR ~ 74%, N ₆₀ ~ 74/60 * N ~ 1.23 * N								
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION			
	830		B-1									FILL: SILTY SAND (SM); loose to medium dense; dark yellowish brown (10YR 4/4); moist; mostly fine to medium SAND; some fines; trace GRAVEL and COBBLE; nonplastic. Some tree roots.			
5			R-2	4 7 7	14	11	10.1	113		5		YOUNG ALLUVIUM: SILTY SAND (SM); medium dense; dark yellowish brown (10YR 4/4); moist; mostly fine to medium SAND; some fines; trace GRAVEL and COBBLE; nonplastic			
	825														
10			S-3	2 2 2	4	5			PA	10		SANDY SILT (ML); loose; strong brown (10YR 5/6); moist; mostly fines; some fine SAND; micaceous; nonplastic. (0% Gravel; 31% Sand; 69% Fines)			
	820														
15			R-4	11 12 14	26	21	6.5	101		15		SILTY SAND (SM); medium dense; strong brown (10YR 5/6); moist; mostly fines; little fine SAND; nonplastic.			
	815														
20			S-5	9 13 26	39	48				20		OLD ALLUVIUM: SILTY SAND (SM); dense; dark yellowish brown (10YR 4/4); moist; mostly fine to medium SAND; some fines; nonplastic.			
	810														
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.				FIGURE A-14 a	

GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD							PROJECT NAME Riverside Community Hospital			PROJECT NUMBER SD809		BORING B-09		
SITE LOCATION Proposed Parking Garage and Tower Sites									START 4/5/2024		FINISH 4/5/2024		SHEET NO. 2 of 2	
DRILLING COMPANY Yellow Jacket Drilling							DRILLING METHOD Hollow Stem Auger			LOGGED BY JWJ		CHECKED BY MAF		
DRILLING EQUIPMENT CME 75 Limited Access Rig #142							BORING DIA. (in) 8		TOTAL DEPTH (ft) 31.5		GROUND ELEV (ft) 832		DEPTH/ELEV. GROUNDWATER (ft) ▼ / na	
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)							NOTES ETR ~ 74%, N ₆₀ ~ 74/60 * N ~ 1.23 * N							
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION		
	805		R-6	8 14 22	36	30	2.5	114				OLD ALLUVIUM: POORLY-GRADED SAND WITH SILT (SP-SM); dense; brownish yellow (10YR 6/8); moist; mostly fine to medium SAND; little GRAVEL; few fines; nonplastic.		
30			S-7	7 12 16	28	34				30		Grades from very pale brown (10YR 7/4) to yellowish brown (10YR 5/6); micaceous; trace GRAVEL.		
	800											Total Depth: 31½ Feet Groundwater Not Encountered		
35										35				
	795													
40										40				
	790													
45										45				
	785													
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.			FIGURE A-14 b	

GDC_LOG_BORING_MMXX_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD							PROJECT NAME Riverside Community Hospital			PROJECT NUMBER SD809		BORING B-10		
SITE LOCATION Proposed Parking Garage and Tower Sites									START 4/2/2024		FINISH 4/2/2024		SHEET NO. 1 of 3	
DRILLING COMPANY Yellow Jacket Drilling							DRILLING METHOD Hollow Stem Auger			LOGGED BY JWJ		CHECKED BY MAF		
DRILLING EQUIPMENT CME 85 Truck Mounted Rig #120							BORING DIA. (in) 8		TOTAL DEPTH (ft) 51.5		GROUND ELEV (ft) 838		DEPTH/ELEV. GROUNDWATER (ft) ▼ / na	
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)							NOTES ETR ~ 80%, N ₆₀ ~ 80/60 * N ~ 1.33 * N							
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION		
												PAVEMENT: 8-inches of Asphalt Concrete over 6-inches of Aggregate Base.		
5	835		B-1						PA EI CR	5		FILL: SANDY SILT (ML); medium dense; yellowish brown (10YR 5/6); moist; mostly fines; some fine to medium SAND; low plasticity; micaceous. (0% Gravel; 33% Sand; 67% Fines)		
			S-2	8 9 9	18	24								
	830													
10			R-3	8 14 20	34	30	5.8	99	DS	10		SILTY SAND (SM); dense; yellowish brown (10YR 5/6); moist; mostly fine to medium SAND; some fines; low plasticity; micaceous.		
	825													
15			S-4	8 9 11	20	27				15		SANDY SILT (ML); medium dense to dense; brownish yellow (10YR 6/8); moist; mostly fines; little fine to medium SAND; low plasticity; micaceous.		
	820													
20			R-5	7 13 24	37	33	11.2	99	DS	20				
	815													
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.			FIGURE A-15 a	

GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD							PROJECT NAME Riverside Community Hospital			PROJECT NUMBER SD809		BORING B-10	
SITE LOCATION Proposed Parking Garage and Tower Sites							START 4/2/2024		FINISH 4/2/2024		SHEET NO. 2 of 3		
DRILLING COMPANY Yellow Jacket Drilling					DRILLING METHOD Hollow Stem Auger			LOGGED BY JWJ		CHECKED BY MAF			
DRILLING EQUIPMENT CME 85 Truck Mounted Rig #120					BORING DIA. (in) 8		TOTAL DEPTH (ft) 51.5		GROUND ELEV (ft) 838		DEPTH/ELEV. GROUNDWATER (ft) ▼ / na		
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)					NOTES ETR ~ 80%, N ₆₀ ~ 80/60 * N ~ 1.33 * N								
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION	
			S-6	5 9 12	21	28						FILL: SANDY SILT (ML); medium dense; light yellowish brown (10YR 4/4); moist; mostly fines; little fine SAND; nonplastic.	
30	810		R-7	7 30 50	80	71	7.5	113		30		OLD ALLUVIUM: SILTY SAND WITH GRAVEL (SM); very dense; strong brown (7.5YR 5/8); moist; mostly fine SAND; some fines; little GRAVEL; nonplastic; micaceous.	
35	805		S-8	8 17 18	35	47				35		SILTY SAND (SM); very dense; strong brown (7.5YR 5/8); moist; mostly fine SAND; some fines; few GRAVEL; nonplastic; micaceous.	
40	800		R-9	15 30 42	72	64	5.6	115		40		SILTY SAND WITH GRAVEL (SM); very dense; strong brown (7.5YR 5/8); moist; mostly fine SAND; some fines; little GRAVEL; nonplastic; micaceous.	
45	795		S-10	10 27 33	60	80				45		SANDY SILT (ML); medium dense; light yellowish brown (10YR 4/4); moist; mostly fines; little fine SAND; nonplastic; mildly cemented.	
	790											SILTY SAND (SM); very dense; very pale brown (10YR 7/4) to brownish yellow (10YR 6/6), moist; mostly fine to medium SAND; some fines; few GRAVEL; nonplastic; micaceous.	
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.		FIGURE A-15 b	

GDC_LOG_BORING_MMXX_SOIL_SD_SD809_LOGS.GPJ GDCLOG.GDT 4/19/24

BORING RECORD							PROJECT NAME Riverside Community Hospital			PROJECT NUMBER SD809		BORING B-10			
SITE LOCATION Proposed Parking Garage and Tower Sites									START 4/2/2024		FINISH 4/2/2024		SHEET NO. 3 of 3		
DRILLING COMPANY Yellow Jacket Drilling							DRILLING METHOD Hollow Stem Auger			LOGGED BY JWJ		CHECKED BY MAF			
DRILLING EQUIPMENT CME 85 Truck Mounted Rig #120							BORING DIA. (in) 8		TOTAL DEPTH (ft) 51.5		GROUND ELEV (ft) 838		DEPTH/ELEV. GROUNDWATER (ft) ▼ / na		
SAMPLING METHOD Hammer: 140 lbs., Drop: 30 in. (Automatic)							NOTES ETR ~ 80%, N ₆₀ ~ 80/60 * N ~ 1.33 * N								
DEPTH (feet)	ELEVATION (feet)	SAMPLE TYPE	SAMPLE NO.	PENETRATION RESISTANCE (BLOWS / 6 IN)	BLOW/FT "N"	N ₆₀	MOISTURE (%)	DRY DENSITY (pcf)	OTHER TESTS	DEPTH (feet)	GRAPHIC LOG	DESCRIPTION AND CLASSIFICATION			
			R-11	17 27 50	87	77	2.0	---				OLD ALLUVIUM: POORLY-GRADED SAND WITH SILT (SP-SM); very dense; very pale brown (10YR 8/3) to light yellowish brown (10YR 6/4); moist; mostly fine to medium SAND; few fines; few GRAVEL; nonplastic. Total Depth: 51½ Feet Groundwater Not Encountered			
785										55					
55															
780										60					
60															
775										65					
65															
770										70					
70															
765															
GROUP DELTA CONSULTANTS, INC. 9245 Activity Road, Suite 103 San Diego, California 92126										THIS SUMMARY APPLIES ONLY AT THE LOCATION OF THIS BORING AND AT THE TIME OF DRILLING. SUBSURFACE CONDITIONS MAY DIFFER AT OTHER LOCATIONS AND MAY CHANGE AT THIS LOCATION WITH THE PASSAGE OF TIME. THE DATA PRESENTED IS A SIMPLIFICATION OF THE ACTUAL CONDITIONS ENCOUNTERED.			FIGURE A-15 c		

APPENDIX A1
PREVIOUS EXPLORATIONS (2007 to 2013)

EXPLORATORY BORING NO. 1

Date Drilled: 12/20/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 799.0

Logged by: VJR

Measured Depth to Water(ft): 39.0

DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	SAMPLES		BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
				DRIVE	BULK				
		(SM) Silty Sand, fine to coarse with gravel to 1", brown	Fill				14.8		
				X		8			
						14	13.4	120	Ring
						19			
5		(ML) Sandy Silt, fine with medium, light brown	Native						SA
				X		3			
						4	6.7	101	Ring, Consol.
						5			
10				X		4			
						7	11.5	120	Ring, DS
						9			
		(SP-SM) Sand, medium to coarse with fine and silt, brown					2.3		
				X		7			
15						12	2.3	114	Ring
						18			
		(SW-SM) Sand with silt, fine to coarse with clay and gravel to 3", brown					5.3		SA
				X		38			
20						50/5"	3.6	117	Ring
				X		40			
25						50/5"	4.1	105	Ring, DS
				X		50/4"			
30							3.5	104	Ring
		(SP) Sand, medium to coarse with fine, gravel and cobbles to 4", brown							

BORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ/GDT 1/18/08



C.H.J.

PROPOSED HOSPITAL EXPANSION AND PARKING GARAGE
RIVERSIDE, CALIFORNIA

Job No.
07881-3

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B-1a

EXPLORATORY BORING NO. 1

Date Drilled: 12/20/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 799.0

Logged by: VJR

Measured Depth to Water(ft): 39.0

DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	SAMPLES		BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
				DRIVE	BULK				
		(SP) Sand, medium to coarse with fine, gravel and cobbles to 4", brown				50/4"	6.0	111	Ring
40			Groundwater			50/4"	17.2	Dist.	Ring
45						21 50/5"	10.5	123	Ring, DS
50						50	11.8	128	Ring
55						50	12.4	125	Ring
60						50/4"	15.4	123	Ring
65		NO BEDROCK REFUSAL AT 70.0' FILL TO 4.0' SLIGHT CAVING GROUNDWATER AT 39.0'				50/4"	9.6	134	Ring
		END OF BORING				50/1"	N R	N R	Ring

BORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ/GDT 1/18/08



C.H.J.

PROPOSED HOSPITAL EXPANSION AND PARKING GARAGE
RIVERSIDE, CALIFORNIA

Job No.
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Enclosure
B-1b

EXPLORATORY BORING NO. 2

Date Drilled: 12/20/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 800.0

Logged by: VJR

Measured Depth to Water(ft): 36.0

DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	SAMPLES		BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
				DRIVE	BULK				
		(ML) Sandy Silt, fine to medium with coarse and gravel to 1", light brown	Fill				7.3		SA, MDE, SE
5		(ML) Sandy Silt, fine with medium, light brown	Native			5 6 4	7.9		SPT
10						3 2 2			SPT
15		(SP-SM) Sand, medium to coarse with fine, silt and gravel to 1", brown				3 3 5	3.3		SPT
20		(SP) Sand, medium to coarse with fine and gravel to 1", brown				5 6 6			SPT
25						19 24 19			SPT
30		(SP-SM) Sand, fine to coarse with silt and gravel to 1", light brown				41 50/5"			SPT
						50			SPT

BORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ/GDT 1/18/08



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PROPOSED HOSPITAL EXPANSION AND PARKING GARAGE
RIVERSIDE, CALIFORNIA

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EXPLORATORY BORING NO. 2

Date Drilled: 12/20/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 800.0

Logged by: VJR

Measured Depth to Water(ft): 36.0

DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	SAMPLES		BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
				DRIVE	BULK				
		(SP-SM) Sand, fine to coarse with silt and gravel to 1", light brown	▼ Groundwater						
40				X		19 23 32			SPT
				X		50/5"			SPT
		END OF BORING							
45		NO BEDROCK REFUSAL AT 43.0' FILL TO 4.0' SLIGHT CAVING GROUNDWATER AT 36.0'							
50									
55									
60									
65									

BORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ.GDT 1/18/08



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PROPOSED HOSPITAL EXPANSION AND PARKING GARAGE
RIVERSIDE, CALIFORNIA

Job No.
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Enclosure
B-2b

EXPLORATORY BORING NO. 6

Date Drilled: 12/26/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 839.0

Logged by: JMZ

Measured Depth to Water(ft): 76.7

DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	SAMPLES		BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
				DRIVE	BULK				
		Asphalt							
		Base	Fill				7.3		
		(SM) Silty Sand, fine with gravel to 3", red brown				7 6 12	7.8	107	Ring
5		(SM) Silty Sand, fine, light brown	Native			12 20 31	14.1	106	Ring
10							10.8		
						9 13 17	10.8	120	Ring
15						9 16 39	15.0	117	Ring
20						11 13 18	12.8	113	Ring
25		(ML) Sandy Silt, fine, brown					25.0		
						13 19 23	24.4	102	Ring
30		(ML) Sandy Silt, fine with clay, red brown					13.6		
						13 18 30	13.3	122	Ring

BORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ.GDT 1/18/08



C.H.J.

PROPOSED HOSPITAL EXPANSION AND PARKING GARAGE Job No. 07881-3
RIVERSIDE, CALIFORNIA

Enclosure
B-6a

EXPLORATORY BORING NO. 6

Date Drilled: 12/26/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 839.0

Logged by: JMZ

Measured Depth to Water(ft): 76.7

DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	SAMPLES		BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
				DRIVE	BULK				
		(SC) Clayey Sand, fine to medium with silt, red brown					11.7		Cor.
40				X		10 18 35	10.8	125	Ring
		(SM) Silty Sand, fine to medium with coarse and gravel to 3/4", light brown					10.0		
45				X		17 30 50	10.3	126	Ring
		(SP-SM) Sand, fine to coarse with silt and gravel to 1", light brown					1.8		
50				X		48 43 41	1.8	Dist.	Ring
		(SP) Sand, fine to medium with coarse and gravel to 1", light brown							
55				X		40 50/5"	N.R.	N.R.	Ring
		(SP) Sand, fine to coarse with gravel and cobbles to 4", light brown							
60				X		50	N.R.	N.R.	Ring
65				X		50/4"	3.0	112	Ring
				X		50	3.3	96	Ring

BORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ/GDT 1/18/08



C.H.J.

PROPOSED HOSPITAL EXPANSION AND PARKING GARAGE Job No. 07881-3
RIVERSIDE, CALIFORNIA

Enclosure
B-6b

EXPLORATORY BORING NO. 6

Date Drilled: 12/26/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 839.0

Logged by: JMZ

Measured Depth to Water(ft): 76.7

DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	SAMPLES		BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
				DRIVE	BULK				
75		(SP) Sand, fine to coarse with gravel and cobbles to 4", light brown				50/4"	4.2	105	Ring
80		(SM) Silty Sand, fine to coarse with clay and gravel 3", brown	Groundwater			50/5"	9.6	114	Ring
85		END OF BORING				50/2"	N.R.	N.R.	Ring
90		NO BEDROCK REFUSAL AT 83.0' FILL TO 5.0' SLIGHT CAVING GROUNDWATER AT 76.7'							
95									
100									

BORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ/GDT 1/18/08



C.H.J.

PROPOSED HOSPITAL EXPANSION AND PARKING GARAGE
RIVERSIDE, CALIFORNIA

Job No.
07881-3

Enclosure
B-6c

EXPLORATORY BORING NO. 7

Date Drilled: 12/26/07

Client: Riverside Community Hospital

Equipment: CME 75 Track Rig

Driving Weight / Drop: 140 lbs./ 30 in.

Surface Elevation(ft): 825.0

Logged by: JMZ

Measured Depth to Water(ft): N.A.

DEPTH (ft)	GRAPHIC LOG	VISUAL CLASSIFICATION	REMARKS	SAMPLES		BLOWS/6 IN.	FIELD MOISTURE (%)	DRY UNIT WT. (pcf)	LAB/FIELD TESTS
				DRIVE	BULK				
		Asphalt							
		Base	Fill				6.5		
		(SM) Silty Sand, fine to medium with coarse and gravel to 3", brown							
5		(SM) Silty Sand, fine to coarse with gravel to 1", brown	Native				5.8		Cor.
				X		6 8 8			SPT
10				X		3 3 3			SPT
15				X		2 4 3			SPT
20		(SP-SM) Sand, fine to medium with coarse and silt, light brown		X		2 3 5			SPT
25		(SP-SM) Sand, fine to coarse with silt and gravel to 1", light brown		X		6 8 11			SPT
30		(SP) Sand, fine to coarse, light brown		X		8 12 19			SPT
		(SP) Sand, fine to coarse with gravel to 1", light brown							

BORING LOG - NO EQUIV & BLOW PER 6 IN 07881-3.GPJ CHJ.GDT 1/18/08



C.H.J.

PROPOSED HOSPITAL EXPANSION AND PARKING GARAGE
RIVERSIDE, CALIFORNIA

Job No.
07881-3

Enclosure
B-7a